WITHERS & RAVENEL

ENGINEERS | PLANNERS | SURVEYORS

CENTRAL CAROLINA TIRE
PHASE 3 SCRAP TIRE MONOFILL LANDFILL

EROSION & SEDIMENT CONTROL PLAN

HARNETT COUNTY, NORTH CAROLINA

Carmen Johnson

Fac/Perm/Co ID#	Date	Doc ID#
43-04	~~3/26/12~~ 4/2/12 (cj)	

Prepared For:

Central Carolina Tire
1616 McKoy Town Road
Cameron, North Carolina 28326

APPROVED
DIVISION OF WASTE MANAGEMENT
SOLID WASTE SECTION
DATE 4/14/05 BY DJB
PHASE III
PTC
43-04
RCO CENTRAL FILE

By:

Withers & Ravenel, Inc.
111 MacKenan Drive
Cary, North Carolina 27511

January 7, 2005
(modified March 23, 2005)

W&R Project No. 202334.01

NCDENR

North Carolina Department of Environment and Natural Resources
Division of Land Resources
Land Quality Section

James D. Simons, PG, PE
Director and State Geologist

Michael F. Easley, Governor
William G. Ross Jr., Secretary

April 8, 2005

LETTER OF APPROVAL

CORRECTED COPY

Thomas Womble
Central Carolina Holdings, LLC
1616 McKoy Town Road
Cameron, NC 28326

RE: Project Name: Central Carolina Tire-Ph 3 Tire Monofill Landfill Acres Approved: 63
Project ID: **Harne-2005-022**
County: Harnett
River Basin: Cape Fear Stream Classification: Other
Submitted By: Whithers & Ravenel
Date Received by LQS: 3/24/2005
Plan Type: Revised

Dear Thomas Womble:

This office has reviewed the subject erosion and sedimentation control plan. We find the plan to be acceptable and hereby issue this Letter of Approval. The enclosed Certificate of Approval must be posted at the job site. This plan approval shall expire three (3) years following the date of approval, if no land-disturbing activity has been undertaken, as is required by Title 15A NCAC 4B .0129.

Title 15A NCAC 4B .0118(a) requires that a copy of the approved erosion control plan be on file at the job site. Also, this letter gives the notice required by G.S. 113A-61.1(a) of our right of periodic inspection to insure compliance with the approved plan.

North Carolina's Sedimentation Pollution Control Act is performance-oriented, requiring protection of existing natural resources and adjoining properties. If, following the commencement of this project, the erosion and sedimentation control plan is inadequate to meet the requirements of the Sedimentation Pollution Control Act of 1973 (North Carolina General Statute 113A-51 through 66), this office may require revisions to the plan and implementation of the revisions to insure compliance with the Act.

COPY

Fayetteville Regional Office
225 Green Street – Suite 714, Fayetteville, North Carolina 28301 • Phone: 910-486-1541 / FAX: 910-486-0707

CORRECTED COPY

Acceptance and approval of this plan is conditioned upon your compliance with Federal and State water quality laws, regulations, and rules. In addition, local city or county ordinances or rules may also apply to this land-disturbing activity. This approval does not supersede any other permit or approval.

Please be aware that your project will be covered by the enclosed NPDES General Stormwater Permit NCGO1000 (Construction Activities). You should first become familiar with all of the requirements for compliance with the enclosed general permit.

Please note that this approval is based in part on the accuracy of the information provided in the Financial Responsibility Form, which you provided. You are requested to file an amended form if there is any change in the information included on the form. In addition, it would be helpful if you notify this office of the proposed starting date for this project. Please notify us if you plan to have a preconstruction conference.

Your cooperation is appreciated.

Sincerely,

Steve Cook

Steve Cook, C.P.E.S.C
Land Quality Section

Enclosures: Certificate of Approval
NPDES Permit

cc: Withers & Ravenel
Linwood McDonald, Building Inspector
Belinda Henson, Division of Water Quality
Land Quality Section-Regional Office File

NCDENR
North Carolina Department of Environment and Natural Resources
Division of Land Resources
Land Quality Section

James D. Simons, PG, PE
Director and State Geologist

Michael F. Easley, Governor
William G. Ross Jr., Secretary

February 17, 2005

RECEIVED
FEB 22 2005
WITHERS & RAVENEL

LETTER OF APPROVAL WITH MODIFICATIONS

Thomas Womble
Central Carolina Holdings, LLC
1616 McKoy Town Road
Cameron, NC 28326

RE: Project Name: Central Carolina Tire-Ph 3 Tire Monofill Landfill Acres Approved: 63
Project ID: Harne-2005-022
County: Harnett 1616 McKoy Town Rd
River Basin: Cape Fear Stream Classification: Other
Submitted By: Withers & Ravenel
Date Received by LQS: 1/19/2005
Plan Type: New

Dear Thomas Womble:

This office has reviewed the subject erosion and sedimentation control plan. We find the plan to be acceptable with modifications and hereby issue this letter of Approval With Modifications. The Modifications Required for Approval are listed on the attached page. This plan approval shall expire three (3) years following the date of approval, if no land-disturbing activity has been undertaken, as is required by Title 15A NCAC 4B .0129.

Please be advised that Title 15A NCAC 4B .0118(a) requires that a copy of the approved erosion control plan be on file at the job site. Also, you should consider this letter to give the Notice required by G.S. 113A-61.1(a) of our right of periodic inspection to insure compliance with the approved plan.

North Carolina's Sedimentation Pollution Control Program is performance-oriented, requiring protection of existing natural resources and adjoining properties. If, following the commencement of this project, it is determined that the erosion and sedimentation control plan is inadequate to meet the requirements of the Sedimentation Pollution Control Act of 1973 (North Carolina General Statute 113A-51 through 66), this office may require revisions to the plan and implementation of the revisions to insure compliance with the Act.

Acceptance and approval of this plan is conditioned upon your compliance with Federal and State water quality laws, regulations, and rules. In addition, local city or county ordinances or rules may also apply to this land-disturbing activity. This approval does not supersede any other permit or approval.

Please be aware that your project will be covered by the enclosed NPDES General Stormwater Permit NCGO1000 (Construction Activities). You should first become familiar with all of the requirements for compliance with the enclosed general permit.

Please note that this approval is based in part on the accuracy of the information provided in the Financial Responsibility Form, which you have provided. You are requested to file an amended form if there is any change in the information included on the form. In addition, it would be helpful if you notify this office of the proposed starting date for this project. Please notify us if you plan to have a preconstruction conference.

Your cooperation is appreciated.

