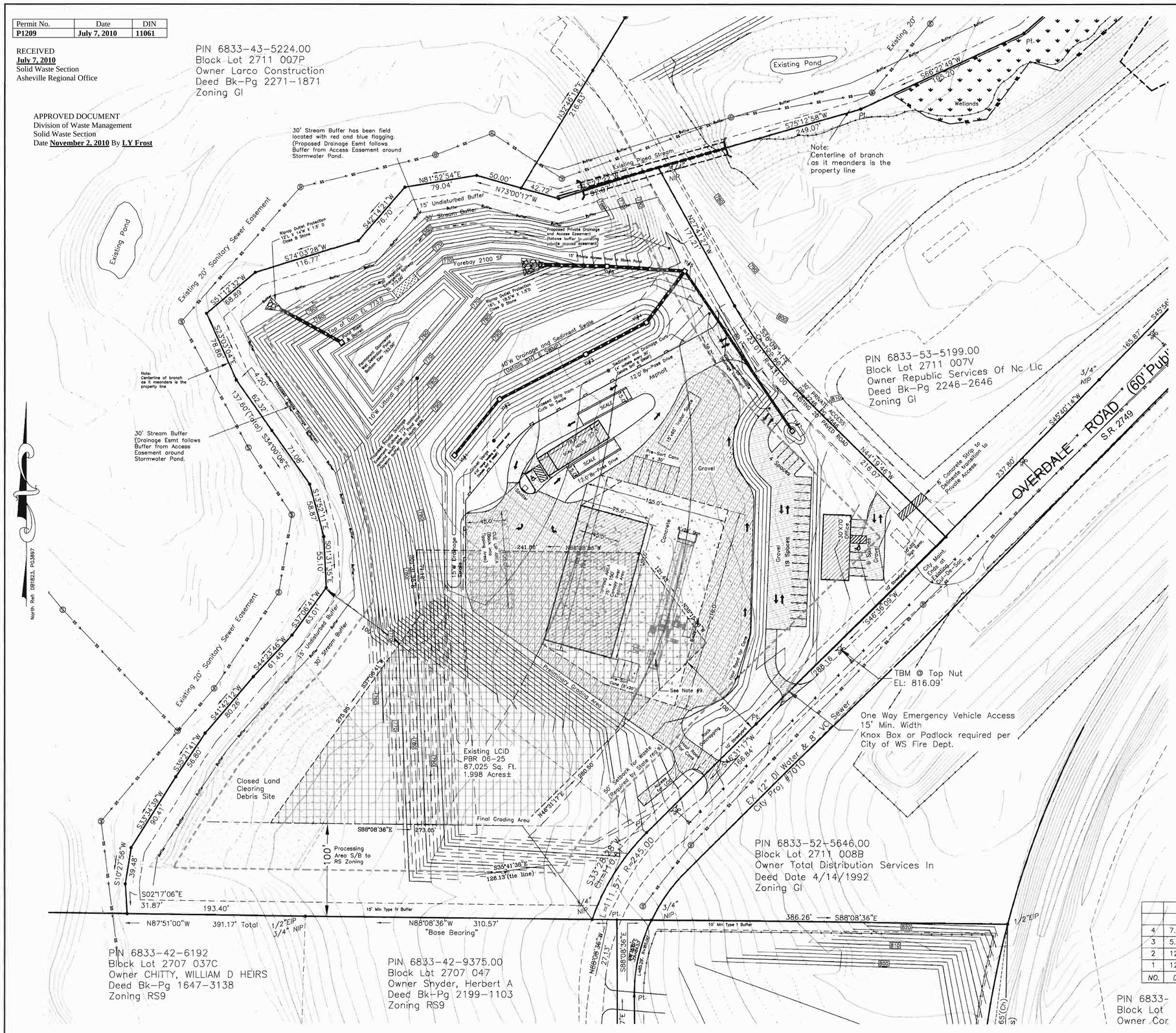
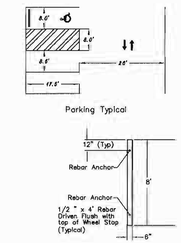
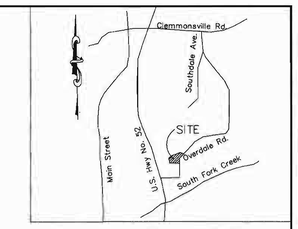
Permit No.	Date	DIN
P1209	**July 7, 2010**	**11061**

RECEIVED
July 7, 2010
Solid Waste Section
Asheville Regional Office

APPROVED DOCUMENT
Division of Waste Management
Solid Waste Section
Date **November 2, 2010** By **LY Frost**







Location Map N.T.S.

The Purpose of this Submittal is for Inspections Site Plan review.

GENERAL NOTES:
1. This map is a compilation of information from deeds, county tax information, structures and topographic information furnished by the Forsyth County GIS.
2. All utilities indicated are provided from Forsyth County GIS and Utilities Administration files. Manholes and water infrastructure on ground have been field located.
3. No work may be performed inside the 30' stream buffer until Corp of Engineers permitting and City of WS buffer variance is issued.
4. Stream has been field located. 30' Stream Buffers have been located with red and blue flagging.
5. Owner to obtain driveway permit prior to construction.
6. Wetlands have been delineated as shown on plans.
7. A 50' grading easement for the North Site will be required prior to grading operations beginning on adjacent owner.
8. Existing LCID to be used outside of graded area only.
9. The sorting line and area near the sorting line hopper shall be covered by roof system or other fixed canopy structures to prevent rainwater from contacting materials being processed.

Line Legend

- Public Water Line
- Public Sanitary Sewer Line
- Underground Electric Line
- Underground Gas Line
- Drainage Limits
- Fence
- Ditchline
- Wetlands Boundary
- Stream or Creek Buffer

Symbol Legend

- Pt. Point (As Shown)
- I.P.S. Iron Pin Set (3/4" conduit)
- E.I.P. Existing Iron Pin (As Shown)
- Stone (Found)
- NCDOT Monument/Disc
- Utility Pole
- Fire Hydrant
- Water Valve
- Control Corner
- FG FG Flush with ground
- AG AG x.xx' Above ground(Distance in ft.)
- UG UG x.xx' Underground(Distance in ft.)
- Sanitary Sewer Manhole
- Electrical Pedestal
- Wetlands
- Specimen Tree (symbols vary)
- Telephone Pedestal
- Electrical Transformer Box

ZONING

Existing Zoning: GI
Proposed Zoning: GI
Type of Review Requested: Inspections
Jurisdiction: City Of Winston-Salem

SITE SIZE AND COVERAGES

Total Acreage: 10.227 ± Acres.
Site Coverages: 445,487 sq ft.
Building to Land 0.9 %
Pavement to Land: 21.0 %
Open Space: 78.1 %
(Total = 100%)
Building Sq. Footage (Max.): 12000 Sq. Ft.
Building Height: 40 Ft.
Public Streets: N/A (approx.)

INFRASTRUCTURE

Water: X Public ___ Private
Sewer: X Public ___ Private
Streets: ___ Public X Private

OFF-STREET PARKING

Proposed Uses: Recycling Plant
Parking Calc.: 1 Spaces/ emp + 2 spaces
Required Parking: 29 Spaces.
Parking Provided: 30 Spaces.

BUFFERYARDS (if applicable)

Adjoining Zoning: GI and RS9
Type Required: Type IV
Width Provided: 15' Min.
Fence Option: ___ Yes X No

NOT AN ACTUAL SURVEY

NORTH CAROLINA PROFESSIONAL SEAL 4896 ENGINEER JOHN EDWARD BEESON

CORP#: C-1144

NO.	DATE	DESCRIPTION	BY
4	7.1.10	Added Note #9 on roof system	ATC
3	5.3.10	Revised Site layout	ATC
2	12.18.09	Note on Fire Access	ATC
1	12.8.09	Updates and revisions	ATC

Preliminary Site Plan
Overdale Recycling Plant

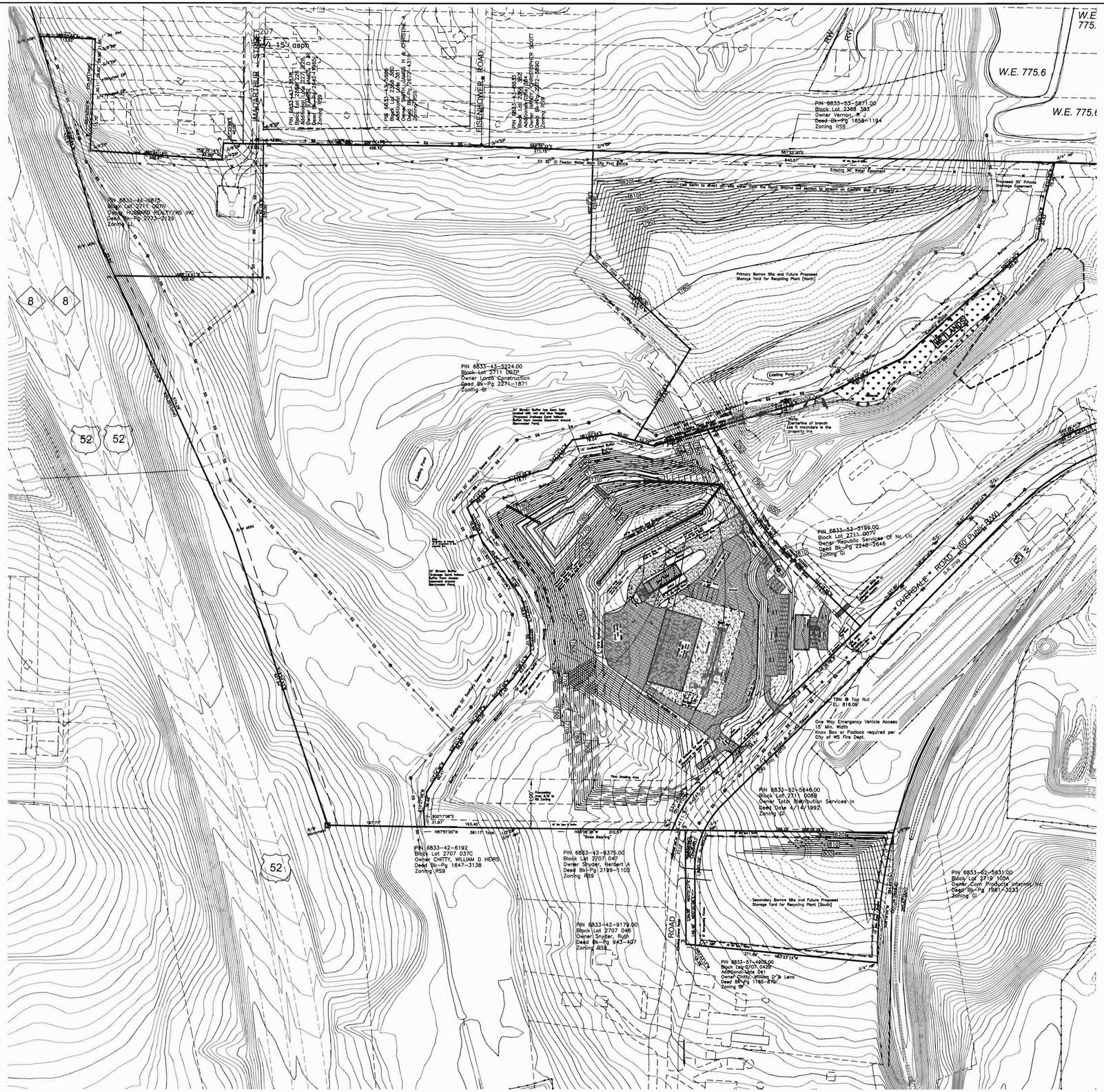
ABBEY GREEN INC.
4400 Silas Creek Pkwy, Suite 301
Winston-Salem, NC 27104
(336) 784-1890
bryanlj@mywway.com

Scale: 1" = 50'
50 25 0 25 50 100

FIELD WORK BY		CHECKED BY:
Others		JEB
Block / Lot	PIN:	Deed Book/Page
2711 007W	6833-52-1826.00	DB 2175 PG 3070
2711 007T	6833-42-9505.00	DB 2276 PG 2177
2707 041A	6833-43-3283.00	DB 2287 PG 2430
2711 007U	6833-53-3585.00	DB 2276 PG 2177

TAX MAP:	CITY:	COUNTY:
630830	Winston-Salem	Forsyth
STATE:	DATE:	SHEET NUMBER:
North Carolina	08-19-2008	1 of 2
JOB NUMBER:	DRAWN BY:	
08100.051	ATC	

BEESON ENGINEERING INC.
ENGINEERS SURVEYORS PLANNERS
503 HIGH STREET
WINSTON-SALEM, NC 27101
TELEPHONE: (336) 748-0071
FAX: (336) 748-0470
www.beesonengineering.com



NOT AN ACTUAL SURVEY

Preliminary Site Plan

Overdale Recycling Plant

ABBEY GREEN INC.
4400 Silas Creek Pkwy, Suite 301
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(336) 784-1890
bryanlj@mpway.com

Scale: 1" = 100'

100 50 0 50 100 200

FIELD WORK BY		CHECKED BY:
Others		JEB
Block / Lot	PIN:	Deed Book/Pages
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2711 007T	6833-42-9606.00	DB 2576 PG 2177
2707 042A	6833-63-3388.00	DB 2576 PG 2180
2711 007U	6833-63-3686.00	DB 2576 PG 2177

TAX MAP:	CITY:	COUNTY:
630830	Winston-Salem	Forsyth
STATE:	DATE:	SHEET NUMBER:
North Carolina	08-19-2008	2 of 2
JOB NUMBER:	DRAWN BY:	
08100.051	ATC	

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 4898 JOHN EDWARD BEESON

CORP#: C-1144

NO.	DATE	DESCRIPTION	BY
3	5.3.10	Revised Site layout	ATC
2	12.18.09	Note on Fire Access	ATC
1	12.8.09	Updates and revisions	ATC

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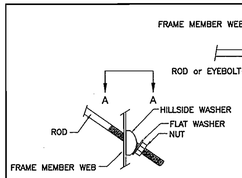
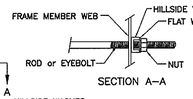
TELEPHONE: (336) 748-0071
FAX: (336) 748-0470
www.beesonengineering.com

RB0101-1

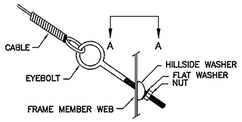
THRU WEB TYPE BRACING



THRU WEB ROD BRACE END DETAIL

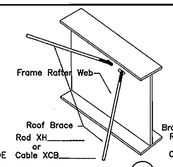


SECTION A–A

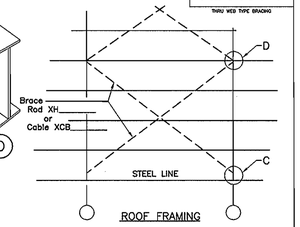


THRU WEB CABLE BRACE END DETAIL

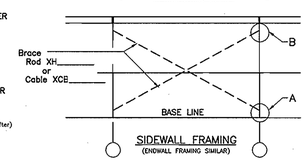
IN ORDER FOR THE GALVANIZED WIRE STRAND TO PERFORM PROPERLY IT IS NECESSARY TO PROVIDE AN INITIAL "PRELOAD" BY DRAWING THE BRACING CABLE UP TO A MAXIMUM INTIAL DRAPE. THE MAXIMUM DRAPE FOR CABLE SIZES ARE: 1" FOR 1/4 AND 3/8 CABLE AND 2" FOR 1/2 CABLE THE CABLE DRAPE IS THE VERTICAL DISTANCE MEASURED AT MID SPAN BETWEEN THE TWO CABLE END POINTS. CAUTION: INSTALLER SHOULD NOT OVERDRAW THE CABLE AS THIS MAY PULL THE FRAME OUT OF PLUMB OR MAY OVERLOAD COMPONENTS OF THE FRAME.



AT FRAME RAFTER (D)



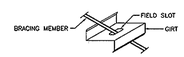
ROOF FRAMING

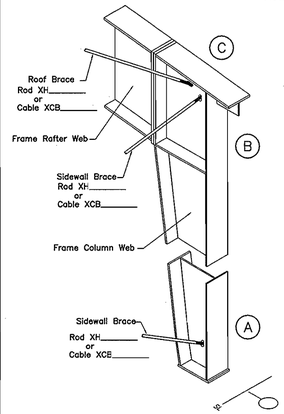


SIDEWALL FRAMING
(ENDWALL FRAMING SIMILAR)

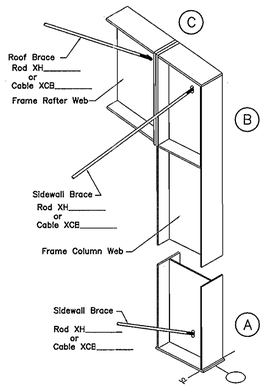
SPECIAL NOTE AT FLUSH GIRT CONDITIONS

IF FLUSH FRAMED GIRTS ARE LOCATED IN BRACED BAYS AND INTERFERE WITH THE BRACING MEMBER THE GIRTS MUST BE FIELD SLOTTED TO ALLOW PASSAGE OF THE BRACING MEMBER.



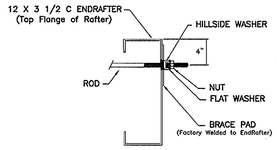


AT EXTERIOR FRAMED TAPERED COLUMN
(STRAIGHT COLUMN SIMILAR)

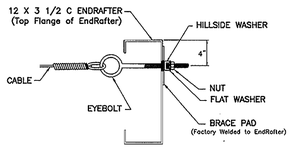


AT FLUSH FRAMED STRAIGHT COLUMN
(TAPERED COLUMN SIMILAR)

THRU WEB TYPE BRACING



WITH ROD BRACE



WITH CABLE BRACE

SECTION THRU ENDRAFTER
12 x 3 1/2 C

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0		ISSUED FOR ERECTOR INSTALLATION			

These drawings and the metal building they represent are the product of Mesco Building Solutions or another division or affiliate of NCI Group, Inc. - 5244 Bear Creek Court, Irving, TX 75061
The engineer whose seal appears hereon is employed by Mesco Building Solutions or another division or affiliate of NCI Group, Inc. and is not the engineer of record for this project.

Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

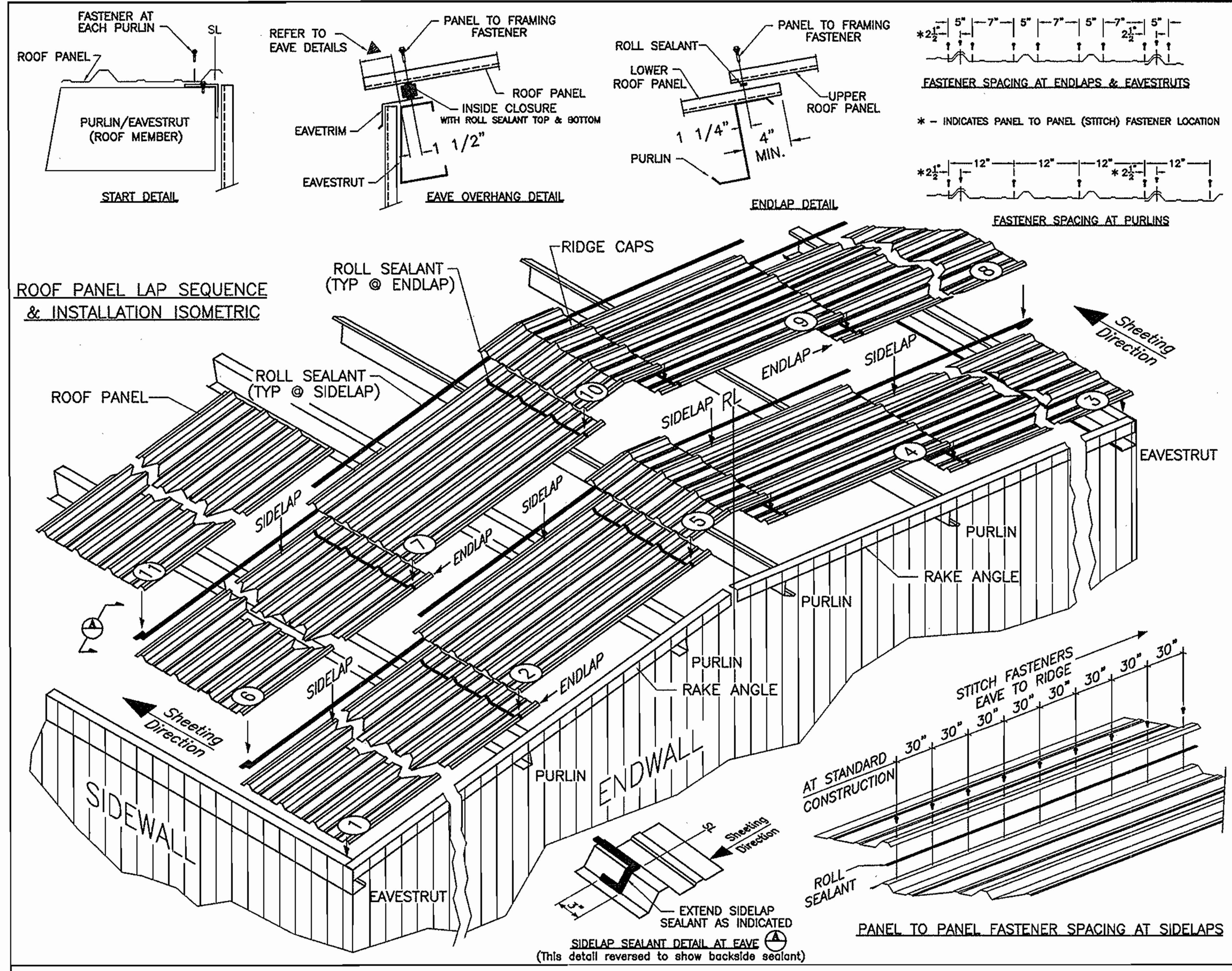
MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: ABBY GREEN,INC
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: S4 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION "
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTHACHAI SUVUNNINGSI
JUN 28 2010

2/23/2010 10:26 ver. 2006.26.08 Machine



RR0101-2

GABLE ROOF STANDARD CONSTRUCTION

PBR ROOF PANEL INSTALLATION INSTRUCTIONS
(Gable Roof Standard Construction)

1. INSTALL FIRST ROOF PANEL AT EAVE WITH THE CL OF RIB ALIGNED WITH THE ENDWALL STEEL LINE AS SHOWN IN THE START DETAIL AND EAVE OVERHANG DETAIL. INSTALL PANEL TO FRAMING FASTENERS AT EVERY PURLIN AS SHOWN.
2. SHOULD ENDLAP BE REQUIRED, APPLY ROLL SEALANT TO LOWER PANEL AS SHOWN IN ENDLAP DETAIL. ATTACH PANEL TO FRAMING FASTENERS AT SPACING SHOWN FOR ENDLAP AND EAVESTRUT.
3. INSTALL PANEL TO FRAMING FASTENERS AT ALL PURLINS OTHER THAN ENDLAP LOCATIONS AS SHOWN IN SPACING DETAIL.
4. PROCEED SHEETING BUILDING FROM RIGHT TO LEFT AS SHOWN IN PANEL LAP SEQUENCE & INSTALLATION ISOMETRIC DRAWING.
5. APPLY ROLL SEALANT IF REQUIRED TO RIB OF PREVIOUSLY INSTALLED PANEL. LOCATE NEXT PANEL AS THE FIRST, INSURING THAT PANEL ENDS ARE ALIGNED PROPERLY. INSTALL SIDELAP FASTENERS AT SPACING SHOWN FOR STANDARD ROOF CONSTRUCTION AS REQUIRED.
6. CONTINUE SHEETING ACROSS BUILDING UNTIL ROOF IS COMPLETE. INSTALL RIDGECAPS USING ROLL SEALANT AS SHOWN IN ENDLAP DETAIL AND ATTACH FASTENERS AT ENDLAP SPACING ON BOTH SIDES OF RIDGE.

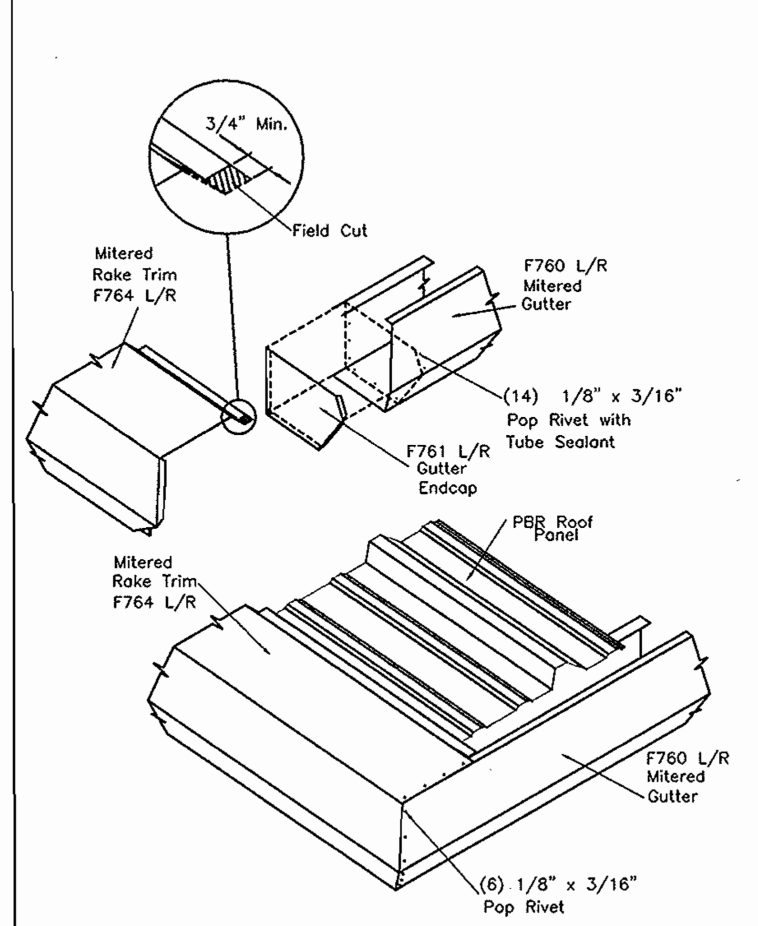
GENERAL NOTES

1. REFER TO RG0103 & TITLE SHEET FOR FASTENER TYPE AND RECOMMENDED INSTALLATION.
2. NET COVERAGE OF PBR PANEL IS 36 INCHES.
3. THE ROOF MUST BE SWEPT CLEAN DAILY, OF ALL METAL PARTICLES AFTER FASTENERS ARE INSTALLED, TO PREVENT PANEL DAMAGE.
4. REFER TO EAVE AND RAKE DETAILS FOR EAVE AND RAKE TRIM INSTALLATION REQUIREMENTS. EAVE TRIM MUST BE INSTALLED PRIOR TO ROOF PANEL INSTALLATION.
5. DIRECTION OF WORK SHOULD BE TOWARD PREVAILING WINDS.
6. THIS IS A STANDARD REFERENCE DRAWING TO BE USED IN CONJUNCTION WITH THE ERECTION DRAWINGS AND OTHER DRAWINGS OF THIS SERIES FOR RAKE ANGLE & CAP CHANNEL LOCATIONS.
7. REFER TO ERECTION DRAWINGS AND DETAIL SHEETS FOR NON–STANDARD DETAILS AND SECTIONS.

NOTE: CAUTION!!
WHEN NORTHERN ROOF CONSTRUCTION IS REQUIRED REFER TO THE REFERENCE DRAWING RR0107. SOME OF THE FASTENER SPACINGS AND SEALANT TYPES FOR NORTHERN ROOF CONSTRUCTION ARE DIFFERENT THAN THOSE SHOWN ON THIS DRAWING.

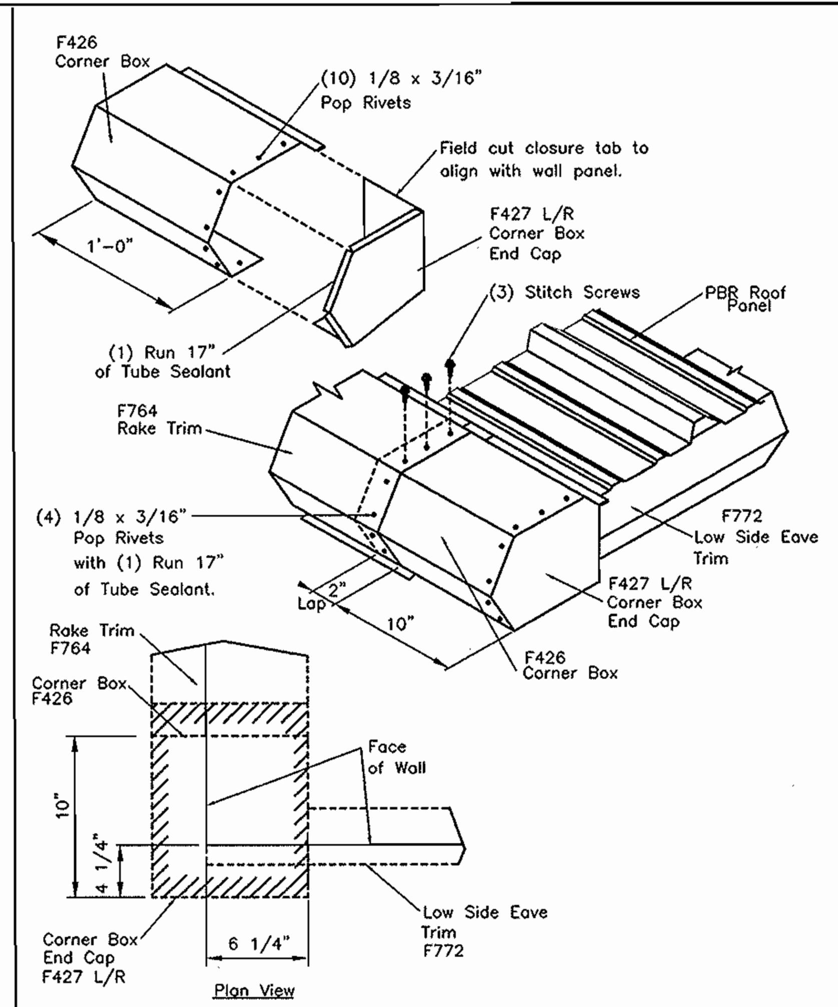
RR0307-0

EAVE/EAVE CORNERS LS – STANDARD TRIM



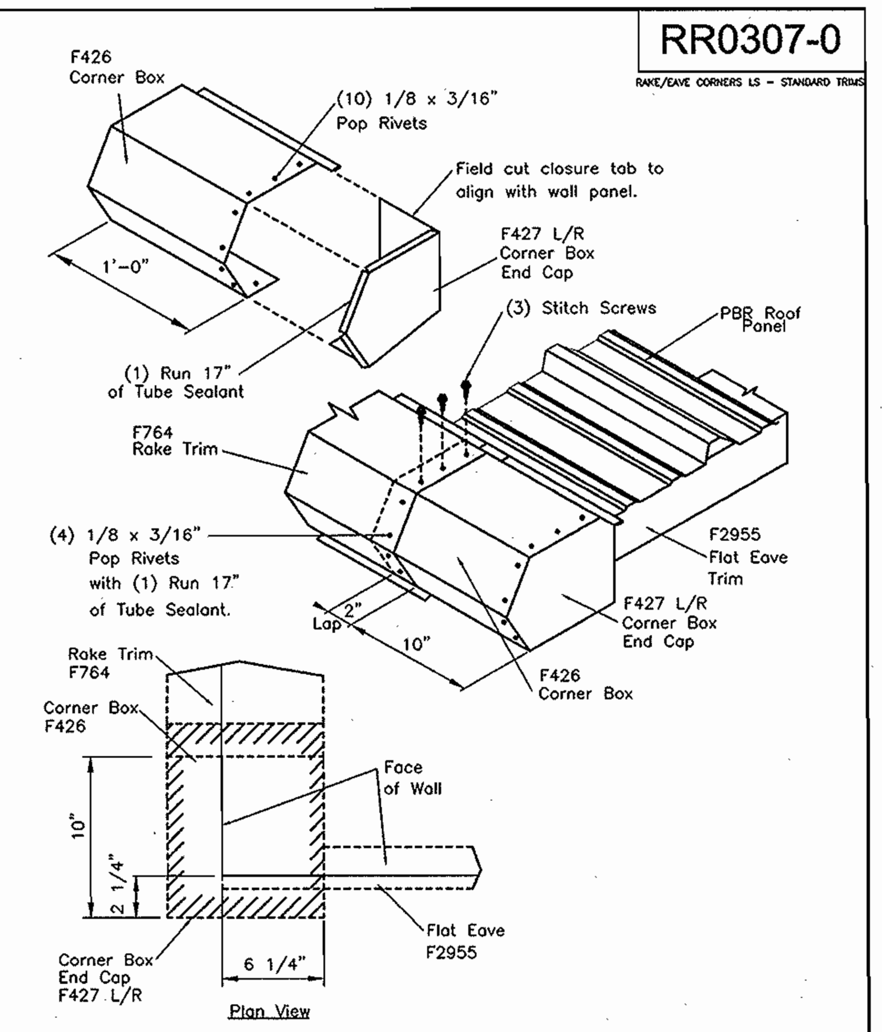
LOW SIDE RAKE CORNER with EAVE GUTTER
(Standard)

Locate the gutter end cap 1 1/2" from end of gutter.
Apply one bead of tube sealant between End Cap and the Gutter.
Attach the Endcap to the Gutter using (14) 1/8" x 3/16" Pop Rivets.
Check to insure all edges are sealed with Tube Sealant.
Insert Gutter into Rake Trim, align the Mitered edges and attach to the Rake Trim with (6) 1/8" x 3/16" Pop Rivets.



LOW SIDE RAKE CORNER with LOW SIDE EAVE TRIM
(Standard)

Apply 17" of tube sealant along tops of Corner Box Endcap and attach to the end of the corner box with (10) 1/8" x 3/16" Pop Rivets.
Apply 17" of tube sealant along the profile of the Corner Box and align with the end of the Rake Trim as shown in the Plan View.
Note: Corner Box laps under Rake Trim.
Attach Corner Box to the Rake Trim with (3) Stitch Screws and (4) 1/8" x 3/16" Pop Rivets.



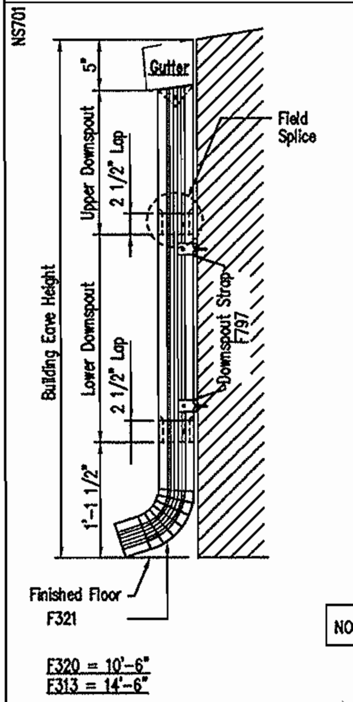
LOW SIDE RAKE CORNER with FLAT EAVE TRIM
(Standard)

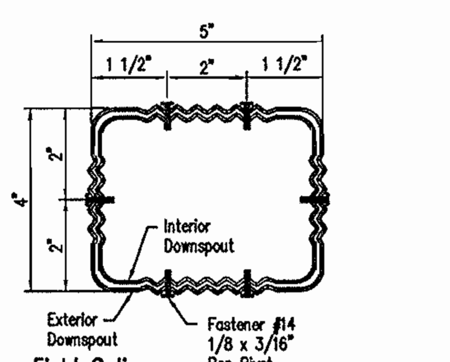
Apply 17" of tube sealant along tops of Corner Box Endcap and attach to the end of the corner box with (10) 1/8" x 3/16" Pop Rivets.
Apply 17" of tube sealant along the profile of the Corner Box and align with the end of the Rake Trim as shown in the Plan View.
Note: Corner Box laps under Rake Trim.
Attach Corner Box to the Rake Trim with (3) Stitch Screws and (4) 1/8" x 3/16" Pop Rivets.

RW0303-0

ROLL FORMED DOWNSPOUTS PG.1

NS701





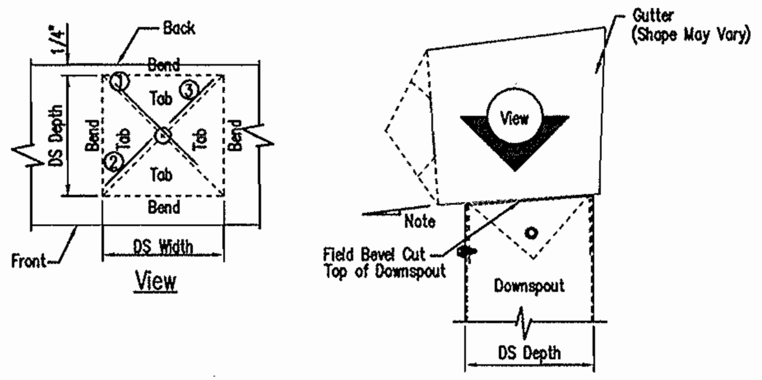
Field Splice
A field splice is required for eave heights greater than 16'–0". Field Cut downspout to length. Use a 10'–6" or 14'–6" length of downspout. Always measure the required length starting at the swaged end. After cutting to length, insert the swaged end of the cut downspout into the top of the lower downspout. Secure with pop rivets.

NOTE: A Downspout Strap is required at all Downspout Splices.

F320 = 10'–6"
F313 = 14'–6"

4" x 5" Roll Form Downspout Elevation

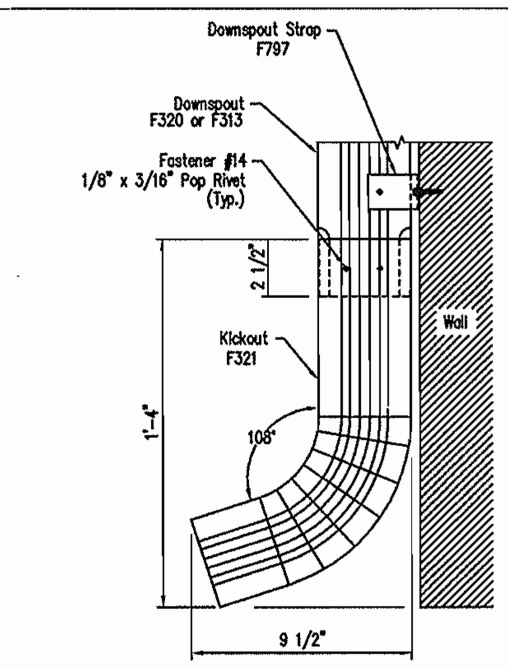
NS702



1. Refer to the building erection drawings for the location and spacing of the downspouts.
2. Locate all downspouts over a major panel rib if possable.
3. Make a cardboard template of the downspout shape . Place the template on the bottom of the gutter and trace the outline. Remove the template and draw a line from corner to corner, forming an "X" pattern.
4. Drill a whole at the center of the "X". Using tin snips, cut along the lines of the X only. Do not cut along the outside lines of the downspout square.
5. Bend each triangular tab down toward the ground, 90 Degrees to the bottom of the gutter.
6. Position the top of the downspout under the gutter. Make sure all four gutter tabs are on the inside of the downspout.
7. Install Fastener #4 1/4–14 X 7/8 Lap Tek self drilling fastener through the downspout into the gutter tab. Only the two sides and the front of the downspout will receive fasteners.