Sincerely,

Steve Cook

Steve Cook, C.P.E.S.C
Land Quality Section

Enclosures: Certificate of Approval
Modifications Required for Approval
NPDES Permit

Cc: Withers & Ravenel
Linwood McDonald, Building Inspector
Paul Rawls, Division of Water Quality
Land Quality Section

MODIFICATIONS REQUIRED FOR APPROVAL

Project Name: Central Carolina Tire-Ph 3 Tire Monofill Landfill
Project ID: Harne-2005-022
County: Harnett

1. Diversion ditches should discharge into the Temporary Sediment Traps as far away as possible from the trap outlets to avoid short-circuiting the traps.

March 23, 2005

Mr. Steve Cook
North Carolina DENR
Fayetteville Regional Office
Systel Building
225 Green St., Suite 714
Fayetteville, NC 28301-5094

Dear Mr. Cook:

As you can recall, Withers & Ravenel previously submitted an Erosion and Sediment Control Permit Application for land disturbance activities associated with the Central Carolina Tire (CCT) Phase 3 Tire Monofill Landfill dated January 7, 2005. The Division approved the application on February 17, 2005.

Subsequent to the submission of the January 7, 2005 Erosion and Sediment Control Permit Application, the proposed site development (phasing) plan was modified. To support the modified phasing plan, W&R has prepared the enclosed modified Erosion and Sediment Control Permit Application for your review.

The herein submitted modified Application, dated March 23, 2005, revises the facility filling/phasing plan (i.e., revised configuration for construction associated with landfill Phases 3A, 3B, 3C, and 3D). No changes to the total acreage disturbed (63 acres) are proposed as part of this modification. Specifically, as compared to the January 7, 2005 submitted Application, the modified March 23, 2005 application revises the following items:

- Erosion & Sediment Control Plan Report
    - *Section 1.1. Project Narrative.* The project narrative has been modified to provide discussion of the modified phasing plan.
    - *Section 2.0 Planned Erosion & Sediment Control Practices.* The discussions for Practice Items *2.1, 2.5, 2.6, 2.7, 2.8, 2.9, 2.10, and 2.11* have been modified to identify revised figure references.
    - *Section 3.0 Construction Sequence.* The construction sequence has been modified to support the modified phasing plan. Construction sequences for each phase of construction have been added to the figures.
    - *Appendix C*: Stormwater calculations have been revised to support the

modified phasing plan.

- Erosion & Sediment Control Plan Figures (*Appendix A*)
    - *Sheets C-1, C-2, SD-1, SD-2, SD-3, SD-4, SD-5, and SD-6.* The figures have been revised to show the modified landfill phasing plan. Additionally, a sediment and erosion control plan for each phase of landfill construction (Phases 3A, 3B, 3C, and 3D) is included on Figures *SD-1, SD-2, SD-3, SD-4*, respectively. There are no modifications to the overall landfill base grades, ancillary features (including perimeter road, ditches, sediment basins, etc.), and total acreage disturbed as part of this modification.

The herein March 23, 2005 submitted Application is intended to replace the January 7, 2005 submitted Application in its entirety.

Central Carolina Tire and Withers & Ravenel appreciate your assistance and cooperation in this matter. If you have any questions or comments, or need any additional information during your review, please do hesitate to call.

Very truly yours,

WITHERS & RAVENEL, INC.

Stefan Bray, P.E.

cc: Thomas Womble, Central Carolina Tire

CERTIFICATION PAGE

I HEREBY CERTIFY THAT THIS APPLICATION FOR THE CENTRAL CAROLINA TIRE PAHSE 3 MONOFILL LANDFILL CONTAINED HEREIN WERE PREPARED BY ME OR UNDER MY DIRECT SUPERVISION UNLESS NOTED OTHERWISE BY A SIGNATURE AND SEAL FROM ANOTHER REGISTERED ENGINEER.

WITHERS & RAVENEL, INC.

SIGNED, SEALED, AND DATED THIS 23 DAY OF MARCH 20 05

By: J. Stefan Bray

Title: Engineer

NORTH CAROLINA PROFESSIONAL SEAL 025900 ENGINEER J. STEFAN BRAY

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For
Land Disturbance Activities
Associated with the
Central Carolina Tire Phase 3 Monofill Landfill Site

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STATE OF NORTH CAROLINA
Department of Environment, Health and Natural Resources,
Land Quality Section

EROSION AND SEDIMENTATION CONTROL PLAN CHECKLIST

The following items should be incorporated with respect to specific site conditions, in an erosion and sedimentation control plan:

LOCATION INFORMATION

- X Project location
- X Roads, streets
- X North arrow
- X Scale
- X Adjoining lakes, streams or other major drainage ways

GENERAL SITE FEATURES

- X North arrow
- X Scale
- X Property line
- X Legend
- X Existing contours
- X Limit and acreage of disturbed area
- N/A Planned existing buildings location and elevations
- X Planned and existing roads location and elevations
- N/A Lot and/or building numbers
- X Land use of surrounding areas
- N/A Rock outcrops
- N/A Seeps or springs
- X Wetlands limits
- X Easements
- X Streams, lakes, ponds, drainage ways, dams
- X Boundaries of the total tract
- X Borrow and/or waste areas (Note – when the person financially for all the overall project is not the person financially responsible for off-site borrow areas not regulated by the provisions of the Mining Act of 1971 and off-site waste areas other than land fills regulated by the Department of Human Resources, such areas should be considered separate land disturbing activities subject to the Sedimentation Pollution Control Act of 1973. Off-site borrow areas are subject to the requirements of the Mining Act of 1971).
- X Stockpiled topsoil or subsoil location
- N/A Street profiles

SITE DRAINAGE FEATURES

- X Existing and planned drainage patterns (include of-site areas that drain through project)
- X Size of Areas (Acreage)
- N/A Site of location of culverts and sewers
- X Soils information (type, special characteristics)
- X Design calculations for peak discharges of runoff (including the construction phase and final runoff coefficients of the site)
- X Design calculations and construction details for culverts and storm sewers
- X Design calculation, cross sections and method of Stabilization of existing planned channels (include temporary linings)
- X Design calculations and construction details of energy dissipaters below culverts and storm sewer outlets (for rip-rap aprons, include stone sizes (diameters) and apron dimensions)
- X Soil information below culvert and stone sewer outlets
- X Design calculations and construction details to control groundwater, i.e. seeps, high water table, etc.
- N/A Name of receiving watercourse or name of municipal operator (only where stormwater discharges are to occur)

EROSION CONTROL MEASURES

- X Legend
- X Location of temporary and permanent measures
- X Construction drawings and details for temporary and permanent measures
- X Design calculations for sediment basins and other measures
- X Maintenance requirements during construction
- X Person responsible for maintenance during construction
- X Maintenance requirements and responsible person(s) of permanent measures

VEGETATIVE STABILIZATION

- X Areas and acreage to be vegetatively stabilized
- X Planned vegetation with details of plants, seed, mulch and fertilizer
- X Specifications for permanent and temporary vegetation
- X Method of soil preparation

NOTE: Should include provisions for ground cover on exposed slopes within 30 working days following completion of any phase of grading, permanent ground cover for all disturbed areas within 30 working days or 120 calendar days (whichever is shorter) following completion of construction or development.