Downspout To Gutter Attachment Detail

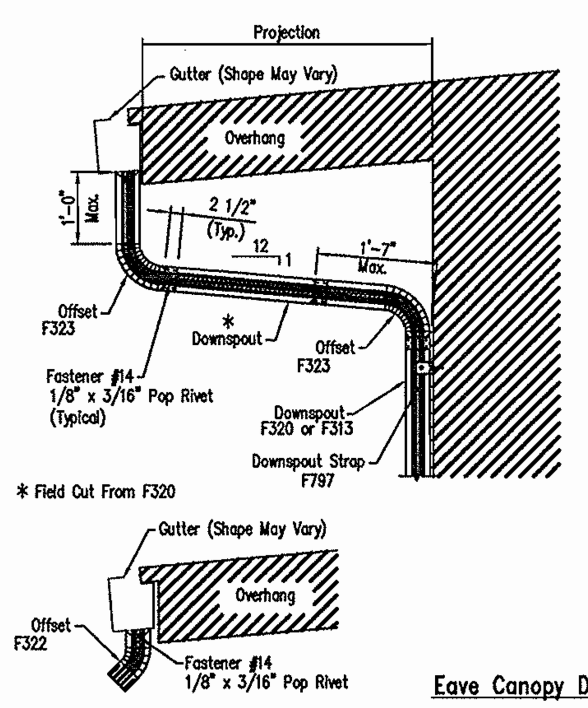
NS703



4" x 5" Roll-Form

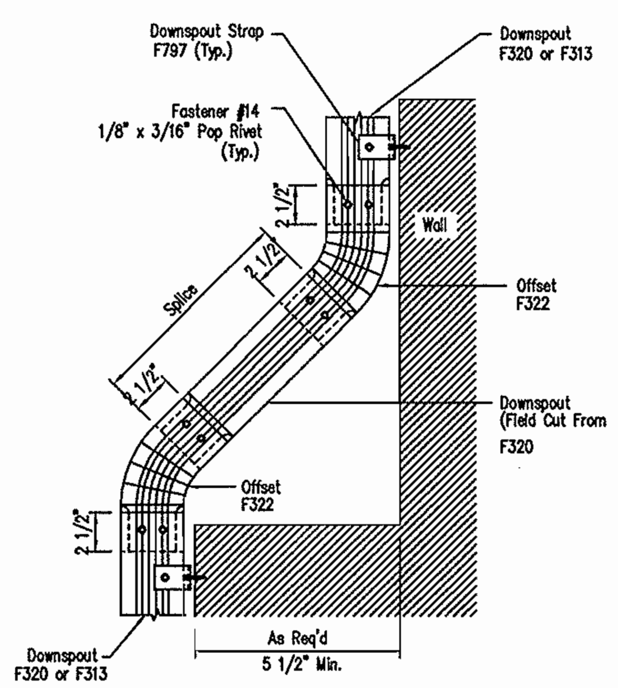
Downspout Kickout

NS704



Eave Canopy Downspout Detail

NS705



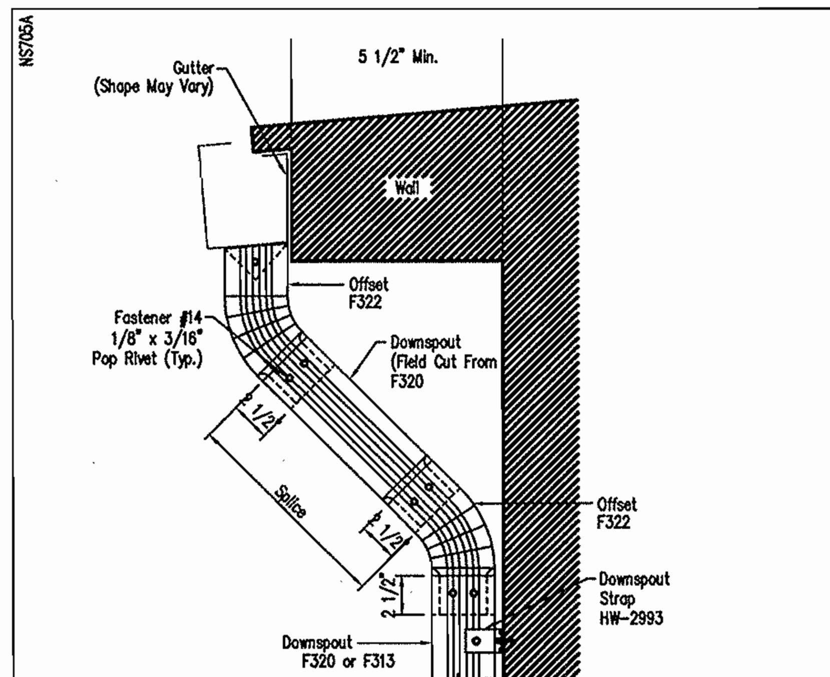
4" x 5" Roll-Form

Wall Offset

RW0304-0

ROLL FORMED DOWNSPOUTS PG.2

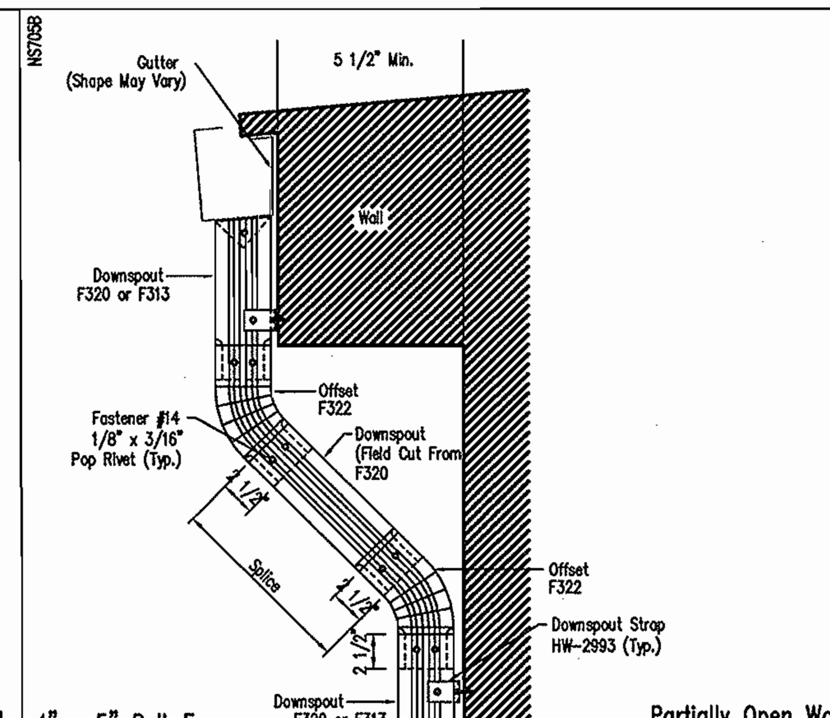
NS705A



4" x 5" Roll-Form

Full Height Open Wall

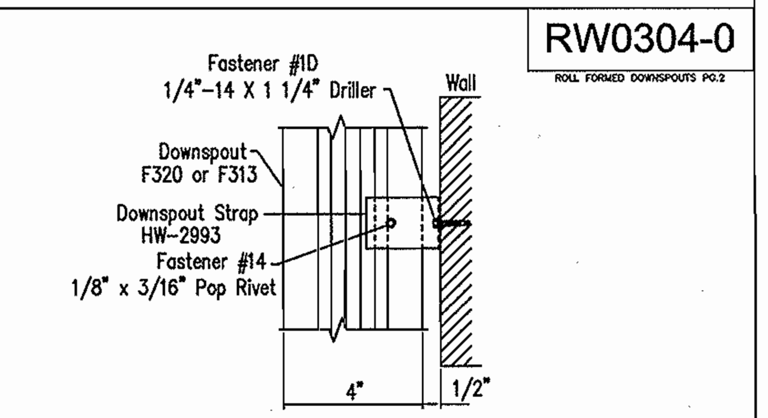
NS705B



4" x 5" Roll-Form

Partially Open Wall

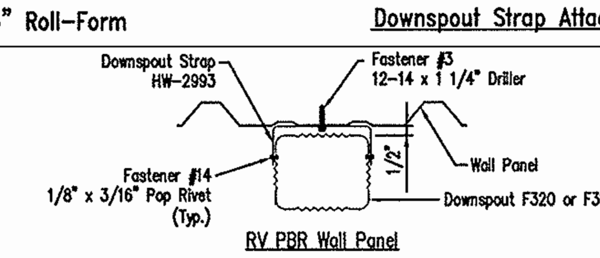
NS706



4" x 5" Roll-Form

Downspout Strap Attachment Detail

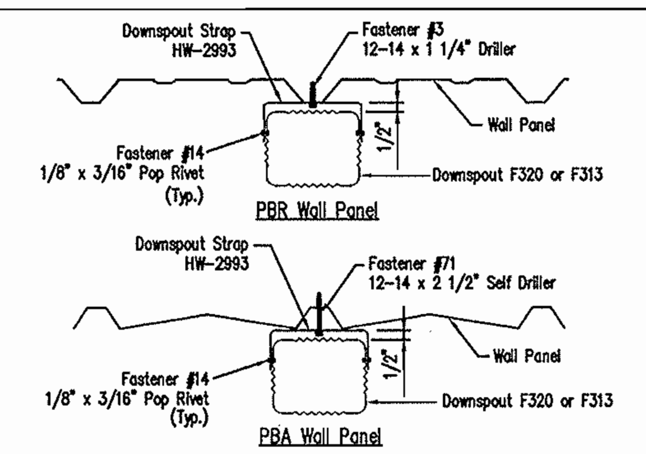
NS707



4" x 5" Roll-Form

Downspout Strap Attachment Detail

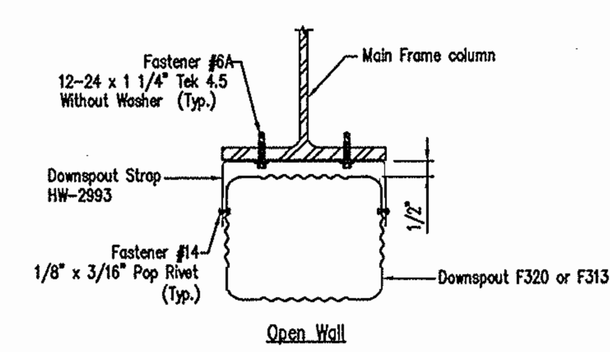
NS707



4" x 5" Roll-Form

Downspout Strap Attachment Detail

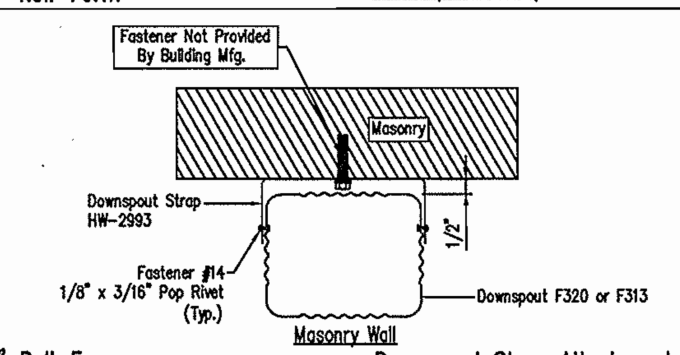
NS708



4" x 5" Roll-Form

Downspout Strap Attachment Detail

NS709



4" x 5" Roll-Form

Downspout Strap Attachment Detail

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0		ISSUED FOR ERECTOR INSTALLATION			

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MESCO

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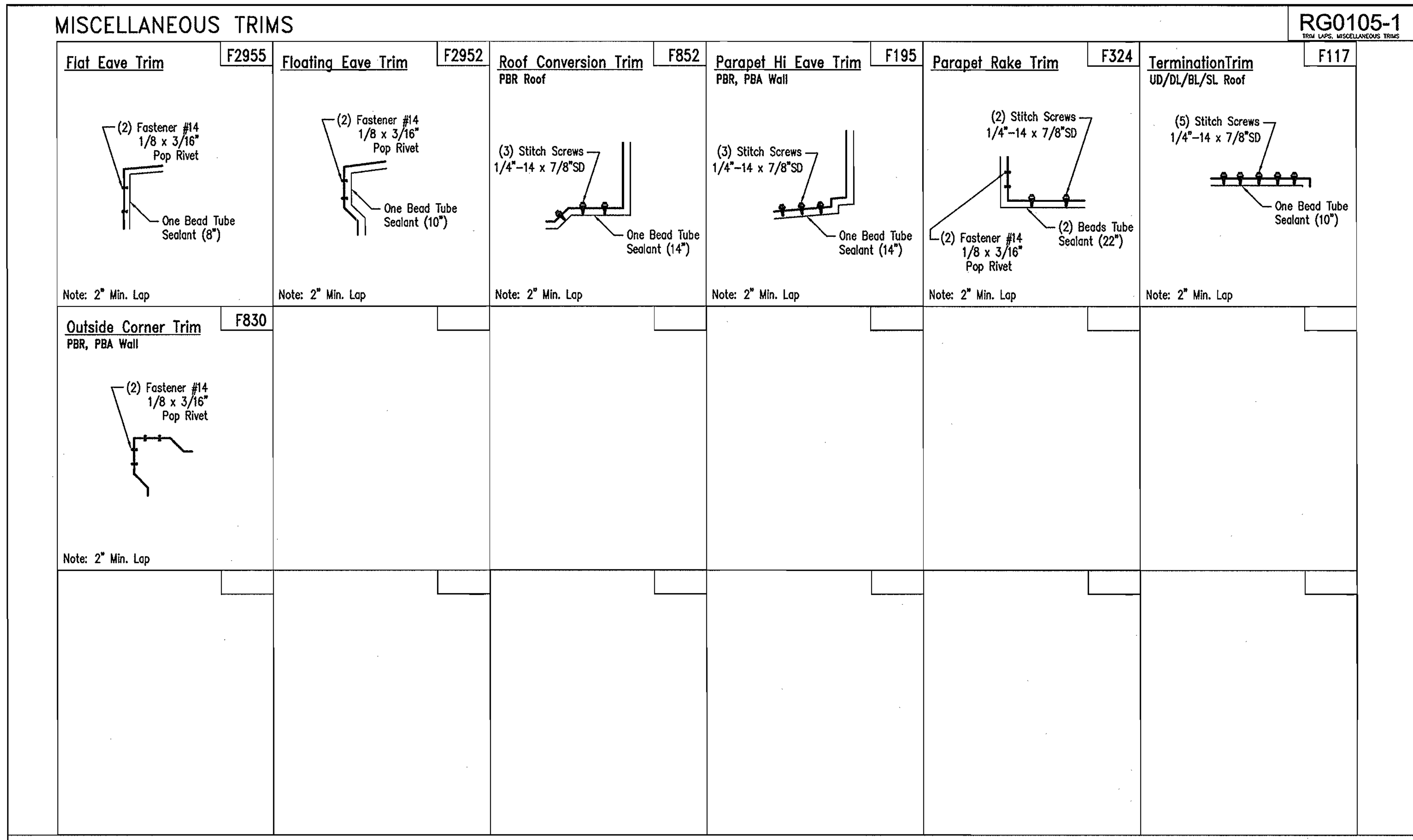
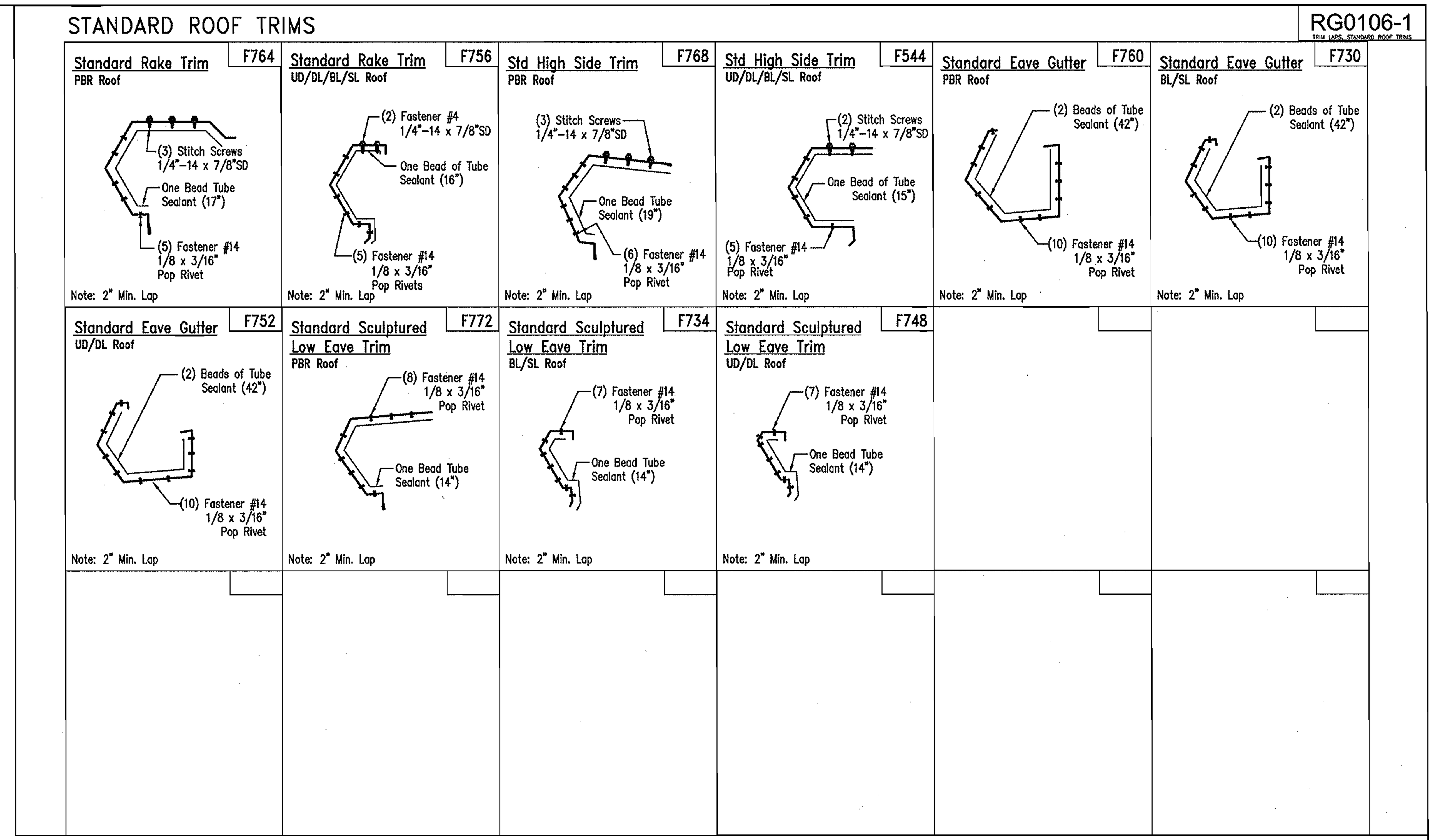
MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: ABBY GREEN,INC
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: S3 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTHACHAI SUVUNRUNGSI
JUN 28 2010





Bolt Tightening Methods

RG0109-0

CONNECTION BOLTS

Bolts used to make connections in primary framing members such as columns and rafters are usually ASTM A325 bolts with A563 grade C nuts. Bolts used to make connections in secondary framing members such as the purlins and girts are usually ASTM A307 bolts with A563 grade A nuts. The size and grade of the bolts are indicated on the erection drawings marked "Issued For Erector Installation".

The reuse of galvanized A325 bolts / A563 grade C nuts and A490 bolts / A563 grade DH nuts is not permitted.

The following information is taken from the 2004 Edition RCSC (Research Council on Structural Connections). This is provided for the installer as a guide. It is the responsibility of the installer to assure proper tightness of all bolts.

SNUG-TIGHTENED JOINTS

All bolt holes shall be aligned to permit insertion of the bolts without undue damage to the threads. Bolts shall be placed in all holes and nuts threaded to complete the assembly. Compacting the joint shall progress systematically from the most rigid part of the joint. The snug-tightened condition is the tightness that is attained with a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench to bring the connected piles into firm contact. More than one cycle through the bolt pattern may be required to achieve the snug-tighten joint.

TURN-OF-NUT PRETENSIONED JOINTS

When turn-of-nut tightening is used, hardened washers are not required unless noted as required on the erection drawings marked "Issued For Erector Installation".

Pre-Installation Verification Procedures:

A representative sample of not fewer than three complete fastener assemblies of each combination of diameter, length, grade and lot to be used in the work shall be checked at the site of installation in a tension calibrater to verify that the pretensioning method develops a pretension that is equal to or greater than 1.05 times that specified for installation and inspection in *Table 4*.

All bolts shall be installed in accordance with the *snug-tightened joint* requirements described above. Following the initial snug-tightening operation, all bolts in the connection shall be tightened futher by the applicable amount of rotation specified in *Table 5*. The part not turned by the wrench shall be prevented from rotation during the operation.

See *Turn-of Nut Sequence Order* and *Turn-of Nut Procedure* for additional information. Match marking of the nut and protruding end of the bolt after snug-tightening can be helpful in the subsequent installation process, and is certainly an aid to inspection.

Table 4
Minimum Bolt Pretension for Pretensioned and Slip Critical Joints

Nominal Bolts Diameter	Minimum Tension in 1000's of Pounds (kips)	
	A325 Bolts	A490 Bolts
1/2	12	15
5/8	19	24
3/4	28	35
7/8	39	49
1	51	64
1 1/8	56	80
1 1/4	71	102
1 3/8	85	121
1 1/2	103	148

[a] Equals to 70 percent of specified minimum tensile strengths of bolts (as specified in ASTM Specifications for tests of full size A325 and A490 bolts with UNC threads loaded in axial tension) rounded to the nearest kip.

Table 5
Nut Rotation from Snug Tight Condition

Bolts length (under side of head to end of bolt)	Disposition of Outer Face of Bolted Parts: Both faces normal to bolt axis	One face normal to bolt axis and other sloped not more than 1:20 (beveled washer not used)	Both faces sloped not more than 1:20 from normal to the bolt axis (beveled washer not used)
Up to and including 4 diameters	1/3 turn	1/2 turn	2/3 turn
Over 4 diameters but not exceeding 8 diameters	1/2 turn	2/3 turn	5/6 turn
Over 8 diameters but not exceeding 12 diameters[c]	2/3 turn	5/6 turn	1 turn

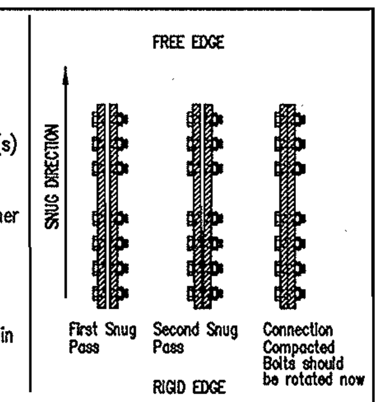
[a] Nut rotation is relative to bolt regardless of the element (nut or bolt) being turned. For bolts installed by 1/2 turn and less, the tolerance should be plus or minus 30 degrees; for bolts installed by 2/3 turn and more, the tolerance should be plus or minus 45 degrees.

[b] Applicable only to connections in which all material within the grip of the bolt is steel.

[c] No research has been performed by the Council to establish the turn-of- nut procedure for bolt lengths exceeding 12 diameters. Therefore, the required rotation must be determined by actual test in a suitable tension measuring device which simulates conditions of solidly fitted steel.

Turn-of-Nut Sequence Order

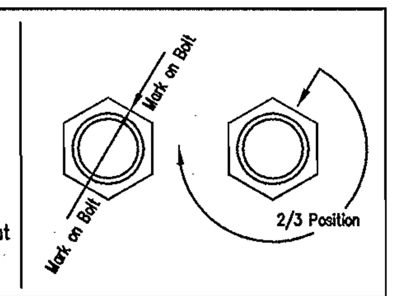
1. Install all bolts in connection.
2. Beginning at a rigid point in the connection (point with no gap), tighten each row of bolts to snug tight progressing toward the free edge(s) of the connection (edge with gap).
3. Repeat step 2 until all plies of the connection are completely pulled together with no gap present, and all bolts are snug tight.
4. Mark each bolt head or nut at its present position. Rotate bolt required amount shown in the table. Difference in mark locations will insure correct bolt tightness.



Turn-of-Nut Procedure

When connection is compacted and bolt is snug:

1. Mark nut and bolt location with keel or chalk.
2. Rotate nut as required per chart conditions.
3. Finished mark locations are proof that bolts have been properly tightened.



RG0110-0

DEALER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE DEALER/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT THAT MESCO METAL BUILDINGS OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION PROJECT.

THE DEALER/CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS REQUIRED.

APPROVAL OF MESCO'S DRAWINGS AND CALCULATIONS INDICATE THAT MESCO METAL BUILDINGS CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD PRACTICES, 9TH ED.)

WHERE DISCREPANCIES EXIST BETWEEN MESCO'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE 9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY MESCO METAL BUILDINGS ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN MESCO METAL BUILDING'S ENGINEERS UNLESS SPECIFICALLY INDICATED.

THE DEALER/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH MESCO METAL BUILDING'S "FOR CONSTRUCTION" DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY MESCO FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSEWORK, CRIBBING OR OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. (SECT. 7.9.1 AISC CODE OF STANDARD PRACTICE, 9TH ED.)

GENERAL NOTES

THE STRUCTURE UNDER THIS PURCHASE ORDER HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS STIPULATED IN THE PURCHASE ORDER AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. MESCO METAL BUILDINGS WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

THIS METAL BUILDING IS DESIGNED WITH MESCO METAL BUILDING'S STANDARD PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES.

1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDIINGS"
2. AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS"
3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1.
4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL"
5. INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS: "UNIFORM BUILDING CODE"
6. SOUTHERN BUILDING CODE CONGRESS INTERNATIONAL: "STANDARD BUILDING CODE"
7. BUILDING OFFICIAL AND CODE ADMINISTRATORS INTERNATIONAL: "BOCA NATIONAL BUILDING CODE"

SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE DEALER/CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT.

SAFETY COMMITMENT

MESCO METAL BUILDINGS HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE SAFELY ERECTED. HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF MESCO METAL BUILDINGS.

IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP PRIORITY OF ANY JOB SITE.

LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE WORKER SAFETY.

MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES.

DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS, RUBBER SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, ARE RECOMMENDED.

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0		ISSUED FOR ERECTOR INSTALLATION			

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MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: ABBY GREEN,INC
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: S2 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 025686 YUTHACHAI SUVUNRUNGSI
JUN 28 2010

RECEIPT AND CARE OF MATERIALS

RG0101-0

CARE OF MATERIALS, PG.1

INTRODUCTION

This article has been prepared as a guideline for Buyer or Buyer's Receiving Agent who is responsible for receipt and care of materials manufactured by the Manufacturer.

The care taken in receipt and handling of materials directly affects the appearance, performance, and life of the completed building. This article identifies the various Shipping Documents received with each shipment of materials and presents Manufacturer's standard procedures in packaging and marking of materials. Cautions are given for the unloading of materials and procedures for receipt of materials are discussed. Finally, cautions for the job site storage of materials are given.