OTHER REQUIREMENTS

- X Narrative describing construction sequence (as needed)
- X Narrative describing the nature and purpose of the construction activity
- X Completed Financial Responsibility/Ownership Form (to be signed by person financially responsible for project)
- N/A Bid specifications regarding erosion control
- X Construction sequence related to sedimentation and erosion control (include installation of critical measures prior to initiation of the land-disturbing activity and removal of measures after areas they serve have been permanently stabilized.

FINANCIAL RESPONSIBILITY/OWNERSHIP FORM
SEDIMENTATION POLLUTION CONTROL ACT

No person may initiate any land-disturbing activity on one or more acres as covered by the Act before this form and an acceptable erosion and sedimentation control plan have been completed and approved by the Land Quality Section, N.C. Department of Environment Health, and Natural Resources. (Please type or print and, if question is not applicable, place N/A in the blank).

PART A.

1. Project Name Central Carolina Tire - Phase III Tire Monofill Landfill

2. Location of land disturbing Activity: County Harnett

   City or Township near Cameron, and Highway/Street NC Highway 1105 1616 McKoy Town Rd.

3. Approximate date land-disturbing activity will be commenced: Summer 2005

4. Purpose of development (residential, commercial, industrial, etc.): commerical

5. Total acreage disturbed or uncovered (including off-site borrow and waste areas): 63 acres

6. Amount of fee enclosed $ 3,150.00

7. Has an erosion and sedimentation control plan been filed? Yes X No _____
   Enclosed X

8. Person to contact should sediment control issues arise during land-disturbing activity.
   Name Thomas Womble Telephone (919)-499-2301

9. Land owner(s) of Record (Use blank page to list additional owners):

Diane Womble	
Name	Name
1616 McKoy Town Road	
Current Mailing Address	Current Mailing Address
Cameron NC 28326	
City State Zip	City State Zip

10. Recorded in Deed Book No. 1434 Page No. 541

PART B.

1. Person(s) or firm(s) who are financially responsible for this land-disturbing activity. (Use a blank page to list additional persons or firms):

   Thomas Womble, Central Carolina Holdings, LLC
   Name of Person or Firm

1616 McKoy Town Road	Same
Mailing Address	Street Address
Cameron NC 28326	
City State Zip	City State Zip
(919)-499-2301	
Telephone	Telephone

2. (a) If the Financially Responsible Party is not a resident of North Carolina, give name and street address of a North Carolina Agent.

N/A
Name

Mailing Address | Street Address

City State Zip | City State Zip

Telephone | Telephone

(b) If the Financially Responsible Party is a Partnership or other person engaging in business under an assumed name, attach a copy of the Certificate of Assumed Name. If the Financially Responsible Party is a Corporation, give name and street address of the Registered Agent.

N/A
Name of Registered Agent

Mailing Address | Street Address

City State Zip | City State Zip

Telephone | Telephone

The above information is true and correct to the best of my knowledge and belief and was provided by me under oath. (This form must be signed by the financially responsible person if an individual or his attorney-in-fact or if not an individual by an officer, director, partner, or registered agent with authority to execute instruments for the financially responsible person). I agree to provide corrected information should there be any change in the information provided herein.

Thomas Womble
Type or print name
Signature
Thomas Womble

Central Carolina Holdings, LLC
Title or Authority
Date
1-7-05

I, Sue E. Crow, a Notary Public of the County of ~~Harnett~~ Wake, State of North Carolina, hereby certify that Thomas Womble, personally appeared before me this day and being duly sworn acknowledged that the above form was executed by him.

Witness my hand and notarial seal, this 7th day of January, 20 05

(Seal) SUE E. CROW NOTARY * * * PUBLIC WAKE COUNTY, N.C.

Sue E. Crow
Notary

My commission expires 12-10-2008

1.0 Narrative

1.1 Project Description

The Phase 3 Tire Monofill Landfill project site is located on the property of the Central Carolina Tire facility in Harnett County, near Cameron, North Carolina. A vicinity map for the site is presented on the cover page of the plan drawings, which are presented in Appendix A. The purpose of this project is to construct the proposed Central Carolina Tire Phase 3 Tire Monofill Landfill. Figures showing the existing site conditions and proposed overall site grading plan are presented in Figures C-1 and C-2 of the plan drawings. The Phase 3 landfill is proposed for development in four construction sequences (Phase 3A, 3B, 3C, and 3D), as shown on the site development drawings (SD-1 through SD-6).

The first proposed phase of the construction, Phase 3A, will consist of construction of the Phase 3A landfill area, a portion of the overall access road, one new sediment pond, a network of diversion channels, and temporary sediment and erosion control structures and devices (sediment traps, silt fencing, etc.). It is currently anticipated that initial land disturbance activities for Phase 3A will occur in 2005.

The second proposed phase of the construction, Phase 3B, will consist of construction of the Phase 3B landfill area, a portion of the overall access road, and a network of diversion channels and temporary sediment and erosion control structures and devices (sediment traps, silt fencing, etc.). Construction of Phase 3B requires the deposition of fill material into about 1.96-acres of jurisdictional freshwater wetlands. Complete wetlands impact permitting including a U.S. Army Corp of Engineers issued Individual Permit, a Water Quality Section issued 401 Water Quality Certification, and an approved and executed mitigation plan will be prepared prior to initiating wetland filling activities in this area. It is currently anticipated that land disturbance activities for Phase 3B will occur no later than 2015.

The third proposed phase of the construction, Phase 3C, will consist of construction of the Phase 3C landfill area, one new sediment pond, and a network of diversion channels and temporary sediment and erosion control structures and devices (sediment traps, silt fencing, etc.). It is currently anticipated that initial land disturbance activities for Phase 3C will occur no later than 2023.