SHIPPING DOCUMENTS

A complete set of Shipping Documents are provided by Manufacturer with each shipment of materials. The Shipping Documents include a Bill of Lading, a Shipping Manifest, and a Shipping list.

The Bill of Lading shows the general classification on weight of materials shipped. Essentially the Bill of Lading serves three functions: (1) receipt; (2) contract of shipment; and (3) the document of title.

The Shipping Manifest shows the quantity of individual parts and packages included in a shipment. The Shipping manifest is designed to permit the receiving agent to quickly and accurately receive a shipment of materials.

The Shipping List is a detail listing of all materials included in an order showing the total quantity, part number, and description of materials. A copy of the Shipping List marked to show the materials included in the package is provided with each shipment.

PACKAGING AND MARKING

Materials for the Building System are packaged for ease in loading, unloading and to prevent loss of materials during transit. Standard packaging as outlined below is suitable for transportation by Manufacturer truck, and any other packaging requirements must be specified by Buyer on the Contract Documents.

Panel is wrapped in waterproof paper in packages of a maximum weight of 3500 pounds. Panel packages are separated by configuration and color and loaded separated by configuration. A waste or cover panel is provided only when other materials are loaded on top of a panel package. Ridge caps are banded in packages with approximately 50 per package. Each panel package is marked with the package number and the order number.

Trim is packaged in a waterproof carton, except when a small amount of trim is required, in which case the trim is wrapped in waterproof paper. Like pieces of trim are nested together and marked with the part number. Each trim package is marked with a package number and the order number.

Heavy structural framing, such as frame columns, frame rafters and wind columns are not packaged. Each item is marked with the part number and the order number.

Press-brake structural items, such as bearing end frame rafters, endwall columms, etc... are blocked and banded in groups of a maximum weight of 3500 pounds. Each item is marked with the part number and each group is marked with the package number and order number.

Rollform structural items, such as purlins, girts, etc... are blocked and banded in packages of a maximum weight of 3500 pounds. Each item is marked with the part number and each package is marked with the package number and order number.

Light structural items, such as rod bracing, angle bracing, base, rake, and corner angles are wired in groups of like items. Each group of like items is marked with the part number and the group is marked with the package number and order number.

Clips, when identical, are wired together, and all clips are banded together as a group. Flange braces are wired together by size. Each group of like items is marked with the part number. The group of clips and flange braces is marked with the package number and order number.

Fasteners, such as connection bolts, panel fasteners, nuts, etc... are bagged by item type, size, and color, and packaged in a can or drum. Each package is marked with the package number and order number.

Other items, such as sealants, panel closures, light hardware, etc... are boxed & the placed in the trim or fastener packages.

Accessory items, such as vents, doors, windows, etc... are prepackaged in boxes, cartons, crates, or paper wrapped by item, except roll-up doors, which are loaded with rubber spacers.

UNLOADING

Buyer is responsible for unloading materials at the delivery address or jobsite. If shipment is by truck, delivery is made to the nearest safe and accessible point to the job site. It is essential that the Buyer or Buyer's Receiving Agent be prepared with adequate workmen and equipment to meet and unload the shipment promptly at the scheduled time of delivery.

Substantial damage can be done to the materials if careless methods or inadequate equipment is used during unloading. All materials must be lifted from the truck using cranes, forklifts, or hoists of the proper size and capacity. Materials must never be pushed or dropped from the truck. Trim and panel packages need careful handling to avoid damage to these materials, The use of metal slings or chains for lifting trim and panel packages must be avoided. Error in determining the correct balance points, or careless attempts to lift oversize loads can result in damage to items, such as frame members, light gauge roll formed members, or long panel packages. Structural beam flanges can be damaged from improper placement of chains during attempts to unload heavy lifts. Buyer is responsible for all claims, damages, or injuries related to the unloading of materials.

STORAGE

Substantial deterioration and damage can occur to materials during job site storage. Materials should always be assembled or erected promptly on receipt. If assembly must be delayed, materials must be stored under protection.

All materials must be located in areas that are free from standing water. Materials must always be placed well above ground and stacked in a manner that provides free drainage of rain water or condensed moisture. Frame columns, rafters, and other items must be turned so water does not stand inside enclosed portion of members.

Particular care must be exercised in the storage of nested panels (see "Building Panels" published by the Manufacturer for additional information).

Materials should be stored in fenced and/or locked areas to reduce loss by vandalism or theft. Buyer is responsible for all deterioration, damage, loss, or claims related to the job site storage of materials.

RECEIPT

It is the Buyer's responsibility to receive each shipment including verifying all quantities as shown on the Shipping Manifest and inspecting all pieces or packages for damage

Quantities are verified by marking the quantity received on the Shipping Manifest. All shortages of part or package must be noted by Buyer or Buyer's Receiving Agent on the Shipping Manifest .

Buyer must carefully inspect pieces or packages for damage as materials are received. Each damaged part or package must be listed and the damage described on the Shipping Manifest.

All nested or packaged materials must be carefully inspected and any materials that have become wet in transit must be immediately unpacked, unstacked, and dried by Buyer. Particular care must be exercised in receipt of nested panels (see "Building Panels" published by the Manufacturer for additional information).

When all materials have been received and Buyer has identified all shortages or damages on the Shipping Manifest, both Buyer or Buyer's Receiving Agent and the driver of the carrier must sign the Bill of Lading and Shipping Manifest.

All packages must be opened by Buyer and the quantity of each item or part must be verified against the quantity shown on the Shipping List within ten (10) days following receipt of the shipment.

All claims for shortages or damages are subject to the Terms and Conditions of the Contract Documents.

IMPORTANT NOTICE

This material has been coated with a shop-applied primer. The primer is intended to protect the manufactured framing parts for only a short period of exposure to ordinary atmospheric conditions. This primer DOES NOT provide uniformity of appearance, durability and/or corrosion resistance of a field applied finish coat of paint.

The manufacturer IS NOT responsible for deterioration of the shop-applied primer or corrosion that may result from exposure to atmospheric and environmental conditions, nor the compatibility of the primer to any field-applied coating. Minor abrasions to the primer caused by handling, loading, shipping, unloading and erection are unavoidable. Shop-primed steel which is stored in the field pending erection, should be kept free of the ground and positioned as to minimize water-holding pockets, dust, mud and other contamination of the primer film.

METAL BUILDING PANELS

INTRODUCTION

Metal building panels are high qualityconstruction materials. Proper care of these building components can enhance the appearance of a building and will assure full service life. Improper care or lack of maintenance will definitely produce the opposite results. In addition, cost of materials repair or replacement and delays in construction due to problems resulting from lack of care are very expensive. In those instances where lack of care and maintenance is evident, warranties will be voided. These recommendations are presented by the Manufacturer in an effort to help the Buyer protect and insure the appearance and performance of these materials.

TYPES OF FINISHES

GALVALUME

Galvalume is the trade name for a patented steel sheet & coil product having a coating of corrosion resistant aluminum-zinc alloy. The alloy mixture is balanced to obtain the coating that retains the corrosion resistance & heat reflectivity of aluminum & the sacrifical action of galvanized. The best properties of both aluminum & zinc are combined in this coating & offer added service life for building panels.

PRE-PAINTED

Using galvalume steel as a substrate, pre-painted steel is given an additional rust inhibitor primer coat. This primer coat further increases the corrosion resistance. It also permits the use of attractive finish coats formulated to give the best in color and gloss retention. These coatings are applied to the exterior surface of the panels and the wash coat, designed only for interior use, is applied on the opposite side.

Galvalume, and Pre-painted steel can give excellent service for many years if a few rules concerning their care and maintenance are observed. All of these finishes are equally subject to damage and corrosion when care is not provided.

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CARE OF MATERIALS, PG.2

PACKAGING

Packaging of Manufacturer panels is designed to protect materials and prevent material loss during transit. Standard packaging includes wrapping in waterproof paper in bundles not exceeding 3500 pounds in weight. Panel bundles are separated by configuration and and color and loaded separated by configuration.

Bundles are marked with Manufacturer's order number. An envelope marked "Packing List Enclosed" is attached to the bundle in an accessible place. This packing list shows the quantity, length, color and configuration of the panels in the bundles. Most importantly, recommendations for panel care are printed on each bundle.

On all shipments transported by Manufacturer truck or commercial carrier, a waste or extra panel is placed on the bundle top when other materials are to be loaded atop the bundle. Panels transported by motor freight (component shipment) are banded to 2x4 skids at the bottom with an extra panel banded to the top of the bundle.

DAMAGE DURING CONSTRUCTION

The quality of workmanship in steel erection, construction practices, and handling methods used during the construction of the metal building can significantly affect the appearance and performance of the building panels. Most panel damage during construction is the result of faulty methods or carelessness. A good example of faulty methods is found in the installation of panel fasteners. The practice of overdriving these fasteners is fairly common. Overdriven fasteners cause indentations or shallow pockets in the panel around the fastener head. Rain water or condensed moisture combined with atmospheric pollutants (principally sulfur dioxides) and dirt particles collect in these pockets. The combination of pollutants and water creates acid solutions that will cause corrosion damage to the panel and fastener. Rain may wash some pollutants away, but moisture in the form of high humidity can keep these areas wet and continue the problem. Overdriving the fastener also forces the sealing washer from under the fastener head creating a leak at this point. Proper torque adjustment of the screw gun or preferably the use of a depth gauge will eliminate the problem of overdriven fasteners.

It is extremely important that all drill shavings from the installation of panel fasteners and fillings from the saw cutting of panels be removed from the panel surface. Corrosion will occur in a matter of hours when these shavings or fillings are not removed and are allowed to come in contact with water or condensed moisture. When panels are pre-drilled or cut in the stack prior to erection all shavings or fillings must be cleaned from both sides of the panel. Many times these raw steel particles are embedded in the panel finish coat and must be removed with a bristle brush. To prevent corrosion of the panel by these particles it is imperative that the roof be swept clean at least daily and certainly at job completion. The final cleaning of the roof should be done prior to installing the gutter so that the shavings are not deposited into the gutter and left to corrode. Any other foreign objects or debris left by construction personnel should also be removed from the roof.

During the erection of the roof and the installation of equipment such as air conditioning units, etc... carelessness by personnel walking on the panels can cause damage. Workmen should step or walk in the broad flat areas of the panel and avoid stepping on the major corrugations.

Panel ends and edges can be bent by careless handling. If this damage is severe, the edges must be straightened prior to erection since the appearance and/or weather tightness of the panel could be affected. Dragging one panel across another can cut or abrade the coating causing unsightly marks on the panel surface. Attempts to erect panels during windy conditions should be avoided to prevent damage and for safety considerations.

Another cause of panel damage that is consistently overlooked by the builder is the practice of leaving dirt piled against the exterior wall panels at the foundation. This dirt may be wet or at least will contain some moisture. Mud may have been splashed onto the wall during construction. Corrosion damage may occur where this dirt or mud contacts the panels. In areas where lime stabilization of the soil is required, corrosion damage from the soil's content will be accelerated and most likely be severe. All dirt must be removed from the panel walls at the completion of the work.

Pre-painted panels may require touch-up if the coating has been damaged during handling or erection. The appearance of the bldg. may be affected if damaged spots or scratches are located in highly visible places such as around doors, windows, etc... If the damage is extensive then replacement of the entire panel should be considered. Touch-up paint can be supplied by Manufacturer on request. The paint should be applied with a small, soft bristle artist's style brush.

MAINTENANCE

All panel finishes offered by the Manufacturer have excellent durability and provide an attractive appearance for the metal building's exterior. Periodic maintenance will extend the service life and add to the aesthetic value of the building panels.

Only minor maintenance is required for any of the finishes mentioned in this article and consists principally of an occasional thorough cleansing of the panel surface. Air borne dirt and pollutants may cause apparent discoloration of the panels, especially affecting the appearance of the pre-painted steel. Simply washing the building with plain water using hoses or pressure sprays is adequate. If dirt deposits are heavy, a solution of water and mild laundry detergent (1/3 cup detergent per gallon of water) may be used with a long handled, soft bristle brush.

RECEIPT OF MATERIALS

It is the responsibility of the Buyer to receive all shipments from the Manufacturer verifying the quantities received and inspecting the shipment for damage. A document titled "Shipping Manifest" is provided with each load listing the number of bundles of panels as well as other building components. An initial count of the bundles should be made at the time of unloading and any shortage noted on the manifest. Concurrent with the bundle count a close inspection for panels damaged during shipment should be made. It is very important at this time to check for panels that may have become wet during transit. The Buyer is responsible for checking panels and for unstacking and drying these panels to prevent water staining. An exact count of the panels within the bundles verifying the quantities shown on the "Packing List" must be made by the Buyer within fourteen (14) days of the date of delivery and any claim for material shortage filed with the Manufacturer within this time limit.

Notation of shortages or damages made on the shipping manifest should be acknowledged with the signature of the Manufacturer's driver or the carrier's driver or agent.

Claims for shortages or damaged materials on shipments transported by the Manufacturer should be filed with the Manufacturer and claims on shipments transported by common carrier filed with the carrier.

UNLOADING, STACKING AND STORAGE

Proper unloading and stacking techniques can help prevent unnecessary damage, reduce handling time, and pay dividends in increased erection efficiency.

To assist in unloading panels from the truck, each bundle is loaded with the ends off-set. This allows the unloader to lift one end and insert a wooden block or dunnage under the bundle. Long panel bundles may require two (2) blocks. A forklift or bridle with web slings can now be used to lift the bundle. Under no circumstances should metal slings, chokers or chains be used as lifting devices. Damage to the panels will result from using such devices. Tag lines tied to the bundles and/or ground personnel can guide the materials safely into place. Dunnage should be placed on the ground or building floor spaced 8' on center to receive the bundles. Panel bundles should be stacked separate from the structural steel and in a dry area free from danger of water ponding. If possible, bundles should be stored under roof. For open storage, bundles must be placed on dunnage with one and elevated approximately 24 inches above ground or floor. The lower end should be approximately 4 inches above ground or floor. Dunnage should be placed under the center of long bundles to prevent sagging and water collection. Special attention must be given to keeping the low end out of contact with mud or water. The wrapping at the low end should be opened to allow ventilation and drainage. All bundles or unwrapped panels must be secured to avoid wind damage.

DAMAGE FROM CONDENSATION OR TRAPPED WATER

It is extremely important that the panels be checked at unloading, and then constantly monitored for evidence of trapped water or moisture condensation while awaiting erection. High humidity conditions with temperature cycling will cause condensation between the panels within the bundle. Condensation can occur frequently near the sea coast or other large bodies of water. In addition, panels can become wet by wind-driven rain if the wrappings are torn during handling. If jobsite covers are used, they should be tied away from the bundle at the corners to allow air circulation around the bundle. This will help prevent moisture evaporating from the ground or building floor from condensing on the panels. Plastic or other impermeable covers are not recommended. Immediate action is required if the panels are found to be wet from any cause. The bundles must be opened and each panel unstacked and thoroughly dried on both sides. Re-stacking the panels at a slight angle to each other to prevent nesting will allow air circulation and assist in keeping the panels dry. In severe conditions large fans can be used to circulate air between the unstacked panels and accelerate drying.

Damage to the panel coating occurs when panels become wet and are allowed to stay wet. Damage can occur to nested panels within 24-48 hours. This damage shows as corrosion and discoloration of the panel surface and is commonly called wet storage stain, zinc oxidation, or "white rust". All of the panel finishes discussed earlier in this article are subject to this damage and discoloration. Pre-painted steel is perhaps less vulnerable but by no means exempt. A softening of the paint film can occur with pre-painted steel under wet storage conditions and the durability of the panel finish substantially decreased. Galvanized and Galvalume panels react more quickly to surface oxidation since they lack the additional protection of paint. Zinc coated or Galvalume panels under normal exposure form a zinc or aluminum oxide film on their surface allowing a slow oxidation process called "weathering" to occur that inhibits further corrosion. In nested bundles constant contact of the panels with condensed or trapped water prevents this weathering process. Rapid oxidation of the zinc or zinc aluminum coating can now occur and may lead to "red rust" in a short time.

If discoloration or stains are minor, a household cleaner of the type used on porcelain sinks and bathtubs may be used with a bristle brush to remove the stains. Wire brushing or using abrasive materials should be avoided since scratching or removal of the coating could occur.

Damage from condensation or trapped water adversely affects the service life of metal building panels. Panels with significant damage should be replaced by the Buyer before erection.

RG0103-1

SCREW IDENTIFICATION

SELF-DRILLING STITCH SCREWS

THESE SCREWS DRILL & TAP IN ONE OPERATION

SOME OF THE COMMON USES FOR THESE SCREWS ARE PANEL TO PANEL SIDELAPS, TRIMS TO PANEL, TRIMS TO TRIMS AND AS THE GUTTER SUPPORT SCREW.

STITCH SCREWS
Part No. FS4Afc
fc = Finish Code
1/4 x 7/8" SELF-DRILLER WITH WASHER
STANDARD LIFE

STITCH SCREWS
Part No. FS4fc
fc = Finish Code
1/4 x 7/8" SELF-DRILLER WITH WASHER, EXTENDED LIFE
EXTENDED LIFE

SELF-DRILLING MEMBER SCREWS

THESE SCREWS DRILL & TAP IN ONE OPERATION

SOME OF THE COMMON USES FOR THESE SCREWS ARE PANEL TO FRAMING AT POLAR WHITE OR UNPAINTED, LINER AND INTERIOR PARTITION PANELS.

MEMBER SCREWS
Part No. FS17PW
Polar White
#12 x 7/8" SELF-DRILLER WITHOUT WASHER
STANDARD LIFE ONLY

MEMBER SCREWS
Part No. FS1APL
Corrosion Resistant
#12 x 7/8" SELF-DRILLER WITHOUT WASHER
STANDARD LIFE ONLY

SELF-DRILLING MEMBER SCREWS

THESE SCREWS DRILL & TAP IN ONE OPERATION

SOME OF THE COMMON USES FOR THESE SCREWS ARE PANEL TO FRAMING AT SOFFIT, FACIA, BACK PANEL AND MINI-WHSE EXTERIOR PANEL. ALSO AT PANEL TO FRAMING AT LINER AND INTERIOR PARTITION PANELS NOT POLAR WHITE OR UNPAINTED.

MEMBER SCREWS
Part No. FS17fc
fc = Finish Code
#12 x 7/8" SELF-DRILLER WITH WASHER
STANDARD LIFE ONLY

RECOMMENDED INSTALLATION TOOLS & BITS

1. RECOMMENDED TOOL – AN 1800 rpm MAXIMUM, 4 TO 6 AMP SCREW GUN EQUIPPED WITH A DEPTH LOCATING NOSE PIECE. USE ADEQUATE CORDAGE TO PREVENT VOLTAGE DROP AT TOOL.
2. RECOMMENDED SOCKETS AND BITS:
   SELF-DRILING SOCKETS – ONE PIECE 5/16" HEX
   SELF-TAPPING SOCKETS – ONE PIECE 3/8" HEX
   BITS – 1/8" PANEL TO PANEL: #10 (.195)
   BITS – 1/8" PANEL OR TRIM TO FRAMING: #8 (.199)
   BIT SIZE FOR 1/8" POP RIVET IS #30 (.1285)
   BIT SIZE FOR 5/32" POP RIVET IS #20 (.161)

RECOMMENDED INSTALLATION GUIDELINES

1. DO NOT USE IMPACT TOOLS OR NUT RUNNERS TO INSTALL SCREWS.
2. REPLACE AND CLEAN SOCKETS OFTEN. THIS WILL ALLOW SCREWS TO SEAT PROPERLY IN THE SOCKET AND PREVENT "WOBBLING".
3. ALWAYS DRIVE SCREW PERPENDICULAR TO THE WORK SURFACE. THIS WILL PREVENT SCREW "WALKING" OR PANEL/TRIM DAMAGE.
4. INCORRECT SCREW INSTALLATION CAN RESULT IN SCREW FAILURE, WATER LEAKS, AND/OR PREMATURE DETERIORATION OF PANEL/SCREW.
5. POP RIVETS REQUIRE CONNECTING PARTS TO BE PRE-DRILLED AND INSTALLED WITH AN ADEQUATE TOOL.

SELF-DRILLING MEMBER SCREWS

THESE SCREWS DRILL & TAP IN ONE OPERATION

SOME OF THE COMMON USES FOR THESE SCREWS ARE PANEL TO FRAMING AT EXTERIOR ROOF AND WALL PANEL WITH A MAXIMUM OF 4" OF BLANKET INSULATION.

MEMBER SCREWS
Part No. FS17Afc
fc = Finish Code
#12 x 1-1/4" SELF-DRILLER WITH WASHER
STANDARD LIFE

MEMBER SCREWS
Part No. FS3fc
fc = Finish Code
#12 x 1-1/4" SELF-DRILLER WITH WASHER, EXTENDED LIFE
EXTENDED LIFE

SELF-DRILLING MEMBER SCREWS

THESE SCREWS DRILL & TAP IN ONE OPERATION

SOME OF THE COMMON USES FOR THESE SCREWS ARE PANEL TO FRAMING AT EXTERIOR ROOF AND WALL PANEL WITH OVER 4" OF BLANKET INSULATION TO A MAXIMUM OF 6" OF BLANKET INSULATION.

MEMBER SCREWS
Part No. FS17Bfc
fc = Finish Code
#12 x 1-1/2" SELF-DRILLER WITH WASHER
STANDARD LIFE

MEMBER SCREWS
Part No. FS3Afc
fc = Finish Code
#12 x 1-1/2" SELF-DRILLER WITH WASHER, EXTENDED LIFE
EXTENDED LIFE

TRIM POP RIVET 1/8 X 3/16 GRIP

THESE POP RIVETS ARE USED AT TRIM ENDLAPS

Part No. FS14fc
fc = Finish Code

MEMBER POP RIVET 5/32 X 1/4"

THESE POP RIVETS ARE USED AT VALLEY AND PARAPET GUTTER ENDLAPS

Part No. FS14Afc
ALUMINUM

SELF-TAPPING MEMBER SCREWS

THESE SCREWS REQUIRE PRE-DRILLING A PILOT HOLE

THIS SCREW IS THE TRANSLUCENT ROOF PANEL TO FRAMING SCREW.

TRANSLUCENT PANEL SCREWS
Part No. S48CW-W
Cameo White
14 x 1" SELF-TAPPER WITH 1 1/8"ø SEALING WASHER

SELF-TAPPING MEMBER SCREWS

THESE SCREWS REQUIRE PRE-DRILLING A PILOT HOLE

SOME OF THE COMMON USES FOR THESE SCREWS ARE THRU PANEL TO FRAMING AT ROOF TO WALL TRANSITIONS AND AT HIGH SIDE EAVE TRIMS.

MEMBER SCREWS
Part No. FS18CPL
Corrosion Resistant
14 x 2" SELF-TAPPING WITH WASHER

RG0104-2

BOLT & NUT IDENTIFICATION

1/2" Diameter A307 HEX HEAD BOLT/NUT ASSEMBLIES

INSTALLATION OF THESE BOLTS IS TO BE "SNUG TIGHTENED"

SOME OF THE COMMON USES FOR THESE BOLT/NUT ASSEMBLIES ARE PURLIN, GIRT, EAVESTRUT AND FLANGE BRACE TO FRAME CONNECTIONS. ALSO END FRAME COLUMN TO END FRAME RAFTER CONNECTIONS.

Assembly No. BN410AA

Part No. B410A — 1/2" Dia. x 1-1/4" A307 Hex Head Bolt, Finish = Plated
Part No. N4A — 1/2" Dia. A563 Hex Nut, Finish = Plated

1/2" Diameter A307 FINNECK BOLT/NUT ASSEMBLIES

INSTALLATION OF THESE BOLTS IS TO BE "SNUG TIGHTENED"

SOME OF THE COMMON USES FOR THESE BOLT/NUT ASSEMBLIES ARE HEADER TO JAMB AND JAMB TO GIRT AND JAMB TO BASE CLIP CONNECTIONS.

Assembly No. BN410FA

Part No. B410F — 1/2" Dia. x 1-1/4" A307 Finneck Bolt, Finish = Plated
Part No. N4A — 1/2" Dia. A563 Hex Nut, Finish = Plated

7/8" Diameter A490 HEX HEAD BOLT/NUT ASSEMBLIES

INSTALLATION OF THESE BOLTS IS TO BE "TURN-OF-NUT"

THE MOST COMMON USE FOR THESE BOLTS IS THE MAIN FRAME COLUMN TO MAIN FRAME RAFTER CONNECTIONS.

Assembly No. BN722DD

Part No. B722D — 7/8" Dia. x 2 3/4" A490 Hex Head Bolt, Finish = Plain (Black)
Part No. N7D — 7/8" Dia. A563 Hex Nut, Finish = Plain (Black)

7/8" Daimeter A325 HEX HEAD BOLT/NUT ASSEMBLIES

THE MOST COMMON USE FOR THESE BOLTS IS THE MAIN FRAME COLUMN TO MAIN FRAME RAFTER CONNECTIONS.

Assembly No. BN720BB

Part No. B720B — 7/8" Dia. x 2 1/2" A325 Hex Head Bolt, Finish = Plated
Part No. N7B — 7/8" Dia. A563 Hex Nut, Finish = Plated

3/4" Daimeter A325 HEX HEAD BOLT/NUT ASSEMBLIES

THE MOST COMMON USE FOR THESE BOLTS IS THE MAIN FRAME COLUMN TO MAIN FRAME RAFTER CONNECTIONS.

Assembly No. BN616BB

Part No. B616B — 3/4" Dia. x 2" A325 Hex Head Bolt, Finish = Plated
Part No. N6B — 3/4" Dia. A563 Hex Nut, Finish = Plated

Assembly No. BN622BB

Part No. B622B — 3/4" Dia. x 2 3/4" A325 Hex Head Bolt, Finish = Plated
Part No. N6B — 3/4" Dia. A563 Hex Nut, Finish = Plated

5/8" Daimeter A325 HEX HEAD BOLT/NUT ASSEMBLIES

THE MOST COMMON USE FOR THESE BOLTS IS THE MAIN FRAME COLUMN TO MAIN FRAME RAFTER CONNECTIONS.

Assembly No. BN510BB

Part No. B510B — 5/8" Dia. x 1 1/4" A325 Hex Head Bolt, Finish = Plated
Part No. N5B — 5/8" Dia. A563 Hex Nut, Finish = Plated

Assembly No. BN512BB

Part No. B512B — 5/8" Dia. x 1 1/2" A325 Hex Head Bolt, Finish = Plated
Part No. N5B — 5/8" Dia. A563 Hex Nut, Finish = Plated

Assembly No. BN516BB

Part No. B516B — 5/8" Dia. x 2 " A325 Hex Head Bolt, Finish = Plated
Part No. N5B — 5/8" Dia. A563 Hex Nut, Finish = Plated

1/2" Daimeter A325 HEX HEAD BOLT/NUT ASSEMBLIES

THE MOST COMMON USE FOR THESE BOLTS IS THE MAIN FRAME COLUMN TO MAIN FRAME RAFTER CONNECTIONS.

Assembly No. BN410BB

Part No. B410B — 1/2" Dia. x 1 1/4" A325 Hex Head Bolt, Finish = Plated
Part No. N4B — 1/2" Dia. A563 Hex Nut, Finish = Plated

Assembly No. BN416BB

Part No. B416B — 1/2" Dia. x 2" A325 Hex Head Bolt, Finish = Plated
Part No. N4B — 1/2" Dia. A563 Hex Nut, Finish = Plated

BOLT INSTALLATION INFORMATION

ALL A307 BOLTS WILL BE INSTALLED USING THE "SNUG TIGHTENED" BOLT TIGHTENING METHOD.
ALL A490 BOLTS WILL BE INSTALLED USING THE "TURN-OF-NUT" BOLT TIGHTENING METHOD.
MOST A325 BOLTS WILL BE INSTALLED USING THE "SNUG TIGHTENED" BOLT TIGHTENING METHOD.

SOME GENERAL EXCEPTIONS TO SNUG-TIGHTENED INSTALLATION FOR A325 BOLTS ARE:

1. BUILDINGS SUPPORTING 5 TON CRANE CAPACITIES OR GREATER.
2. BUILDINGS THAT ARE LOCATED IN D, E or F SEISMIC DESIGN CATEGORIES.
3. BUILDINGS THAT SUPPORT MACHINERY OR EQUIPMENT WHERE LIVE LOADS PRODUCE IMPACT OR REVERSAL OF STRESSES.

NOTE: REFER TO THE ERECTION DRAWINGS FOR CONDITION SPECIFIC CASES WHERE THE BOLT INSTALLATION METHOD WILL BE NOTED.

REFER TO REFERENCE DRAWING RG0109 FOR DEFINITIONS AND PROCEDURES OF "SNUG TIGHTENED" AND "TURN-OF-NUT" BOLT TIGHTENING METHODS.

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0		ISSUED FOR ERECTOR INSTALLATION			

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5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

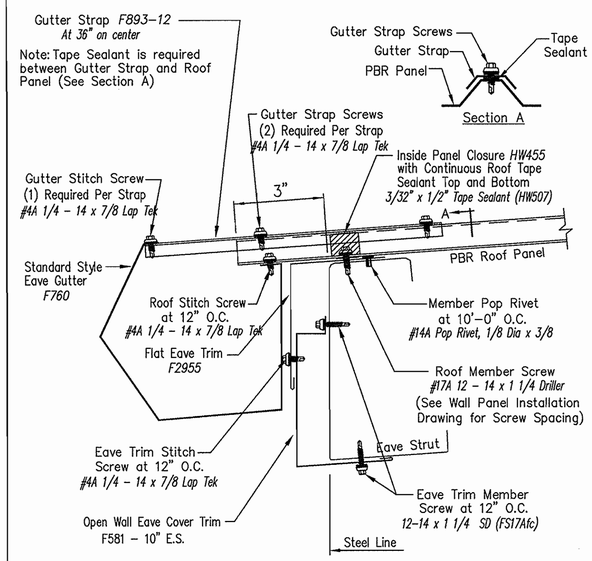
METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: ABBY GREEN,INC
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: S1 of 4

Drawing Status: "FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL ENGINEER SEAL 025686 YUTHACHAI SUVUNRUNGSI
JUN 28 2010

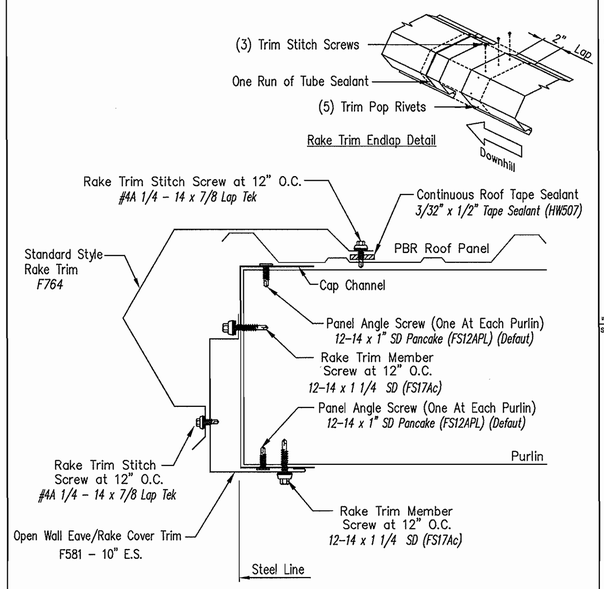
STANDARD



Low Eave Line Section at Steel Line
Fixed Eave-PBR Roof
Standard Gutter-Open To Remain Wall

DWGFS102 — A

STANDARD



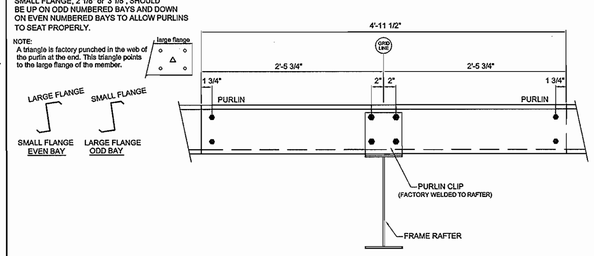
Rake Line Section at Steel Line
Fixed Rake Trim-PBR Roof Panel
Standard Rake-Open To Remain Wall

RAKS11112 — B

STANDARD

THE "Z" PURLINS ARE DESIGNED TO NEST. THE SMALL FLANGE, 2 1/8" or 3 1/8", SHOULD BE UP ON ODD NUMBERED BAYS AND DOWN ON EVEN NUMBERED BAYS TO ALLOW PURLINS TO SEAT PROPERLY.

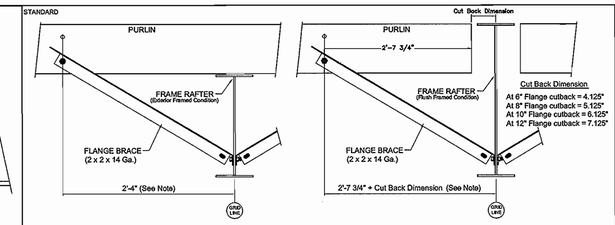
NOTE:
A triangle is factory punched in the web of the purlin at the end. This triangle points to the large flange of the member.



Purlin Lap 33 Using (4) A307 Bolts at Purlin Clip
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

HRLJ_033 — C

STANDARD



By Pass Purlin Condition
Flush Purlin Condition

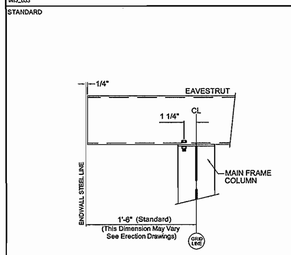
FLANGE BRACES MAY OR MAY NOT BE REQUIRED AT PURLINS .
IF REQUIRED THE FLANGE BRACE MAY BE ON ONE SIDE OR ON BOTH SIDES OF THE FRAME.
REFER TO THE ERECTION DRAWINGS FOR THE ACTUAL FLANGE BRACE REQUIREMENTS OF EACH PURLIN.

NOTE: Flange Brace Hole Location
2'-4" is the standard location for By Pass Condition.
2'-7 3/4" + Cut Back dimension is the standard location for Flush Condition.
See the erection drawings for alternate hole locations.

Web Bolted Flange Brace at Purlins
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

NCRFFB4 — D

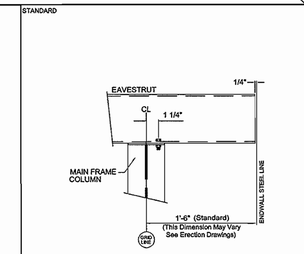
STANDARD



Eave Strut Connection at Flush Main Frame Endwall
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

FEFTLS10 — E

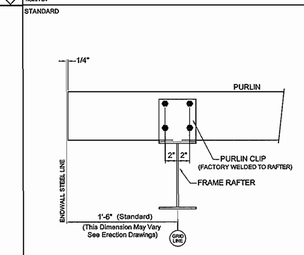
STANDARD



Eave Strut Connection at Flush Main Frame Endwall
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

FEFTRS10 — F

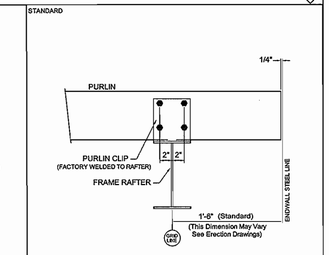
STANDARD



Purlin Connection at Flush Main Frame Endwall
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

FEPHLW00 — G

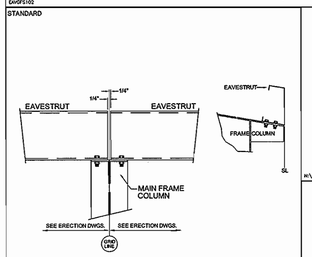
STANDARD



Purlin Connection at Flush Main Frame Endwall
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

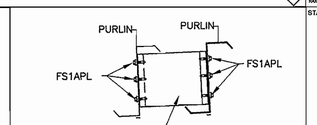
FEPHRW00 — H

STANDARD



Low Side Eave Strut to By Pass Main Frame Column
ALL BOLTS 1/2"Ø x 1 1/4" A307
(Unless Otherwise Noted)

WRC_S10 — I



INTERIOR STRUT SPACER DETAIL

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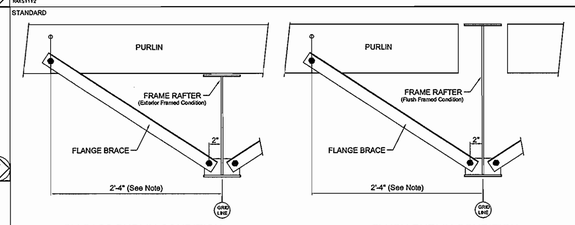
STANDARD



BY-PASS PURLIN CONDITION — FLUSH PURLIN CONDITION

CLIP BOLTED FLANGE BRACE AT PURLINS

FLANGE BRACES MAY OR MAY NOT BE REQUIRED AT PURLINS .
IF REQUIRED THE FLANGE BRACE MAY BE ON ONE SIDE OR ON BOTH SIDES OF THE FRAME.
REFER TO THE ERECTION DRAWINGS FOR THE ACTUAL FLANGE BRACE REQUIREMENTS OF EACH PURLIN.

NOTE: Flange Brace Hole Location
2'-4" is the standard location. See the erection drawings for alternate hole locations.

Flange Brace Bolts 1/2"Ø x 1 1/4" A307

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DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0	6-28-10	ISSUED FOR ERECTOR INSTALLATION	SML	KEB	

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Voice 214-687-9999 Fax 214-687-9737

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: Abby Green,Inc
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: D1 of 1

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER LUTHACHAI SUVUNRUNGSI
JUN 28 2010

Gap: 06-28-10 09:06:27

NOTE:
AT LOCATIONS SHOWN, INSTALL FLANGE BRACES @ NEAR SIDE ONLY ON FL.4, AND @ FAR SIDE ONLY ON FL.1

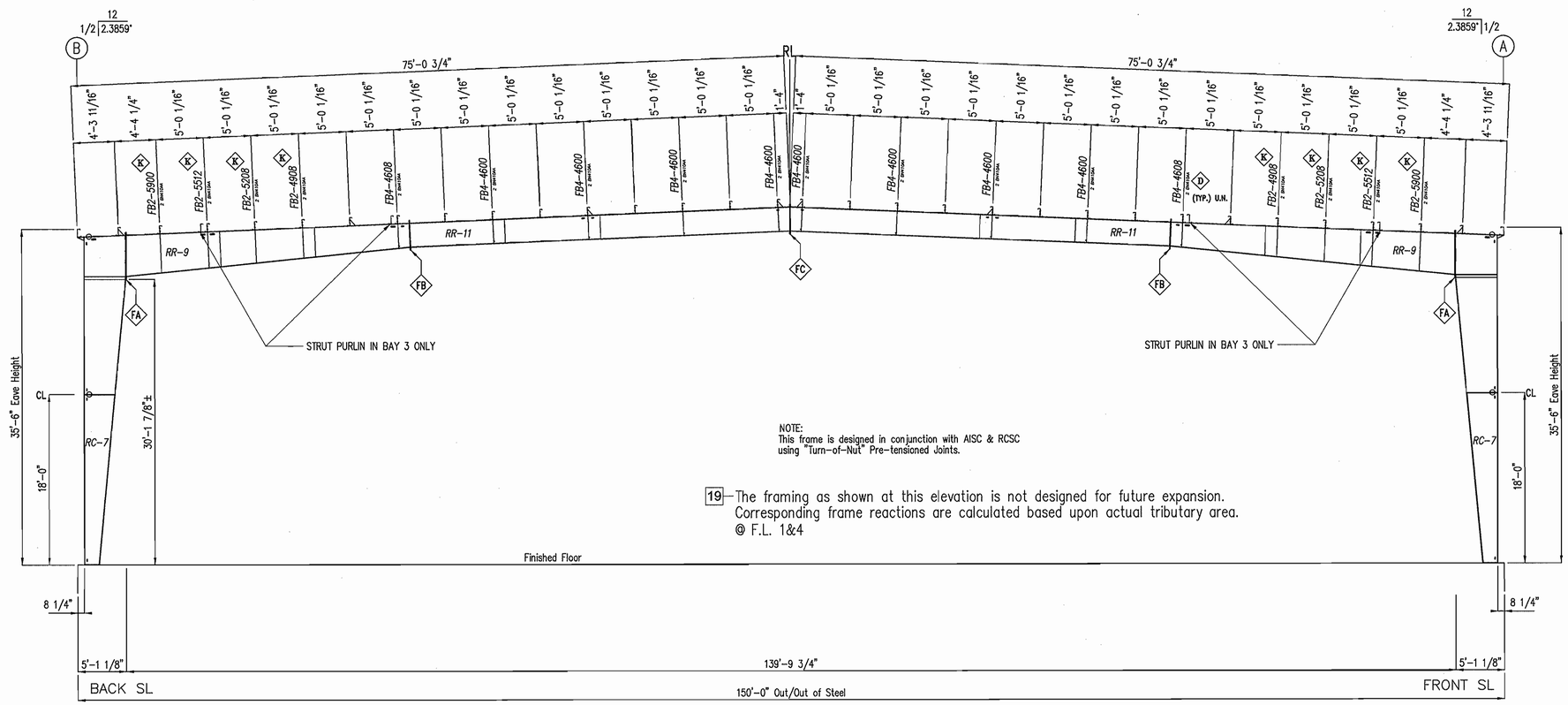
MEMBER DATA

Column data is listed from Base to Eave – Rafter data is listed from Eave to Ridge (High Side)

Mark	Outside Flange	Web	Inside Flange
RC-7	3/8 x 1-2 x 18-0 (50 KSI)	1/4 x 18 to 38 1/8 x 18-0	1/2 x 1-2 x 18-0 (50 KSI)
	3/8 x 1-2 (50 KSI)	1/4 x 38 1/8 to 52	1/2 x 1-2 (50 KSI)
RR-9	5/16 x 8	1/4 x 52 to 44 11/16	1/2 x 8
	1/4 x 8 x 10-0	1/4 x 44 11/16 to 37 5/16 x 10-0	3/8 x 8 x 10-0
	1/4 x 8 x 9-11 5/8	3/16 x 37 5/16 to 30 x 9-11 5/8	3/8 x 8 x 9-11 5/8
RR-11	3/8 x 6 x 9-11 5/8	3/16 x 30 x 9-11 5/8	5/16 x 6 x 9-11 5/8
	3/8 x 6 x 10-0 1/2	3/16 x 30 x 10-0 1/2	5/16 x 6 x 10-0 1/2
	1/2 x 6 x 20-0	8 GA x 30 x 20-0	3/8 x 6 x 20-0

GIRT & PURLIN DATA

Front Girts Depth : N/A	Back Girts Depth : N/A
Front Purlins Depth: 10	Back Purlins Depth: 10

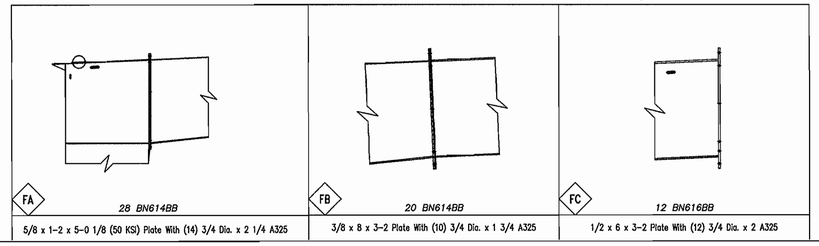


NOTE:
This frame is designed in conjunction with AISC & RCSC using "Turn-of-Nut" Pre-tensioned Joints.