The last proposed phase of the construction, Phase 3D, will consist of construction of the Phase 3D landfill area, the remaining portion of the overall access road, and a network of diversion channels and temporary sediment and erosion control

structures and devices (sediment traps, silt fencing, etc.). It is currently anticipated that initial land disturbance activities for Phase 3D will occur no later than 2037.

Proposed grading for the project site promotes positive drainage. All sediment-laden surface water runoff within the proposed limits of disturbance will be directed across the site via surface sheet flow and the proposed diversion channels located along the edge of the landfill and access roads. All stormwater will be routed through one of the two proposed engineered sediment basins with riser/barrel outlet structures located at the western and eastern extents of the project area.

The purpose of this report is to provide an erosion and sedimentation control plan for the construction and operation of the Central Carolina Tire Phase 3 Tire Monofill Landfill.

1.2 Total Disturbed Acreage

The total disturbed area will be approximately 63 acres.

1.3 Site Description

The existing Central Carolina Tire facility is located approximately ten miles southeast of Cameron, North Carolina along NC 24/27. Specifically, the site is 8,500′ south of the intersection of NC Hwy 24/27 and NC Highway 1105 (McKoy Town Road) and approximately one mile northeast of Lake Surf. A vicinity map for the project site is located on the cover page of the Plan drawings contained in Appendix A.

The existing topography at the proposed site varies in elevation from 300 feet M.S.L. in the southeastern area to 360 feet M.S.L. in the northwestern area. On-site surface drainage travels primarily in a southeast direction, and ultimately discharges into the Cape Fear. There is one perennial or intermittent stream located on-site or within 500 feet of the property boundaries. This stream is located approximately 800 feet to the east of the proposed site.

According to a map (attached in Appendix B) prepared by the North Carolina Center for Geographic Information and Analysis dated December 2003, no designated Critical Areas, High Quality Water Zones, Outstanding Resource Water Zones, or Special Management Strategy Areas have been identified by the various source agencies in proximity to the project site.

According to the Federal Emergency Management Agency's (FEMA) Flood Insurance Map (attached in Appendix B), the 100-year flood-plain does not encroach into the project area.

Discussions regarding jurisdictional wetlands at the site are presented in Item 1.5 of this section

1.4 Solid Waste Regulatory Background

Central Carolina Tire owns and operates a scrap tire collection and processing facility and a scrap tire monofill landfill at the site. The existing 40.7-acre scrap tire monofill landfill (Phase I/II) is permitted to dispose of scrap tires only and is nearing capacity. To provide long-term disposal capacity, Central Carolina Tire intends to construct a new approximate 42.8-acre scrap tire monofill landfill, herein referred to as the Phase III Tire Monofill Landfill. The proposed 42.8-acre Phase III Tire Monofill Landfill will be located adjacent to and generally north of the existing Central Carolina Tire Phases I/II Monofill Landfill (NC DENR Solid Waste Management landfill permit # 43-04), and generally south of the existing Scrap Tire Collection and Processing Facility (NC DENR Solid Waste Management landfill permit # 43-08 TP). A "Permit To Construct" application for the 42.8-acre Phase III Tire Monofill Landfill was submitted to NC DENR Solid Waste Management in October 2004 and was recently approved.

1.5 Wetlands

A wetland delineation was recently performed by Central Carolina Tire's wetland consultant, the Brighman Company of Conway, South Carolina, for the entire site. Subsequent to submitting the Brighman Company's findings to the U.S. Army Corp of Engineers, the Brighman Company's wetland delineation of the site was approved by the U.S. Army Corp of Engineers on March 2, 2004. A copy of the certified wetland delineation is attached in Appendix B.

The construction of the proposed monofill landfill will require the deposition of fill material into about 1.96-acres of jurisdictional freshwater wetlands. As part of Central Carolina Tire's landfill expansion permit document preparation, The Brighman Company of Conway, South Carolina has consulted with the U.S. Army Corps of Engineers and the NC Division of Water Quality (DWQ) concerning wetland permitting and proposed wetland impact issues at the site. A U.S. Army Corps of Engineers Individual (404) Permit Application for the project was submitted to the Corps in January 2005 and is currently under review.

Complete wetlands impact permitting including a U.S. Army Corp of Engineers issued Individual Permit, a Water Quality Section issued 401 Water Quality Certification, and an approved and executed mitigation plan will be prepared prior to initiating wetland filling activities.

1.6 Design and Performance Standards

Erosion and sedimentation control measures, structures, and devices are designed to provide protection from the run off of the 25-year storm which produces the maximum peak rate of run off as calculated according to procedures in the United States Department of Agriculture Soil Conservation Service's "National Engineering Field Manual for Conservation Practices".

PondPack® Detention Pond Design & Urban Hydrology Model software was used in the calculations. Where applicable, local settings including, but not limited to, rainfall intensity and rainfall amounts were obtained from the design curves presented in the North Carolina Erosion and Sediment Control Planning and Design Manual.

The erosion and sedimentation control measures, structures, and devices were designed so that the post construction velocity of the 25-year storm run off in the receiving watercourse does not exceed the velocity of the 25-year storm run off in the receiving watercourse prior to development. Figures DA-1 and DA-2 identify the pre- and post-development drainage areas, respectively. Additionally, the erosion and sedimentation control measures, structures, and devices were designed using the guidelines presented in the North Carolina Erosion and Sediment Control Planning and Design Manual.

Engineering support calculations for the erosion and sedimentation control measures, structures, and devices demonstrating compliance with the design and performance standards are presented in Appendix C.

2.1 Land Grading – Practice 6.02

The proposed landfill will be graded to the overall lines and grades shown on Figure C-2. The initial phases of project development consists of construction of the Phase 3A area, as shown on Figure SD-1. Subsequent development phases (Phases 3B, 3C, and 3D) will be graded to the lines and grades shown on Figures SD-2 through SD-6, respectively. Borrow/grading operations may be performed within future landfill areas (for example, within Phases 3B through 3D areas during operations in Phase 3A area) to provide borrow soils for landfill operations and/or provide for stockpiling of soil surplus from landfill construction.

The two proposed sediment ponds will be graded (cut/fill) to the lines and grades shown on Figures C-2 and D-2 to provide the required storage capacity. The proposed channels will be graded to the specified dimensions.

2.2 Surface Roughening--Practice 6.03

Any disturbed areas will be lightly roughened by discing just prior to vegetating.

Surface stabilization will be accomplished with vegetation and mulch as specified in the vegetation plan.

2.3 Temporary Seeding--Practice 6.10

Temporary seeding will be provided as described in the Vegetative Plan.