19 – The framing as shown at this elevation is not designed for future expansion. Corresponding frame reactions are calculated based upon actual tributary area. @ F.L. 1&4

Frame Section: frame lines 1,4

AssyQty = 2



DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0	6-28-10	ISSUED FOR ERECTOR INSTALLATION	SML	KEB	KH

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Voice 214-687-9999 Fax 214-687-9737

Buyer: DWS STRUCTURES
Owner: Abby Green,Inc
Project Name: Abby Green
Job Site Location: Winston-Salem, NC

Job Number: 24-9706
Drawing Issue: 0
Sheet Number: E3 of 3

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER VITTHACHAI SUVIWANICHKUL
JUN 28 2010

Gap: 06-28-10 09:06:27

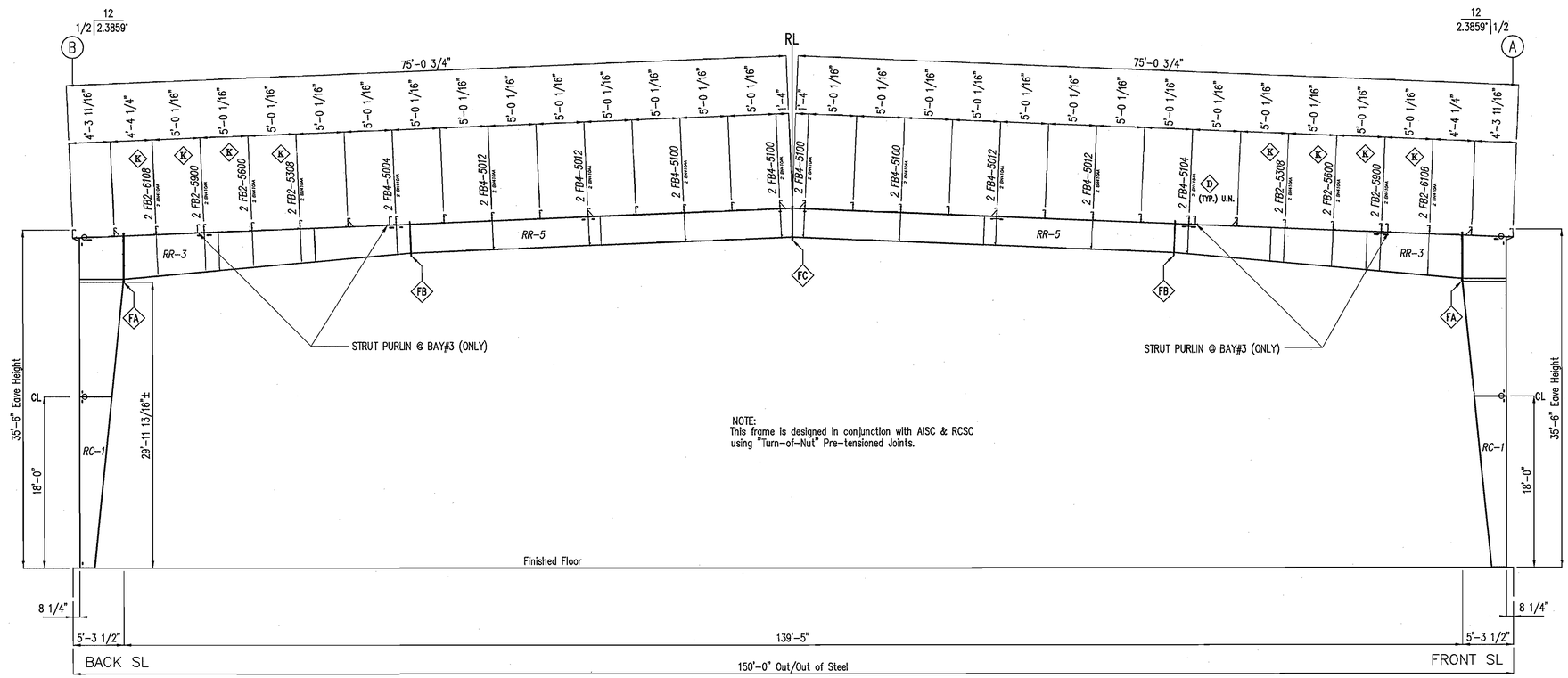
NOTE:
AT LOCATIONS SHOWN, INSTALL FLANGE BRACES THAT ARE AT BOTH SIDE ONLY

MEMBER DATA
Column data is listed from Base to Eave – Rafter data is listed from Eave to Ridge (High Side)

Mark	Outside Flange	Web	Inside Flange
RC-1	1/2 x 1-2 x 18-0 (50 KSI) 1/2 x 1-2 (50 KSI)	5/16 x 18 to 39 7/16 x 18-0 5/16 x 39 7/16 to 54	3/4 x 1-2 x 18-0 (50 KSI) 3/4 x 1-2 (50 KSI)
RR-3	1/2 x 8 3/8 x 8 x 10-0 3/8 x 8 x 9-11 5/8	5/16 x 54 to 48 5/16 x 48 to 42 x 10-0 5/16 x 42 to 36 x 9-11 5/8	5/8 x 8 1/2 x 8 x 10-0 1/2 x 8 x 9-11 5/8
RR-5	1/2 x 6 x 19-9 3/4 3/4 x 6 x 20-0	1/4 x 36 x 19-9 3/4 3/16 x 36 x 20-0	1/2 x 6 x 19-9 3/4 5/8 x 6 x 20-0

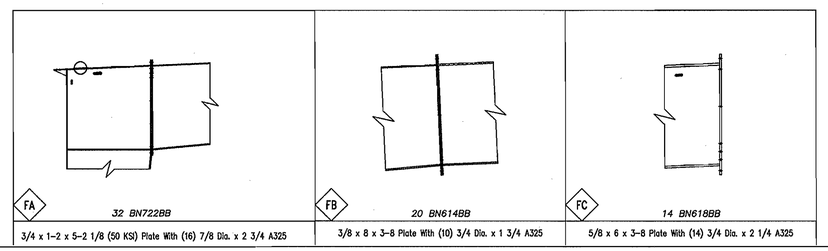
GIRT & PURLIN DATA

Front Girts Depth : N/A	Back Girts Depth : N/A
Front Purlins Depth: 10	Back Purlins Depth: 10



Frame Section: frame lines 2-3

AssyQty = 2



DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0	6-28-10	ISSUED FOR ERECTOR INSTALLATION	SML	KEB	KH

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Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: Abby Green,Inc
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: E2 of 3

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NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTTHACHAI SUVURNPUNGS
JUN 28 2010

P:\NCI2006 10-21-2005 SMLA\ 1/1

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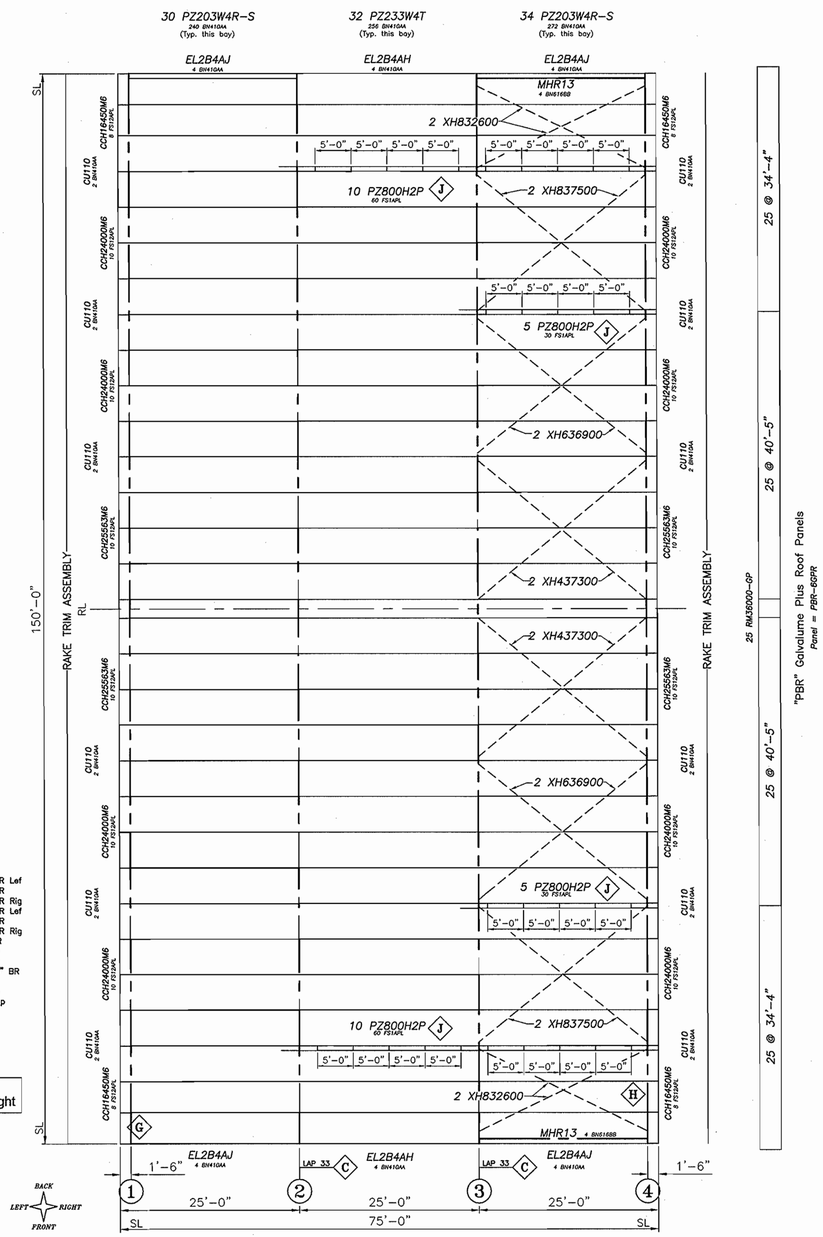


ROOF ASSEMBLY

NOTE:
ROOF FASTENERS AND SEALANT SHALL BE INSTALLED IN ACCORDANCE WITH REFERENCE DRAWING RR0101 FOR STANDARD ROOF CONSTRUCTION.

Roof Panel Fasteners
2888 FS17APL
1566 FS4APL
89 HW507

ADDITIONAL MAT'L (PER DEALER'S REQUEST)
4 F830-BG24000 O.S. CornerTrim PBR 20'-0"

B

RAKE TRIM ASSEMBLY

2	F764L-DS22000	Rake Trim Standard PBR Lef
4	F764-DS	Rake Trim Standard PBR
2	F764-DS22000	Rake Trim Standard PBR Rig
2	F764-DS22000	Rake Trim Standard PBR Lef
4	F764-DS	Rake Trim Standard PBR
2	F764R-DS22000	Rake Trim Standard PBR Rig
2	F160MS-ADS	Peak Box Standard PBR
612	FS4ADS	Rake Trim Stitch Screw
3666	ISL-R4	Rake Trim Sealant PBR
16	F581-ADS	OpenWallCover Strut 10" BR
426	FS17ADS	Trim Member Screw
48	FS4ADS	Rake Trim Stitch Screw
192	ISL-TWH	Tube Sealant @ End Lap
72	FS14DS	Trim Pop Rivets
140	FS14DS	Trim Pop Rivets

ALL WALLS ARE
Open to Remain Full Length, Full Height

Gap: 06-28-10 09:06:27

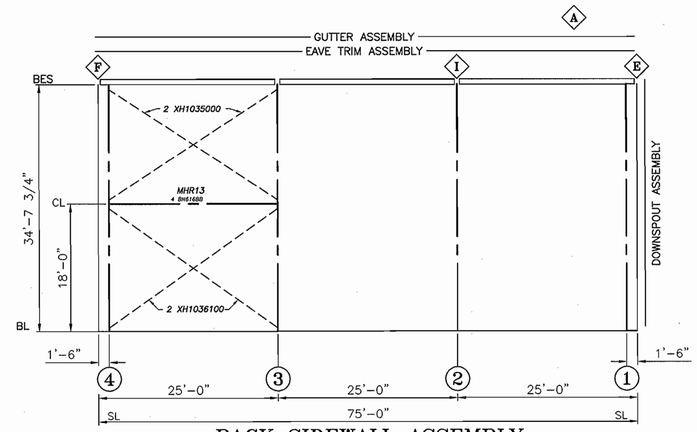
A

EAVE TRIM ASSEMBLY

1	F2955-ADS22400	Low Eave Trim Flat PBR Lef
2	F2955-ADS	Low Eave Trim Flat PBR
1	F2955-ADS22400	Low Eave Trim Flat PBR Rig
77	FS4ADS	Eave Trim Stitch Screw
8	FS14ADS	Member Pop Rivets
27	ISL-TWH	Tube Sealant @ End Lap
12	FS14DS	Trim Pop Rivets
25	HW455	I.S. Closure PBR
2033	ISL-R4	Tape Sealant at Eave PBR
2	F581-ADS	Low CoverTrim OTR 10" 20'-
2	F581-ADS22400	Low CoverTrim OTR 10" 18'-
16	ISL-TWH	Tube Sealant @ End Lap
7	FS14DS	Trim Pop Rivets
77	FS17ADS	Trim Member Screw

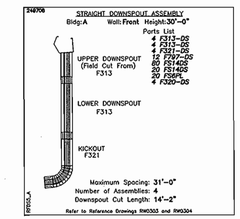
GUTTER ASSEMBLY

1	F760L-ADS22400	Eave Gutter Std. PBR Left
2	F760-ADS	Eave Gutter Std. PBR
1	F760R-ADS22400	Eave Gutter Std. PBR Right
77	FS4APL	Roof Stitch Screw
1	F761L-ADS	EndCap Standard PBR LH
1	F761R-ADS	EndCap Standard PBR RH
50	FS14DS	Trim Pop Rivets
168	ISL-TWH	Tube Sealant @ Splice
26	F893-GP	Gutter Strap PBR
52	FS4APL	Gutter Strap Screw
26	FS4ADS	Gutter Stitch Screw



BACK SIDEWALL ASSEMBLY

Open to Remain Full Length, Full Height



Gap: 06-28-10 09:06:27

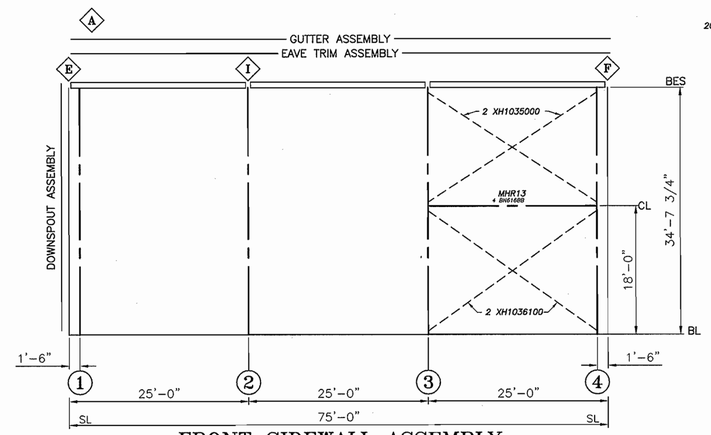
A

EAVE TRIM ASSEMBLY

1	F2955-ADS22400	Low Eave Trim Flat PBR Lef
2	F2955-ADS	Low Eave Trim Flat PBR
1	F2955-ADS22400	Low Eave Trim Flat PBR Rig
77	FS4ADS	Eave Trim Stitch Screw
8	FS14ADS	Member Pop Rivets
27	ISL-TWH	Tube Sealant @ End Lap
12	FS14DS	Trim Pop Rivets
25	HW455	I.S. Closure PBR
2033	ISL-R4	Tape Sealant at Eave PBR
2	F581-ADS	Low CoverTrim OTR 10" 20'-
2	F581-ADS22400	Low CoverTrim OTR 10" 18'-
16	ISL-TWH	Tube Sealant @ End Lap
7	FS14DS	Trim Pop Rivets
77	FS17ADS	Trim Member Screw

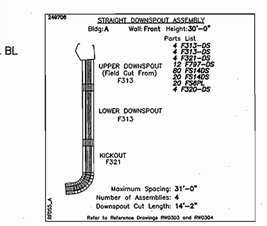
GUTTER ASSEMBLY

1	F760L-ADS22400	Eave Gutter Std. PBR Left
2	F760-ADS	Eave Gutter Std. PBR
1	F760R-ADS22400	Eave Gutter Std. PBR Right
77	FS4APL	Roof Stitch Screw
1	F761L-ADS	EndCap Standard PBR LH
1	F761R-ADS	EndCap Standard PBR RH
50	FS14DS	Trim Pop Rivets
168	ISL-TWH	Tube Sealant @ Splice
26	F893-GP	Gutter Strap PBR
52	FS4APL	Gutter Strap Screw
26	FS4ADS	Gutter Stitch Screw



FRONT SIDEWALL ASSEMBLY

Open to Remain Full Length, Full Height



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Mesco Building Solutions
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MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: Abby Green,Inc
Project Name: Abby Green
Job Site Location: Winston-Salem, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: E1 of 3

Drawing Status:
"FOR ERECTOR INSTALLATION"
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NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YITHACHAI SUVUNPONGSE
JUN 28 2010

SUPPLEMENTAL ANCHOR BOLT LAYOUT DRAWING INFORMATION

THIS DOCUMENT IS INTENDED TO BE USED IN CONJUNCTION WITH THE ANCHOR BOLT LAYOUT DRAWING.

USE ONLY THOSE DRAWINGS MARKED OR STAMPED FOR CONSTRUCTION BEFORE SETTING ANCHOR BOLTS FOR THE METAL BUILDING SYSTEM.

TABLE OF CONTENTS

General Information ---------- 2

Anchor Bolt Layout Drawing Information ---------- 2

Forming Isometric Drawing ---------- 3

Anchor Bolt Template Drawing ---------- 3

Anchor Bolt Pattern Drawing Definition ---------- 4

Column Base Detail Drawing ---------- 5

Panel Base Attachment Drawing ---------- 5

Suggested Notch Detail At Entry Doors ---------- 6

Suggested Notch Detail At Framed Openings -------- 7

These documents are intended to provide the DEALER information to properly interpret the anchor bolt layout drawing prepared by the MANUFACTURER, and convey this information to the parties responsible for the construction and design of the foundation supporting the Metal Building System.

Refer to the Manufacturer's order documents for the complete metal building specifications from which the anchor bolt layout drawings was prepared.

Anchor bolts are not included in the Metal Building Systems nor provided by the Manufacturer. All anchor bolts must be located accurately and in accordance with drawings. The Metal Building System base line must be level and not greater than 1'-0" above the finished grade.

ANCHOR BOLT LAYOUT DRAWING INFORMATION

The anchor bolt layout drawing shows :

The building size as specified on the order documents.

The location and size of the exterior panel notch of paneled walls.

The location of anchor bolts relative to the Metal Building System steel, frame and column lines.

The anchor bolt pattern number and it's required detail showing the dimensions of the pattern, bolt diameter, and bolt projections.

The quantity of anchor bolts.

The location of the column bases that are above and/or below the base line.

NOTE: All column bases (with the exception of fixed base columns) are assumed to be located at the base line, and the base line is at the foundation line.

The anchor bolt layout drawing does not show :

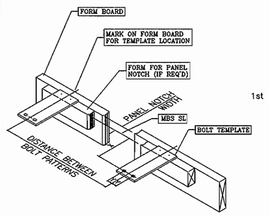
The design, length and type of anchor bolts.

The foundation design, enbedment, grout and or construction details.

The overall project foundation dimensions, size and location of curbs, ramps etc... .

The location of field located wall accessories and their related notch details.

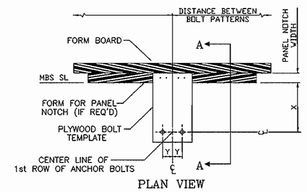
Collateral walls.



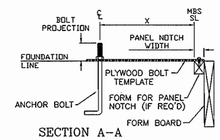
FORMING ISOMETRIC

SEE ANCHOR BOLT TEMPLATE FOR ADDITIONAL BOLT TEMPLATE INFORMATION.

FORM BOARDS WITH BOLT TEMPLATE



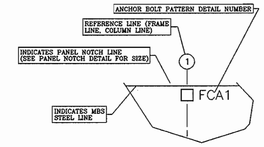
PLAN VIEW



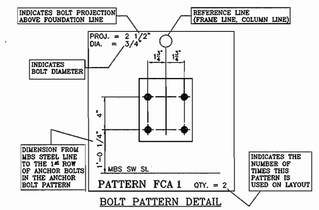
SECTION A-A

1. SEE ANCHOR BOLT LAYOUT DRAWING FOR DIMENSIONS. (X, Y, BOLT PROJECTION, ETC..)
2. GREASE ANCHOR BOLT THREADS FOR PROTECTION PRIOR TO POURING CONCRETE.
3. INSURE ALL FORM BOARDS ARE PROPERLY SET, LEVEL, SQUARED AND MARKED PRIOR TO PLACING BOLT TEMPLATES.

ANCHOR BOLT TEMPLATE

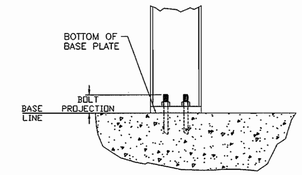


BOLT PATTERN IN PLAN VIEW



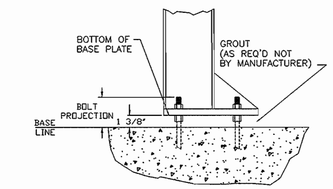
BOLT PATTERN DETAIL

BOLT PATTERN INTERPRETATION



STANDARD COLUMN BASE DETAIL

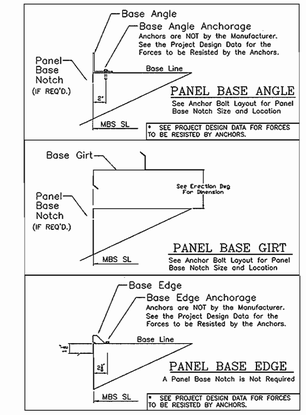
NOTE: All column bases (with the exception of fixed base columns) are assumed to be located at the base line, and the base line is at the foundation line.



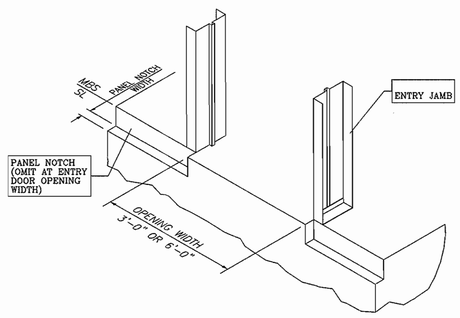
FIXED BASE COLUMNS REQUIRE DOUBLE NUTTED ANCHOR BOLTS. BOTTOM OF BASE PLATE SETS UP ABOVE THE FOUNDATION LINE 1 3/8".