2.4 Permanent Seeding--Practice 6.11

Permanent seeding will be provided as described in the Vegetative Plan.

2.5 Temporary Diversions--Practice 6.20

Temporary diversion ditches and berms will be used as shown on Figures SD-1, SD-2, SD-3, and SD-4 to direct runoff toward sediment ponds. Details for the temporary diversions are shown on Figure D-1.

2.6 Grass-Lined Swales--Practice 6.30

A grass-lined swale will be constructed between the landfill cell and the perimeter access road (perimeter ditch) to divert stormwater to the sediment ponds. This grass-lined swale will be stabilized using Excelsior matting until vegetation can be

established. A detail of the perimeter ditch is shown on sheet Figure D-3 and the location where it is to be used is shown on Figures SD-1, SD-2, SD-3, and SD-4. An existing swale which cuts across the western edge of the landfill site (along existing unpaved road) will be utilized to capture sediment-laden runoff and convey it to the proposed sediment pond No. 2.

2.7 Outlet Stabilization Structure--Practice 6.41

Riprap aprons will be used at the outlets of all culverts and sediment basins to prevent scour. Outlets are shown on Figures SD-1, SD-2, SD-3, and SD-4. A detail and schedule is provided on Figure D-1.

2.8 Fabric Drop Inlet Protection--Practice 6.51

A temporary fabric barrier will be placed around storm drain inlets which are to be made operational prior to permanent stabilization of disturbed areas. Storm drain inlets locations are shown on Figures SD-1, SD-2, SD-3, and SD-4, with details shown on sheet D-3.

2.9 Temporary Sediment Trap – Practice 6.60

A total of nine temporary sediment trap devices will be constructed, as shown on Figures SD-1, SD-2, SD-3, and SD-4, to prevent sediment from leaving the property. Sediment traps schedules are shown on Figures SD-1, SD-2, SD-3, and SD-4 and details for the traps are shown on Figure D-1.

2.10 Sediment Basin–Practice 6.61

Two new sediment ponds with riser-barrel outlet devices will be constructed, as shown on Figures SD-1 and SD-3, to prevent sediment from leaving the property. Sediment pond, riser barrel details, and schedules are shown on Figure D-2.

2.11 Silt Fence--Practice 6.62

Silt fence will be installed at the locations shown on Figures SD-1, SD-2, SD-3, and SD-4 until permanent vegetation has been established. A silt fence detail has been provided on Figure D-1.

3.0 Construction Sequence

The construction sequences for land disturbing activities associated with each phase of landfill construction are identified on the Erosion and Sediment Control Plan figures (Sheets SD-1 through SD-6).

The following provides a general construction sequence:

1. Obtain erosion and sediment control permit approval and other applicable permits including a U.S. Army Corp of Engineers issued 404 Individual Permit and a Water Quality Section issued 401 Water Quality Certification.

2. Hold preconstruction conference at least one week prior to starting construction.

3. Install temporary sediment and erosion control devices (silt fence, temporary sediment traps and diversion ditches, etc) as indicated on the drawings.

4. Clear and grub the site as indicated on the drawings.

5. Construct permanent sediment and erosion control devices (sediment ponds No. 1 and No. 2, complete with riser/barrel outlet and energy dissipators) as indicated on the drawings

6. Grade perimeter access road, landfill area, drainage ditches, and install drop inlets and drainage pipe as indicated on the drawings.

7. Immediately stabilize all areas constructed to finished grade with vegetation, paving, ditch linings, etc. Seed and mulch bare areas should grading be discontinued for more than 3 weeks and upon completion of grading activities.

8. Inspect all erosion and sediment control practices weekly and after rainfall events. Needed repairs will be made immediately.

9. After site is stabilized, remove all temporary erosion control measures. Restore sediment ponds to final grade by excavating and disposing all accumulated sediment.

4.0 Maintenance Plan

Check the sediment ponds' riser/barrel outlet structures after each rainfall. Remove sediment and restore original volume when sediment accumulates to about one-feet above the pond bottom. Check the structure for erosion, piping, and rock displacement after each significant rainstorm and repair immediately.

Inspect sediment fences at least once a week and after each rainfall. Make any required repairs immediately.

Inspect diversion dikes and sediment traps once a week and after every rainfall. Immediately remove sediment from the flow area and repair the dike.

Generally, a stand of vegetation cannot be determined to be fully established until soil cover has been maintained for one full year from planting. Inspect seeded areas for failure and make necessary repairs and reseedings with the same season, if possible.

Periodically check all graded areas and the supporting erosion and sedimentation control practices, especially after heavy rainfalls. Promptly remove all sediment from diversions or other water-disposal practices. If washouts or breaks occur, repair them immediately.

5.0 Vegetative Plan

5.1 Seeding Specifications and Schedules

Temporary seeding will be used to temporarily stabilize areas until permanent vegetation or other erosion control measures can be established.

Areas to be stabilized with permanent vegetation will be seeded or planted within 30 working days after final grade is reached.

Permanently seeded areas will be vegetated according to Seeding No. 1CP, 5CP or 7CP, depending on the application and the actual conditions encountered. The following plan is based on the Coastal Plain region recommendations provided in the North Carolina Erosion and Sediment Control Planning and Design Manual.

5.2 Seeding

Riprap, haybales, or other acceptable temporary methods of erosion control may be required until permanent cover is established. Areas where final grade has been reached can be stabilized by mulching until a vegetative cover is established. Soil mulching can be achieved using wood chips, straw, hay, asphalt emulsion, jute matting, and synthetic fibers. Mulches allow for greater water retention; reduce the amount of runoff; retain seeds, fertilizer, and lime in place; and, improve soil moisture and temperature conditions.