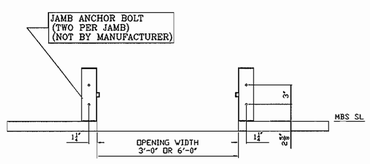
FIXED BASE COLUMN BASE DETAIL

THIS DETAIL IS TO ONLY BE USED AT FIXED BASE COLUMNS



SUGGESTED PANEL BASE ATTACHMENT



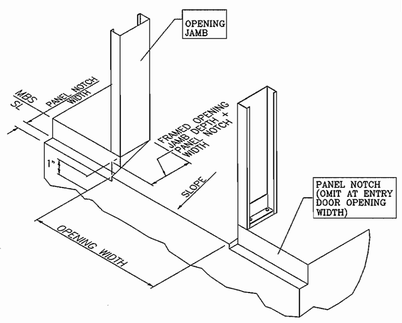


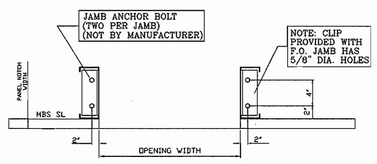
SUGGESTED PANEL NOTCH DETAIL AT PERSONNEL ENTRY DOORS

THIS IS A SUGGESTED DETAIL AND IS NOT INTENDED TO REFLECT ACTUAL FOUNDATION OR OTHER PROJECT DETAILS THAT MAY BE USED.

PANEL NOTCH DETAIL AT ENTRY DOORS

THIS DETAIL IS AT ENTRY DOORS FOR DOORS WITH 8 1/4" JAMBS





SUGGESTED PANEL NOTCH DETAIL AT FRAMED OPENINGS

REFER TO B.O.M. FOR FRAMED OPENING SIZE AND ANCHOR BOLT LAYOUT DRAWING FOR PANEL NOTCH WIDTH.

THIS IS A SUGGESTED DETAIL AND IS NOT INTENDED TO REFLECT ACTUAL FOUNDATION OR OTHER PROJECT DETAILS THAT MAY BE USED.

PANEL NOTCH DETAIL AT FRAMED OPENINGS

THIS DETAIL IS AT FRAMED OPENINGS WITH 8" JAMBS

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	OPER	DET	CKR	ENG
0	6/18/10	ISSUED FOR ERECTOR INSTALLATION	EBF	SPL		

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Voice 214-687-9999 Fax 214-687-9737

Buyer: DWS STRUCTURES
Owner: Abby Greeen, Inc.
Project Name: ABBY GREEN
Job Site Location: WINSTON-SALEM, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: A4 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION"
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NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTTACHAI SUVURUGSI
JUN 18 2010

M A I N B U I L D I N G D E S I G N S U M M A R Y R E P O R T

Customer Name: DWS Structures — Designed By : KXHU
Job Site: Forsyth, North Carolina — Checked By :_____

CS 150'0 x 75'0 x 35'6 0.5:12
Bays: 3@25'0 = 3@300

24-9706

Main Code Requirements Per:
2009 North Carolina
(Reference 2006 International Building Code)
Windforce-resistance System Per:
2005 ASCE 7
Seismic-resistance System Per:
2006 International Building Code
Cold-Formed Steel Design Spec:
2001 AISI Allowable Strength Design with 2004 Supplement
Other Steel Design Spec:
2005 AISC Allowable Strength Design, 13th edition

Building Use Category..... Normal
Roof Dead Load
Superimposed ..... 2.39 psf
Collateral ..... 3.00 psf

Roof Live Load ..... 20.00 psf with trib. reduction allowed

Snow Load
Ground Snow Load (Pg)..... 15.00 psf
Snow Load Importance Factor (I) 1.00
Flat Roof Snow Load (Pf) ..... 11.34 psf
Snow Exposure Factor (Ce) 0.90
Thermal Factor (Ct) 1.20
Wind Load
Basic Wind Speed ..... 90.00 mph
Wind Importance Factor (I) ... 1.00
Wind Exposure Category ..... B
Component Wind Load <= 10 sq. ft.
Zone 4 ..... 10.00 psf pressure -10.00 psf suction
Zone 5 ..... 0.00 psf pressure 0.00 psf suction
Seismic Load
Seismic Use Group I
Seismic Importance Factor (Ie) 1.00
Design Category C
Site Class ..... D -Stiff Soil
Ss ..... 0.231 g Sds..... 0.246 g
S1 ..... 0.086 g Sd1..... 0.138 g
Analysis Procedure ..... Equivalent Lateral Force

	Transverse**	Longitudinal**
Basic Force Resisting System		
Response Modification Coefficient (R)	3.00	3.00
Deflection Amplification Factor (Cd)	3.00	3.00
System Over-Strength Factor (Omega)	2.50	2.50
Entire Building Design Base Shear (V)	9.9k	9.9k

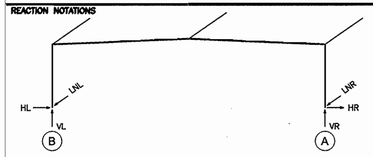
** - Transverse direction is parallel to the main frames;
Longitudinal direction is perpendicular to the main frames;

Rainfall Intensity, 5-minute duration
5-year recurrence (I1) ..... 6 in/hour
25-year recurrence (I2) ..... 9 in/hour

Mesco Building Solutions — FRAME DESCRIPTION: cs 150./35.5/24.25 20./90./ — USER NAME:KxHu JOB NAME:249706A — DATE: 6/18/10 FILE:m1_d-1.fra — PAGE: 1-2

SUPPORT REACTIONS FOR EACH LOAD GROUP — FRAME ID #01 LOCATION:frame lines 2-3 — TIME:08:33:46
NOTES:(1) All reactions are in kips and kip-ft.
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



AT FRAME LINES 2&3

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	14.0X19.5X0.625			14.0X19.5X0.625		
ANC. RODS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	6.9	12.0	0.0	-6.9	12.0	0.0
LL	16.6	21.8	0.0	-16.6	21.8	0.0
COLL	4.2	5.5	0.0	-4.2	5.5	0.0
SNOW	15.7	20.6	0.0	-15.7	20.6	0.0
RBDWEQ	-0.1	4.8	0.0	0.1	4.8	0.0
EQ	-1.5	-0.7	0.0	-1.5	0.7	0.0
RBUPEQ	0.1	-4.8	-3.4	-0.1	-4.8	-3.4
WL1	10.5	19.2	0.0	-12.1	10.6	0.0
WL2	-9.1	-16.9	0.0	9.0	-6.9	0.0
WL3	-13.5	-13.5	0.0	12.1	-20.2	0.0
WL4	-13.1	-19.4	0.0	12.6	-14.3	0.0
WL5	12.1	10.6	0.0	-10.5	19.2	0.0
WL6	-9.0	-6.9	0.0	9.1	-16.9	0.0
WL7	-12.1	-20.2	0.0	13.5	-13.5	0.0
WL8	-12.6	-14.3	0.0	13.1	-19.4	0.0
LWL1	-11.5	-15.4	0.0	11.5	-15.4	0.0
RBUPLW	0.5	-24.2	-17.1	-0.5	-24.2	-17.1
LWL2	12.0	15.4	0.0	-12.0	15.4	0.0
LWL3	-4.2	-5.8	0.0	4.2	-5.8	0.0
LWL4	4.6	5.8	0.0	-4.6	5.8	0.0

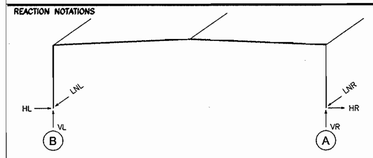
LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LL : Roof Live Load
COLL : Roof Collateral Load
SNOW : Roof Snow Load
RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic
EQ : Lateral Seismic Load [parallel to plane of frame]
RBUPEQ : Upward Acting Rod Brace Load from Longit. Seismic
WL1 : Lateral Primary Wind Load
WL2 : Lateral Primary Wind Load
WL3 : Lateral Primary Wind Load
WL4 : Lateral Primary Wind Load
WL5 : Lateral Primary Wind Load
WL6 : Lateral Primary Wind Load
WL7 : Lateral Primary Wind Load
WL8 : Lateral Primary Wind Load
LWL1 : Longitudinal Primary Wind Load
RBUPLW : Upward Acting Rod Brace Load from Longitud. Wind
LWL2 : Longitudinal Primary Wind Load
LWL3 : Longitudinal Primary Wind Load
LWL4 : Longitudinal Primary Wind Load

Mesco Building Solutions — FRAME DESCRIPTION: cs 150./35.5/24.25 20./90./ — USER NAME:KxHu JOB NAME:249706A — DATE: 6/18/10 FILE:m1_d-1.fra — PAGE: 1-3

SUPPORT REACTIONS FOR EACH LOAD GROUP — FRAME ID #01 LOCATION:frame lines 2-3 — TIME:08:33:46
NOTES:(1) All reactions are in kips and kip-ft.
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



AT FRAME LINES 2&3

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	14.0X19.5X0.625			14.0X19.5X0.625		
ANC. RODS	(4)-1			(4)-1		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
LWL5	-17.4	-23.2	0.0	17.4	-23.2	0.0
LWL6	7.6	9.6	0.0	-7.6	9.6	0.0
LWL7	-8.6	-11.6	0.0	8.6	-11.6	0.0
LWL8	4.6	5.8	0.0	-4.6	5.8	0.0
RS	14.0	12.3	0.0	-14.0	21.2	0.0
LS	14.0	21.2	0.0	-14.0	12.3	0.0
RBDWLW	-0.4	24.2	0.0	0.4	24.2	0.0

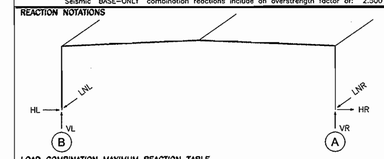
LOAD GROUP DESCRIPTION

LWL5 : Longitudinal Primary Wind Load
LWL6 : Longitudinal Primary Wind Load
LWL7 : Longitudinal Primary Wind Load
LWL8 : Longitudinal Primary Wind Load
RS : Unbalanced Right Roof Snow Load
LS : Unbalanced Left Roof Snow Load
RBDWLW : Downward Acting Rod Brace Load from Longit. Wind

Mesco Building Solutions — FRAME DESCRIPTION: cs 150./35.5/24.25 20./90./ — USER NAME:KxHu JOB NAME:249706A — DATE: 6/18/10 FILE:m1_d-1.fra — PAGE: 1-4

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS — FRAME ID #01 LOCATION:frame lines 2-3 — TIME:08:33:46
NOTES:(1) All reactions are in kips and kip-ft.
(2) These reactions are from loads determined from the applicable code for ASD design. Seismic loads are limit state and include magnification factors when so required by the seismic provisions of the applicable code for ASD design. It is the responsibility of the foundation designer to apply the load factors and load combinations appropriate for the concrete foundation design.
(3) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



AT FRAME LINES 2&3

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	14.0X19.5X0.625			14.0X19.5X0.625		
ANC. RODS	(4)-1			(4)-1		
LOAD COMB	HL	VL	LNL	HR	VR	LNR
GRAVITY LOAD COMBINATION						
1	28.0	39.4	0.0	-27.5	39.1	0.0
19	4.0	3.4	-2.4	-3.9	3.4	-2.4
2	27.5	39.1	0.0	-28.0	39.4	0.0
WIND LOAD COMBINATION						
97	33.0	41.9	0.0	-31.1	48.0	0.0
58	-12.9	-40.2	-17.1	12.7	-40.2	-17.1
159	32.6	63.7	0.0	-31.8	63.4	0.0
49	-6.9	-32.5	-17.1	7.0	-32.5	-17.1
57	-12.7	-40.2	-17.1	12.9	-40.2	-17.1
90	31.1	48.0	0.0	-33.0	41.9	0.0
160	31.8	63.4	0.0	-32.6	63.7	0.0
TRANSVERSE EARTHQUAKE LOAD COMBINATION						
17	24.9	34.7	0.0	-22.8	33.8	0.0
16	22.8	33.8	0.0	-24.9	34.7	0.0

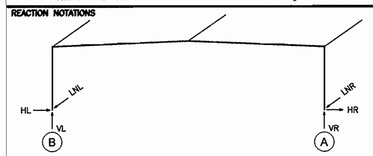
LOAD COMBINATION DESCRIPTION

1 : DL +LL +COLL
2 : DL +LL +COLL
16 : 1.0259DL +0.75LL +1.0259COLL +0.525EQ
17 : 1.0259DL +0.75LL +1.0259COLL -0.525EQ
19 : 0.5655DL +0.76RUPEQ
49 : 0.6DL +LWL1 +RBUPLW
57 : 0.6DL +LWL5 +RBUPLW
58 : 0.6DL +LWL5 +RBUPLW
90 : DL +0.75LL +COLL +0.75WL1
97 : DL +0.75LL +COLL +0.75WL5
159 : DL +0.75LL +COLL +0.75LWL2 +0.75RBDWLW
160 : DL +0.75LL +COLL +0.75LWL2 +0.75RBDWLW

Mesco Building Solutions — FRAME DESCRIPTION: cs 150./35.5/13.25 20./90./ — USER NAME:KxHu JOB NAME:249706A — DATE: 6/18/10 FILE:m2_d-1.fra — PAGE: 2-2

SUPPORT REACTIONS FOR EACH LOAD GROUP — FRAME ID #02 LOCATION:frame lines 1,4 — TIME:08:39:48
NOTES:(1) All reactions are in kips and kip-ft.
(2) The seismic overstrength factor (Omega) is not included in the "LEQ" Load Group reactions.
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



AT FRAME LINES 1&4

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	14.0X19.0X0.375			14.0X19.0X0.375		
ANC. RODS	(6)-3/4			(6)-3/4		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
DL	4.4	7.7	0.0	-4.4	7.7	0.0
LL	9.4	11.9	0.0	-9.4	11.9	0.0
COLL	2.3	3.0	0.0	-2.3	3.0	0.0
SNOW	8.9	11.3	0.0	-8.9	11.3	0.0
RBDWEQ	-0.1	4.8	0.0	0.1	4.8	0.0
EQ	-0.9	-0.4	0.0	-0.9	0.4	0.0
RBUPEQ	0.1	-4.8	-3.4	-0.1	-4.8	-3.4
WL1	5.7	10.4	0.0	-7.0	5.8	0.0
WL2	-5.3	-9.3	0.0	4.9	-3.7	0.0
WL3	-7.8	-7.5	0.0	6.6	-11.0	0.0
WL4	-7.5	-10.6	0.0	6.9	-7.8	0.0
WL5	7.0	5.8	0.0	-5.7	10.4	0.0
WL6	-4.9	-3.7	0.0	5.3	-9.3	0.0
WL7	-6.6	-11.0	0.0	7.8	-7.5	0.0
WL8	-6.9	-7.8	0.0	7.5	-10.6	0.0
LWL1	-6.4	-8.4	0.0	6.4	-8.4	0.0
RBUPLW	0.4	-24.2	-17.1	-0.4	-24.2	-17.1
LWL2	6.8	8.4	0.0	-6.8	8.4	0.0
LWL3	-2.3	-3.2	0.0	2.3	-3.2	0.0
LWL4	2.7	3.2	0.0	-2.7	3.2	0.0

LOAD GROUP DESCRIPTION

DL : Roof Dead Load
LL : Roof Live Load
COLL : Roof Collateral Load
SNOW : Roof Snow Load
RBDWEQ : Downward Acting Rod Brace Load from Long. Seismic
EQ : Lateral Seismic Load [parallel to plane of frame]
RBUPEQ : Upward Acting Rod Brace Load from Longit. Seismic
WL1 : Lateral Primary Wind Load
WL2 : Lateral Primary Wind Load
WL3 : Lateral Primary Wind Load
WL4 : Lateral Primary Wind Load
WL5 : Lateral Primary Wind Load
WL6 : Lateral Primary Wind Load
WL7 : Lateral Primary Wind Load
WL8 : Lateral Primary Wind Load
LWL1 : Longitudinal Primary Wind Load
RBUPLW : Upward Acting Rod Brace Load from Longitud. Wind
LWL2 : Longitudinal Primary Wind Load
LWL3 : Longitudinal Primary Wind Load
LWL4 : Longitudinal Primary Wind Load

Mesco Building Solutions — FRAME DESCRIPTION: cs 150./35.5/13.25 20./90./ — USER NAME:KxHu JOB NAME:249706A — DATE: 6/18/10 FILE:m2_d-1.fra — PAGE: 2-3

SUPPORT REACTIONS FOR EACH LOAD GROUP — FRAME ID #02 LOCATION:frame lines 1,4 — TIME:08:39:48
NOTES:(1) All reactions are in kips and kip-ft.
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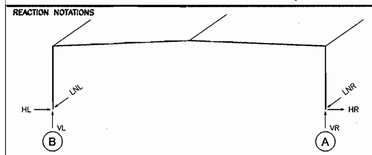
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



AT FRAME LINES 1&4

LOAD GROUP REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	14.0X19.0X0.375			14.0X19.0X0.375		
ANC. RODS	(6)-3/4			(6)-3/4		
LOAD GROUP	HL	VL	LNL	HR	VR	LNR
LWL5	-9.7	-12.7	0.0	9.7	-12.7	0.0
LWL6	4.4	5.3	0.0	-4.4	5.3	0.0
LWL7	-4.8	-6.3	0.0	4.8	-6.3	0.0
LWL8	2.7	3.2	0.0	-2.7	3.2	0.0
RS	7.9	6.7	0.0	-7.9	11.6	0.0
LS	7.9	11.6	0.0	-7.9	6.7	0.0
RBDWLW	-0.4	24.2	0.0	0.4	24.2	0.0

LOAD GROUP DESCRIPTION

LWL5 : Longitudinal Primary Wind Load
LWL6 : Longitudinal Primary Wind Load
LWL7 : Longitudinal Primary Wind Load
LWL8 : Longitudinal Primary Wind Load
RS : Unbalanced Right Roof Snow Load
LS : Unbalanced Left Roof Snow Load
RBDWLW : Downward Acting Rod Brace Load from Longit. Wind

Mesco Building Solutions — FRAME DESCRIPTION: cs 150./35.5/13.25 20./90./ — USER NAME:KxHu JOB NAME:249706A — DATE: 6/18/10 FILE:m2_d-1.fra — PAGE: 2-4

MAX. SUPPORT REACTIONS FOR LOAD COMBINATIONS — FRAME ID #02 LOCATION:frame lines 1,4 — TIME:08:39:48
NOTES:(1) All reactions are in kips and kip-ft.
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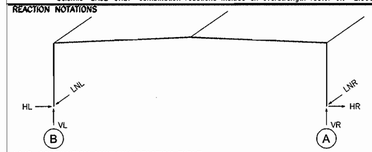
Seismic "BASE-ONLY" combination reactions include an overstrength factor of: 2.500

REACTION NOTATIONS



AT FRAME LINES 1&4

LOAD COMBINATION MAXIMUM REACTION TABLE

COLUMN	LEFT COLUMN			RIGHT COLUMN		
BASE PLATE	14.0X19.0X0.375			14.0X19.0X0.375		
ANC. RODS	(6)-3/4			(6)-3/4		
LOAD COMB	HL	VL	LNL	HR	VR	LNR
GRAVITY LOAD COMBINATION						
1	16.2	22.7	0.0	-16.0	22.6	0.0
19	2.6	1.0	-2.4	-2.5	1.0	-2.4
2	16.0	22.6	0.0	-16.2	22.7	0.0
WIND LOAD COMBINATION						
97	19.2	24.1	0.0	-17.9	27.4	0.0
58	-8.8	-32.3	-17.1	8.6	-32.2	-17.1
159	18.9	44.3	0.0	-18.3	44.0	0.0
49	-3.3	-28.0	-17.1	3.4	-28.1	-17.1
57	-6.6	-32.2	-17.1	8.8	-32.3	-17.1
90	17.9	27.4	0.0	-19.2	24.1	0.0
160	18.3	44.0	0.0	-18.9	44.3	0.0
TRANSVERSE EARTHQUAKE LOAD COMBINATION						
17	14.5	20.2	0.0	-13.3	19.6	0.0
16	13.3	19.6	0.0	-14.5	20.2	0.0

LOAD COMBINATION DESCRIPTION

1 : DL +LL +COLL
2 : DL +LL +COLL
16 : 1.0259DL +0.75LL +1.0259COLL +0.525EQ
17 : 1.0259DL +0.75LL +1.0259COLL -0.525EQ
19 : 0.5655DL +0.76RUPEQ
49 : 0.6DL +LWL1 +RBUPLW
57 : 0.6DL +LWL5 +RBUPLW
58 : 0.6DL +LWL5 +RBUPLW
90 : DL +0.75LL +COLL +0.75WL1
97 : DL +0.75LL +COLL +0.75WL5
159 : DL +0.75LL +COLL +0.75LWL2 +0.75RBDWLW
160 : DL +0.75LL +COLL +0.75LWL2 +0.75RBDWLW

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	OPER	DET	CKR	ENG
0	6/18/10	ISSUED FOR ERECTOR INSTALLATION	EBF	SML		KH

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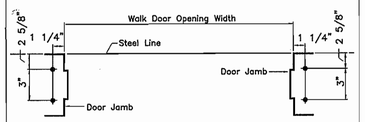
The engineer whose seal appears hereon is employed by Mesco Building Solutions or another division or affiliate of NCI Group, Inc. and is not the engineer of record for this project.

MESCO
Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737
MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: Abby Greeen, Inc.
Project Name: ABBY GREEN
Job Site Location: WINSTON-SALEM, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: A3 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTACHAI SUVUNPUNGSI
JUN 18 2010

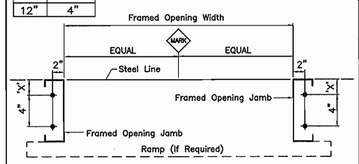


WALK DOOR ANCHOR BOLT LAYOUT

(For Walk Doors with 8 1/4" Deep Jambs)
1/2" Expansion Bolts

This detail is provided for informational purposes only and is not to be used with pre-assemblied door systems. Walk doors, when required, are FIELD LOCATED and are NOT located on the Anchor Bolt Layout

Jamb Depth	Dim. 'X'
8"	2"
10"	3"
12"	4"



FRAMED OPENING ANCHOR BOLT LAYOUT

Maximum 1/2" Anchorage Device with a 1 1/2" Projection

This detail is provided for informational purposes only. FIELD LOCATED Framed Openings, when required, are NOT located on the Anchor Bolt Layout. For SHOP LOCATED Framed Openings, when required, refer to the schedule below and the Anchor Bolt Layout.