5.2.1 Temporary Seeding

Temporary seeding shall be applied as follows:

LATE WINTER & EARLY SPRING

Seeding Mixture	Rate (lb/ acre)	Seeding Dates
Rye (Grain)	120	Dec. 1 - Apr. 15
Annual lespedeza (Kobe in Coastal Plain)	50	

Soil Amendments	Rate (lb/ acre)
Agricultural limestone	2000
Fertilizer (10-10-10)	750
Mulch	4000

SUMMER

Seeding Mixture	Rate (lb/ acre)	Seeding Dates
German millet	40	Apr. 15 - Aug. 15

Soil Amendments	Rate (lb/ acre)
Agricultural limestone	2000
Fertilizer (10-10-10)	750
Mulch	4000

FALL

Seeding Mixture	Rate (lb/ acre)	Seeding Dates
Rye (Grain)	120	Aug. 15 - Dec. 30

Soil Amendments	Rate (lb/ acre)
Agricultural limestone	2000
Fertilizer (10-10-10)	1000
Mulch	4000

5.2.2 Permanent Seeding

Permanent seeding shall be applied as follows:

Seeding Mixture	Rate (lb/ acre)	Seeding Dates
Tall fescue	80	Early Spring
Pensacola Bahiagrass	50	Feb. 15 - Mar. 20
Sericea lespedeza	30	Fall
Kobe lespedeza	10	Sept. 1 - Sept. 30

Soil Amendments	Rate (lb/ acre)
Agricultural limestone	3000-5000
Fertilizer (10-10-10)	1000
Mulch	4000

5.3 Soil Amendments

Soil amendments should be applied as specified in the applicable seeding mixture (above). Alternately, soil may be amended according to recommendations made in a NCDA Soil Test Report for soil samples obtained at the site.

5.4 Mulch

Suitable mulch should be provided and applied as specified in the applicable seeding mixture (above).

5.5 Maintenance

Re-fertilize in the second year unless growth is fully adequate. May be mowed once or twice a year, but mowing is not necessary. Reseed, fertilize, and mulch damaged areas immediately.

6.0 Specifications

6.1 Sediment Pond

1. Site Preparations - Clear, grub, and strip topsoil from areas under the embankment to remove trees, vegetation, roots, and other objectionable material. To facilitate sediment cleanout and restoration, clear the pool area of all brush, trees, and other objectionable materials. Stockpile all topsoil or soil containing organic matter for use on the outer shell of the embankment to facilitate vegetative establishment.

2. Cut-off trench - Excavate a cut-off trench along the centerline of the earth fill embankment. Cut the trench to stable soil material, but in no case make it less than 2 feet deep. The cut-off trench must extend into both abutments to at least the elevation of the riser crest. Make the minimum bottom width wide enough to permit operation of excavation and compaction equipment but in no case less than 2 feet. Make side slopes of the trench no steeper than 1:1. Compaction requirements are the same as those for the embankment. Keep the trench dry during backfilling and compaction operations.

3. Embankment - Take fill material from the approved areas shown on the plans. It should be clean mineral soil, free of roots, woody vegetation, rocks, and other objectionable material. Scarify areas on which fill is to be placed before placing fill. The fill material must contain sufficient moisture so it can be formed by hand into a ball without crumbling. If water can be squeezed out of the ball, it is too wet for proper compaction. Place fill material in 6 to 8-inch continuous layers over the entire length of the fill area and then compact it. Compaction may be obtained by routing the construction hauling equipment over the fill so that the entire surface of each layer is traversed by at least one wheel or tread track of the heavy equipment, or a compactor may be used. Construct the embankment to an elevation 10% higher than the design height to allow for settling.

4. Conduit spillways - Securely attach the riser to the barrel or barrel stub to make a watertight structural connection. Secure all connections between barrel sections by approved watertight assemblies. Place the barrel and riser on a firm, smooth foundation of impervious soil. Do not use pervious material such as sand, gravel, or crushed stone as backfill around the pipe foundation or anti-seep collars. Place the fill material around the pipe spillway in 4-inch

layers and compact it under and around the pipe to at least the same density as the adjacent embankment. Care must be taken not to raise the pipe from the firm contact with its foundation when compacting under the pipe haunches.

Place a minimum depth of 2 feet of hand compacted backfill over the pipe spillway before crossing it with construction equipment. Anchor the riser in place by concrete or other satisfactory means to prevent floatation. In no case should the pipe conduit be installed by cutting a trench through the dam after the embankment is complete.

5. Emergency spillway - The achievement of planned elevations, grade, design width, and entrance and exit channel slopes are critical to the successful operation of the emergency spillway.

6. Inlets - Discharge water into the basin in a manner to prevent erosion. Use diversions with outlet protection to divert sediment-laden water to the upper end of the pool area to improve basin trap efficiency.

7. Erosion control - Construct the structure so that the disturbed area is minimized. Divert surface water away from bare areas. Complete the embankment before the area is cleared. Stabilize the emergency spillway embankment and all other disturbed areas above the crest of the principal spillway immediately after construction.

8. Safety - Sediment basins attract children and can be dangerous. Avoid steep side slopes, and fence and mark basins with warning signs if trespassing is likely. Follow all state and local requirements.

6.2 Sediment Fence

1. Construct the sediment barrier of standard strength or extra strength synthetic filter fabrics.

2. Ensure that the height of the sediment fence does not exceed 18 inches above the ground surface.

3. Construct the filter fabric from a continuous roll cut to the length of the barrier to avoid joints. When joints are necessary, securely fasten the filter cloth only at a support post with overlap to the next post.

4. Excavate a trench approximately 4 inches wide and 8 inches deep along the proposed line of posts and upslope from the barrier.

5. Backfill the trench with compacted soil or gravel placed over the filter fabric.

6. Do not attach filter fabric to existing trees.

6.3 Channel Construction

1. Excavate the channel and shape it to an even cross-section as shown in the detail on the construction plans. When staking, indicate a 0.2' overcut around the channel perimeter for silting and bulking. Bring over-excavated areas to grade by increasing the thickness of the liner or by backfilling with moist soil compacted to the density of the surrounding material.

2. Grade soil away from channel so that surface water may enter freely.

3. Place matting riprap so that it completely covers the channel bottom and provide overlap and anchorage as recommended by the manufacturer. Exercise care in riprap placement to avoid damage to filter fabric.

4. On channel shoulders, apply 100 lb/1,000 ft^2 grain straw and anchor with 0.1 gal/yd^2 (11 gal/1,000 ft^2) asphalt.

6.4 Fabric Drop Inlet Protection

1. As synthetic fabric, use a pervious sheet of nylon, polyester, or ethylene yarn - extra strength (50 lb/1 inch minimum) - that contains ultraviolet ray inhibitors and stabilizers. Fabric should be sufficiently porous to provide adequate drainage of the temporary sediment pool. Burlap may be used for short-term applications. It must be replaced every sixty (60) days.

2. Cut fabric from a continuous roll to eliminate joints.

3. For stakes, use 2 x 4 -inch wood (preferred) or equivalent metal with a minimum length of 3 feet.

4. Space stakes evenly around the perimeter of the inlet a maximum of 3 feet apart, and securely drive them into the ground, approximately 18 inches deep.

5. To provide needed stability to the installation, frame with 2 x 4 -inch wood strips around the crest of the overflow area at a maximum of 1.5 feet above the drop inlet crest.

6. Place the bottom 12 inches of the fabric in a trench and backfill the trench with at least 4 inches of crushed stone or 12 inches of compacted soil.

7. Fasten fabric securely to the stakes and frame. Joints must be overlapped to the next stake.

8. The top of the frame and fabric must be well below the ground elevation downslope from the drop inlet to keep runoff from bypassing the inlet. It may be necessary to build a temporary dike on the downslope side of the structure to prevent bypass flow. Material from within the sediment pool may be used for diking.

6.5 Temporary Diversions

1. Remove and properly dispose of all trees, brush, stumps, and other objectional material.

2. Ensure that the minimum constructed cross section meets all design requirements.

3. Ensure that the top of the dike is not lower at any point than the design elevation plus the specified settlement.

4. Provide sufficient room around diversions to permit machine grading and cleanout.

5. Vegetate the ridge immediately after construction, unless it will remain in place less than 30 working days.

6.6 Outlet Stabilization Structure

1. Ensure that the subgrade for the filter fabric and riprap follows the required lines and grades shown on the plan. Compact any fill required in the subgrade to the density of the surrounding undisturbed material. Low areas in the subgrade on undisturbed soil may also be filled by increasing the riprap thickness.

2. The riprap must conform to the specified grading limits shown on the plans.

3. Filter cloth, when used, must meet design requirements and be properly protected from punching or tearing during installation. Repair any damage by removing the riprap and placing another piece of filter cloth over the damaged area. All connecting joints should overlap a minimum of one foot. If the damage is extensive, replace the entire filter cloth.

4. Riprap may be placed by equipment, be take care to avoid damaging the filter.

5. The minimum thickness of the riprap should be 1.5 times the maximum stone diameter.

6. Riprap may be field stone or rough quarry stone. It should be hard, angular, highly weather-resistant, and well-graded.

7. Construct the apron on a flat grade with no overfall at the end. Make the top of the riprap at the downstream end level with the receiving area or slightly below it.

8. Ensure that the apron is properly aligned with the receiving stream and preferably straight through its length. If a curve is needed to fit site conditions, place it in the upper section of the apron.

9. Immediately after construction, stabilize all disturbed areas withvegetation.

6.7 Land Grading

1. Construct and maintain all erosion and sedimentation control practices and measures in accordance with the approved sedimentation control plan and construction schedule.

2. As conditions merit, remove good topsoil from areas to be graded and filled, and preserve it for use in finishing the grading of all critical areas.

3. Scarify areas to be topsoiled to a minimum depth of 2 inches before placing topsoil

4. Clear and grub areas to be filled to remove trees, vegetation, roots, or other objectionable material that would affect the planned stability of the fill.

5. Ensure that fill material is free of brush, rubbish, rocks, logs, stumps, building debris, and other materials inappropriate for constructing stable fills.

6. Place all fill in layers not to exceed 9 inches in thickness, and compact the layers as required to reduce erosion, slippage, settlement, or other related problems.

7. Do not incorporate frozen material or soft, mucky, or highly compressible materials into fill slopes.

8. Do not place fill on a frozen foundation, due to possible subsidence and slippage.

9. Keep diversions and other water conveyance measures free of sediment during all phases of development.

10. Handle seeps or springs encountered during construction in accordance with approved methods.

11. Permanently stabilize all graded areas immediately after final grading is completed on each area in the grading plan. Apply temporary stabilization measures on all graded areas when work is to be interrupted or delayed 30 working days or longer.

6.8 Surface Roughening

1. Roughen fill areas and areas to be seeded to shallow grooves by normal tilling, disking, harrowing, or use of a cultipacker/seeder.

2. Make grooves formed by such implements close together (less than 10 inches) and not less than 1 inch deep.

3. If tracked machinery is used, limit roughening to sandy soils to avoid undue compaction of the soil surface. Operate tracked machinery up and down the slope to leave horizontal depressions in the soil. Do not back-blade during the final grading operation.

Appendixes A and B

Drawings "Under Seperate Cover"

Appendix C

Permanent Stormwater Calculations

Storm Runoff Calculations

The runoff control system was designed to collect and control the peak discharge resulting from a 25-year, 24-hour storm to comply with NCDENR Solid Waste Regulations.

Precipitation values for the design storm events were obtained for the Central Carolina Tire Monofill Landfill Facility from Appendix 8.03 in the North Carolina Erosion and Sediment Control Planning and Design Manual and are listed below:

Storm Event	Precipitation
2-yr, 24-hr	3.75 inches
10-yr, 24-hr	5.6 inches
25-yr, 24-hr	6.6 inches
100-yr, 24-hr	8.2 inches

Permanent Ditch Calculations

Perimeter landfill stormwater ditches will be constructed and used to transport stormwater that falls on the landfill once the intermediate cover and final cover systems are installed. The ditches will also be used to capture stormwater falling on the undisturbed areas within the proposed landfill footprint prior to waste placement. The following calculations will illustrate that the ditches are sized adequately. Figure 1 illustrates the areas used to size the perimeter ditches.

All channels have been designed to accommodate the runoff from 25-year, 24-hour design storm that will fall on the newly constructed cap prior to vegetative growth. The CN used to calculate the runoff from each completed phase will be 86. This corresponds to a hydrologic soil group B, and a cover type/hydrologic condition of newly graded areas (pervious and no vegetation). Runoff is calculated using the SCS method in PondPack. The time of concentration was calculated using Kirpich (TN).

Perimeter Ditches Areas 1 and 2

Areas 1 and 2 will use perimeter landfill stormwater ditches to route stormwater from the final cover system down to the sedimentation pond 2. This ditch will be sized to handle a flow of 131.86 cfs from the final cover system before being captured in a drop inlet and transported to sediment pond 2. This flow will allow for 1 foot of free board in the ditch during the 25-yr, 24-hour storm event.