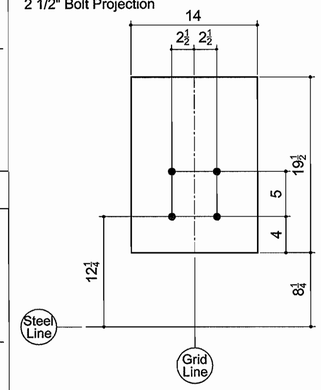
SHOP LOCATED FRAMED OPENING SCHEDULE

Mark	Qty.	Jamb Depth	Opening Width	Mark	Qty.	Jamb Depth	Opening Width
◇				◇			
◇				◇			
◇				◇			
◇				◇			

ATTENTION FOUNDATION DESIGNER

The manufacturer has determined the required anchor bolt diameter assuming full allowable combined shear and tension can be developed in the bolt in accordance with Table J3.3 of the AISC 9th Edition of the Manual of Steel Construction, Allowable Stress Design assuming a threaded part made of ASTM-A36 Steel. It is the responsibility of the foundation designer to design an anchorage system using bolts of the stated diameter that develop sufficient strength to resist the reactions given on the anchor bolt drawings. This may require the addition of embedded angles, plates or other items to allow the bolt group to develop a single resistance cone.

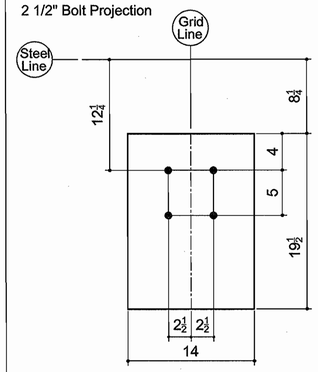
1" Diameter Anchor Bolts
2 1/2" Bolt Projection



PATTERN FCB1

Qty = 2

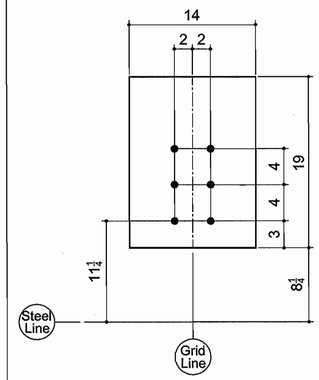
1" Diameter Anchor Bolts
2 1/2" Bolt Projection



PATTERN FCB2

Qty = 2

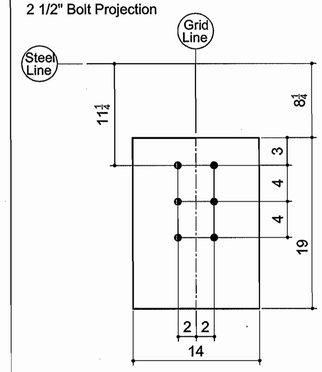
3/4" Diameter Anchor Bolts
2 1/2" Bolt Projection



PATTERN FCB3

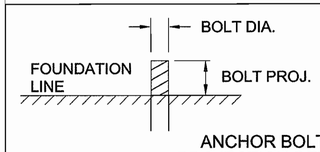
Qty = 2

3/4" Diameter Anchor Bolts
2 1/2" Bolt Projection



PATTERN FCB4

Qty = 2



ANCHOR BOLT PROJECTION DETAIL

ANCHOR BOLT QUANTITIES

Framed Opening and Walk Door Anchorage Devices are NOT Included in this Table

BOLT DIA.	QTY.	BOLT DIA.	QTY.
3/4"	24		
1"	16		

WASHERS SHOULD BE USED AT ALL ANCHOR BOLTS. ANCHOR BOLTS AND WASHERS ARE NOT BY THE METAL BUILDING MANUFACTURER UNLESS OTHERWISE NOTED.

MBS METAL BUILDING SYSTEM
BLDG BUILDING
SL STEEL LINE
BL BASE LINE
EW ENDWALL
SW SIDEWALL
GRID LINE
CENTER LINE
RECESSED OR RAISED COL. BASE
XX INDICATES DIMENSION OF COL. BASE ABOVE OR BELOW MBS BL

SYMBOLS AND ABBREVIATIONS

DESIGN CERTIFICATION

THESE DRAWINGS AND THE METAL BUILDING THEY REPRESENT ARE THE PRODUCT OF MESCO BUILDING SOLUTIONS, (P.O.BOX 93629, SOUTHLAKE, TEXAS 76092, 817-488-8511).

THE ENGINEER WHOSE SEAL APPEARS HEARON IS EMPLOYED BY MESCO BUILDING SOLUTIONS AND IS NOT THE ENGINEER OF RECORD. THE STRUCTURAL DESIGN OF THE BUILDING SYSTEM MEETS THE SPECIFICATIONS INCLUDING THE DESIGN CRITERIA. DESIGN LOADS AND SERVICEABILITY REQUIREMENTS INCORPORATED BY THE BUYER INTO THE ORDER DOCUMENTS.

MESCO'S ENGINEER IS NOT RESPONSIBLE FOR OBSERVATION OR INSPECTION OF THE ERECTION OF THE BUILDING SYSTEM AND IS NOT ACTING AS THE PRIME DESIGN PROFESSIONAL FOR THE CONSTRUCTION PROJECT UTILIZING THIS BUILDING SYSTEM. THE INFORMATION REQUIRED TO INTEGRATE THE BUILDING SYSTEM INTO THE CONSTRUCTION PROJECT IS GIVEN IN THE PROJECT DESIGN DATA.

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	OPER	DET	CKR	ENG
0	6/18/10	ISSUED FOR ERECTOR INSTALLATION	EBF	SML	KB	KH

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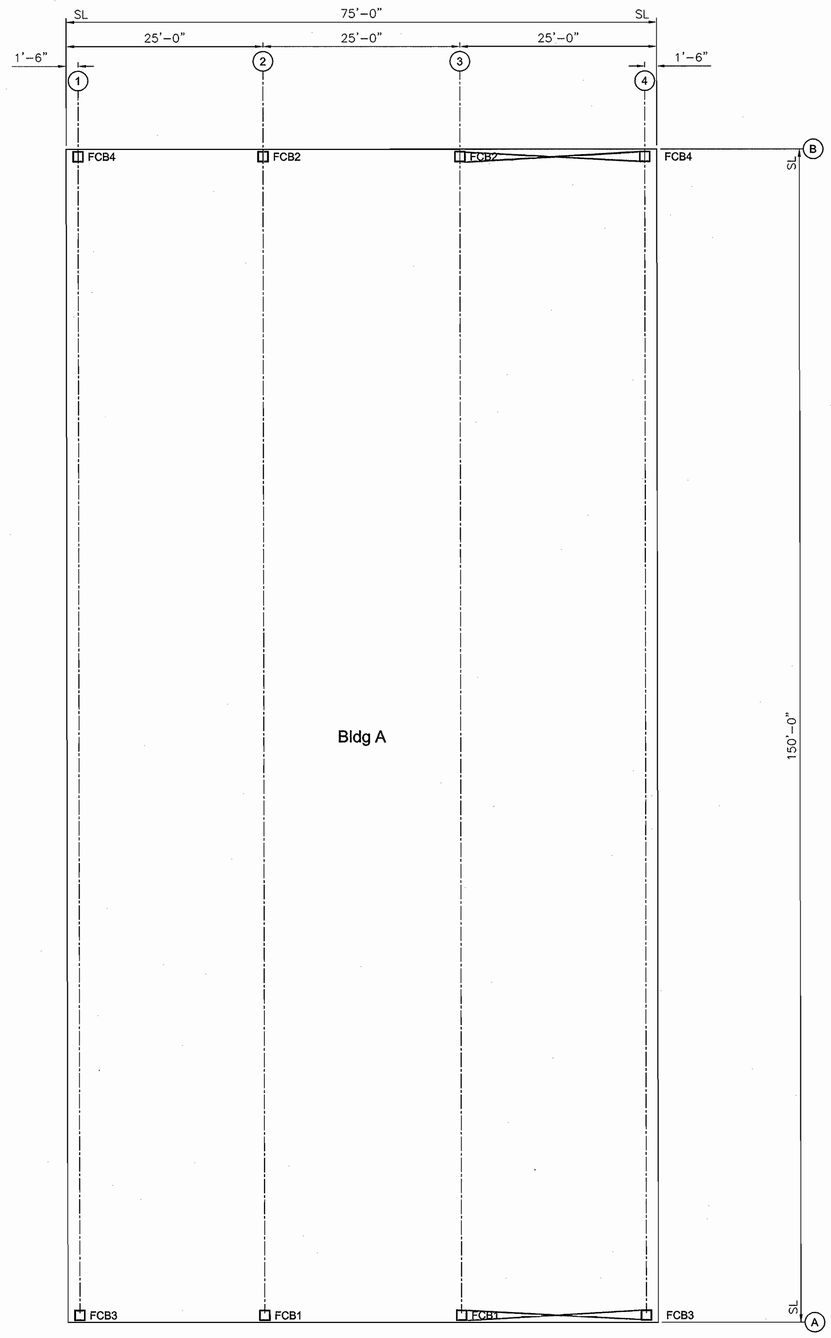
Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer: DWS STRUCTURES
Owner: Abby Greeen, Inc.
Project Name: ABBY GREEN
Job Site Location: WINSTON-SALEM, NC
Job Number: 24-9706
Drawing Issue: 0
Sheet Number: A2 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTHACHAI SUVUNNARUNGSI
JUN 18 2010



SCANNED
JUN 18 2010
MESCO - IDC

BACK
LEFT RIGHT
FRONT

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	OPER	DET	CKR	ENG
0	6/18/10	ISSUED FOR ERECTOR INSTALLATION	EBF	SML	KB	KH

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Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737

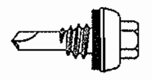
Buyer: DWS STRUCTURES
Owner: Abby Greeen, Inc.
Project Name: ABBY GREEN
Job Site Location: WINSTON-SALEM, NC

Job Number: 24-9706
Drawing Issue: 0
Sheet Number: A1 of 4

Drawing Status:
"FOR ERECTOR INSTALLATION"
These drawings are to be considered as Final Construction Drawings and are to be used for the construction and installation of the project.

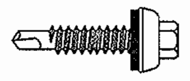
NORTH CAROLINA PROFESSIONAL SEAL 025686 ENGINEER YUTHACHAI SUVORNRUNGSI
JUN 18 2010

EAVE GUTTER
Gutter Stitch Screw
Part No. FS4ADS



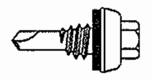
1/4"-14 x 7/8" Standard Life Lap TEK
5/16" Hex Washer Head with
Sealing Washer. Painted
FS4Afc

RAKE TRIM
Trim Member Screw
Part No. FS3DS



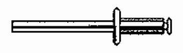
12-14 x 1-1/4" Long Life Driller
5/16" Hex Washer Head with
Sealing Washer. Painted
FS3fc

EAVE GUTTER
Gutter Strap Screw
Part No. FS4APL



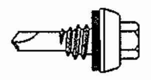
1/4"-14 x 7/8" Standard Life Lap TEK
5/16" Hex Washer Head with
Sealing Washer. Painted
FS4Afc

RAKE TRIM
Trim Pop Rivets
Part No. FS14DS



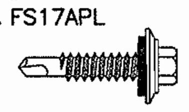
1/8" x 3/16" Pop Rivet
Painted
FS14fc

EAVE GUTTER
Roof Stitch Screw
Part No. FS4APL



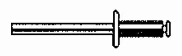
1/4"-14 x 7/8" Standard Life Lap TEK
5/16" Hex Washer Head with
Sealing Washer. Painted
FS4Afc

ROOF
Member Screw
Part No. FS17APL



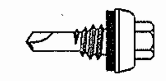
12-14 x 1-1/4" Standard Life Driller
5/16" Hex Washer Head with
5/8" O.D. Washer. Painted
FS17Afc

EAVE GUTTER
Trim Pop Rivets
Part No. FS14DS



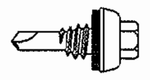
1/8" x 3/16" Pop Rivet
Painted
FS14fc

ROOF
Stitch Screw
Part No. FS4APL



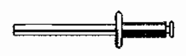
1/4"-14 x 7/8" Standard Life Lap TEK
5/16" Hex Washer Head with
Sealing Washer. Painted
FS4Afc

EAVE TRIM
Eave Trim Stitch Screw
Part No. FS4ADS



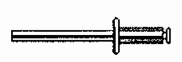
1/4"-14 x 7/8" Standard Life Lap TEK
5/16" Hex Washer Head with
Sealing Washer. Painted
FS4Afc

EAVE TRIM
Trim Pop Rivets
Part No. FS14DS



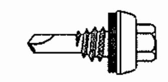
1/8" x 3/16" Pop Rivet
Painted
FS14fc

EAVE TRIM
Member Pop Rivets
Part No. FS14ADS



1/8" x 3/8" Pop Rivet
Painted
FS14Afc

RAKE TRIM
Rake Trim Stitch Screw
Part No. FS4ADS



1/4"-14 x 7/8" Standard Life Lap TEK
5/16" Hex Washer Head with
Sealing Washer. Painted
FS4Afc

EAVE TRIM
Trim Member Screw
Part No. FS3DS

12-14 x 1-1/4" Long Life Driller
5/16" Hex Washer Head with
Sealing Washer. Painted
FS3fc

MAIN BUILDING DESIGN SUMMARY REPORT

Customer Name: DWS Structures — Designed By : KXHU
Job Site: Forsyth, North Carolina — Checked By :_______________

CS 150'0 x 75'0 x 35'6 0.5:12
Bays: 3@25'0 = 3@300

24-9706

Main Code Requirements Per:
2009 North Carolina
(Reference 2006 International Building Code)
Windforce-resistance System Per:
2005 ASCE 7
Seismic-resistance System Per:
2006 International Building Code
Cold-Formed Steel Design Spec:
2001 AISI Allowable Strength Design with 2004 Supplement
Other Steel Design Spec:
2005 AISC Allowable Strength Design, 13th edition

Building Use Category............. Normal
Roof Dead Load
Superimposed .... 2.39 psf
Collateral .... 3.00 psf

Roof Live Load .... 20.00 psf with trib. reduction allowed

Snow Load
Ground Snow Load (Pg)......... 15.00 psf
Snow Load Importance Factor (I) 1.00
Flat Roof Snow Load (Pf) ..... 11.34 psf
Snow Exposure Factor (Ce) 0.90
Thermal Factor (Ct) 1.20
Wind Load
Basic Wind Speed .... 90.00 mph
Wind Importance Factor (I) ... 1.00
Wind Exposure Category .... B
Component Wind Load <= 10 sq. ft.
Zone 4 .... 10.00 psf pressure -10.00 psf suction
Zone 5 .... 0.00 psf pressure 0.00 psf suction
Seismic Load
Seismic Use Group I
Seismic Importance Factor (Ie) 1.00
Design Category C
Site Class .... D -Stiff Soil
Ss .... 0.231 g — Sds...... 0.246 g
S1 .... 0.086 g — Sd1...... 0.138 g
Analysis Procedure .... Equivalent Lateral Force

	Transverse**	Longitudinal**
Basic Force Resisting System		
Response Modification Coefficient (R)	3.00	3.00
Deflection Amplification Factor (Cd)	3.00	3.00
System Over-Strength Factor (Omega)	2.50	2.50
Entire Building Design Base Shear (V)	9.9k	9.9k

** - Transverse direction is parallel to the main frames;
Longitudinal direction is perpendicular to the main frames;

Rainfall Intensity, 5-minute duration
5-year recurrence (I1) .... 6 in/hour
25-year recurrence (I2) ...... 9 in/hour

BUILDING INFORMATION

SYSTEM: 1 AREA: A

TYPE	GET	ROOF PANEL	"PBR"	26Ga. GP
WIDTH	150'-0"	FRONT WALL PANEL	OTR	
LENGTH	75'-0"	BACK WALL PANEL	OTR	
HEIGHT	35'-6"	LEFT WALL PANEL	OTR	
SLOPE	1/2:12	RIGHT WALL PANEL	OTR	

TABLE OF CONTENTS

TITLE SHEET DRAWINGS T1
ANCHOR BOLT DRAWINGS .... A1 – A4
ERECTION DRAWINGS .... E1 – E3
DETAILS/SECTIONS DRAWINGS D1
REFERENCE DRAWINGS .... S1 – S4

GENERAL INFORMATION Version 2000
Care of Material, Pg.1 RG0101-0
Care of Material, Pg.2 RG0102-0
Fastener Identification RG0103-1
Bolt & Nut Identification RG0104-2
Trim Laps, Miscellaneous Trims RG0105-1
Trim Laps, Standard Roof Trims RG0106-1
Bolt Tightening Methods RG0109-0
Dealer Responsibilities RG0110-0
PBR ROOF SYSTEM
Gable Standard Construction RR0101-2
PBR ROOF EAVE AND RAKE
Rake/Eave Corner Standard Trim-LS RR0307-0
DOWNSPOUTS
Roll Formed Downspouts Pg.1 RW0303-0
Roll Formed Downspouts Pg.2 RW0304-0
BRACING DETAILS
Thru Web Type Bracing RB0101-1

ROOF ACCESSORIES

EAVE GUTTER
AT FRONT AND BACK WALLS

[2]-The Metal Building Manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections (including inspection of high strength bolts or field welds) as required during erection. The coordination and costs associated for setting up any Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

[9]-Loads, as noted, are as given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

[18]-"X"-Bracing is to be installed to a taut condition with all slack removed. Do not tighten beyond this state.

*[19]-The framing as shown at this elevation is not designed for future expansion. Corresponding frame reactions are calculated based upon actual tributary area. @ F.L.1&4

[30]-Building is designed assuming a Fully Exposed snow exposure per IBC/ASCE 7, and as such cannot be located near obstructions that may exceed the height of the roof, such as taller evergreen trees, other taller structures within 20 feet of the building in any direction, or any roof mounted equipment. Failure to comply with this requirement will result in voiding of any and all warranties and engineering certifications, including engineering seals.

General Notes

1. THE ERECTION DRAWING PACKAGE MAY CONSIST OF ERECTION DRAWINGS, DETAIL SHEETS, REFERENCE DRAWINGS, REFERENCE MANUALS, THIS TITLE SHEET DRAWING AND A BILL OF MATERIALS.
2. ALL DRAWINGS ARE TO BE USED IN CONJUNCTION WITH EACH OTHER. IF CONFLICTS OCCUR THE ERECTION DRAWINGS SHALL TAKE PRECEDENCE.
3. THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS INTO LINE, SHIMMING, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NORMAL PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM.
4. THE MANUFACTURER IS NOT LIABLE FOR ANY CLAIM RESULTING FROM USE OF ANY DRAWINGS OR LITERATURE NOT SPECIFICALLY RELEASED FOR CONSTRUCTION FOR THE PROJECT.
5. THE MANUFACTURER IS NOT LIABLE FOR ANY CLAIM RESULTING FROM USE BY THE ERECTOR OF ANY IMPROPER MATERIAL OR MATERIAL CONTAINING DEFECTS, WHICH CAN BE DETECTED BY VISUAL INSPECTION. COSTS OF DISASSEMBLING SUCH IMPROPER OR DEFECTIVE MATERIAL AND COSTS OF ERECTING REPLACEMENT MATERIAL ARE NOT SUBJECT TO CLAIM.

Design Certification

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Customer Service "Notice"

In the event of fabrication or design problems, shortages of material, damaged or wrong material customer service is available to assist you. Should one of these problems occur contact our customer service department at the numbers listed below. When contacting us please refer to the "Job Number" shown in the lower right hand corner of this sheet.

Customer Service: Telephone 1-800-556-3726
or Fax 214-687-9737

Note: In order to receive reimbursement of costs, customer service must approve cost of materials or field work prior to procurement of materials or to work being performed. See the truck copy of the erection drawing package for further information and for sample forms of authorization to perform field work.

Special Notice for Buildings with GRAY PRIMER:

When Gray Primer is specified the customer is reminded that Gray Primer is intended as a PRIMER and that it should have minimal exposure to atmospheric conditions. The customer should also be reminded of the potential aesthetic issues that are specifically associated with Gray Primers such as: 1) Gray Primer will show rust spots/streaks due to imperfections in the application process and the properties associated with Gray Primers and 2) Abrasions caused by handling, loading, shipping, unloading, and during the erection process are unavoidable. As a result any rusting or abrasions on members with Gray Primer are NOT subject to customer rejection or claim for touch up to Mesco Building Solutions

Abbreviations

BLDG	= BUILDING	BEF	= BEARING END FRAME	GA	= GAUGE
SW	= SIDEWALL	FEF	= FULL END FRAME	UN	= UNLESS NOTED
EW	= ENDWALL	BUE	= BUILT-UP END FRAME	psf	= POUNDS PER SQUARE FOOT
FW	= FRONT WALL	SL	= STEEL LINE	mph	= MILES PER HOUR
BW	= BACK WALL	BL	= BASE LINE	kip	= 1000 POUNDS
LW	= LEFT WALL	RL	= RIDGE LINE		
RW	= RIGHT WALL				
BES	= BOTTOM OF EAVESTRUT (LOW SIDE)				
BEH	= BOTTOM OF EAVESTRUT (HIGH SIDE)				

Symbols

○ GRID LINE

⬡ PARTITION COLUMN LINE OR FLOOR COLUMN LINE

◇ THIS SYMBOL IS USED ON THE ERECTION DRAWINGS TO REFERENCE TO A DETAIL OR SECTION THAT CAN BE FOUND ON THE "DETAIL SHEETS" (D1, D2 . . .) THE DETAIL NAMES ARE LETTERS ◇A THRU ◇ZZ AND ARE ARRANGED ON THE DETAIL SHEETS STARTING WITH SHEET D1, IN ALPHABETICAL ORDER FROM LEFT TO RIGHT TOP TO BOTTOM.

DRAWING ISSUE HISTORY

NO.	DATE	DESCRIPTION	DET	CKR	ENG
0	6-28-10	ISSUED FOR ERECTOR INSTALLATION	SML	KEB	KH

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MESCO
Mesco Building Solutions
5244 Bear Creek Court, Irving, Texas 75061
Voice 214-687-9999 Fax 214-687-9737
MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

Buyer:
DWS STRUCTURES
Owner:
ABBY GREEN,INC
Project Name:
Abby Green
Job Site Location:
Winston-Salem, NC

Job Number
24-9706

Drawing Issue
0

Sheet Number
T1 of 1

Drawing Status:
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JUN 28 2010