Channel:

Slope:	0.0052 ft/ft
Q_{des}	131.86 cfs
n	0.035

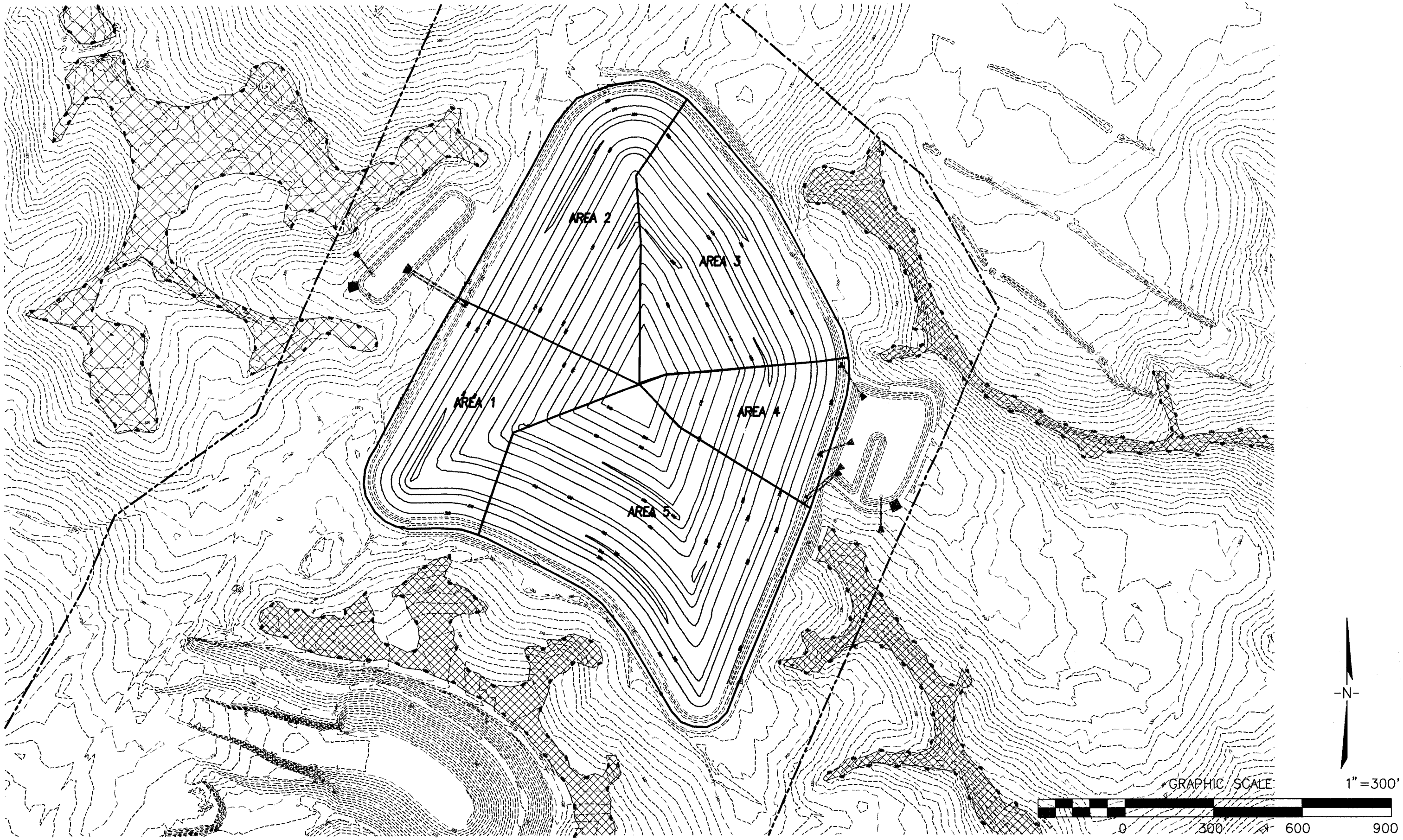
Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)
Bottom width	3 feet
Channel Depth	4 feet (minimum)

FlowMaster results are presented at the end of this summary.

Perimeter Ditch Area 3

Area 3 will use perimeter landfill stormwater ditches to route stormwater from the final cover system down to the sedimentation pond 1. This ditch will be sized to handle a flow of 74.76 cfs from the final cover system before being captured in a drop inlet and transported to sediment pond 1. This flow will allow for 1 foot of free board in the ditch during the 25-yr, 24-hour storm event.



K:\02\02-330\02334.01 LF Expansion\PERMIT TO CONSTRUCT APPLICATION\SW Permit to Construct\202334.01 Permit to Construct-REVISED SW Plan-(2-21-05).pro Tue Mar 22, 2005 10:43:51AM WITHERS & RAVENEL, ENGINEERS, PLANNERS & SURVEYORS

No.	Revision	Date	By

Designer W&R	Scale 1" = 300'
Drawn By JCI	Date 3/22/05
Checked By JCI	Job No. 202334.01

CENTRAL CAROLINA TIRE
PHASE 3 SCRAP TIRE MONOFILL LANDFILL

HARNETT COUNTY NORTH CAROLINA

PERMANENT DITCH DRAINAGE AREAS

WITHERS & RAVENEL
ENGINEERS | PLANNERS | SURVEYORS
111 MacKenan Drive Cary, North Carolina 27511
tel: 919-469-3340 www.withersravenel.com fax: 919-535-4545

Figure No.
1

Channel:

Slope:	0.0093 ft/ft
Q_{des}	74.76 cfs
n	0.035

Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)
Bottom width	3 feet
Channel Depth	4 feet (minimum)

FlowMaster results are presented at the end of this summary.

Perimeter Ditches Areas 4 and 5

Areas 4 and 5 will use perimeter landfill stormwater ditches to route stormwater from the final cover system down to the sedimentation pond 1. This ditch will be sized to handle a flow of 155.85 cfs from the final cover system before being captured in a drop inlet and transported to sediment pond 1. This flow will allow for 1 foot of free board in the ditch during the 25-yr, 24-hour storm event.

Channel:

Slope:	0.0050 ft/ft
Q_{des}	155.85 cfs
n	0.035

Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)
Bottom width	3 feet
Channel Depth	4 feet (minimum)

FlowMaster results are presented at the end of this summary.

Intermediate Ditches

Phase 3A Ditch Calculations

Intermediate landfill stormwater ditches will be constructed and used to transport stormwater that falls on the landfill during operations in phase 3A. The following calculations will illustrate that the ditches are sized adequately.

Phase 3A-1

Phase 3A-1 will use intermediate landfill stormwater ditches to route stormwater from phase 3A-1 to the permanent perimeter ditch. This ditch will be sized to handle a flow of 26.86 cfs during the 25-yr, 24-hour storm event.

Channel:

Slope:	0.0016 ft/ft
Q_{des}	26.86 cfs
n	0.035

Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)

Bottom width	5 feet
Channel Depth	2 feet (minimum)

FlowMaster results are presented at the end of this summary.

Phase 3A-2

Phase 3A-2 will use intermediate landfill stormwater ditches to route stormwater from phase 3A-2 to the permanent perimeter. This ditch will be sized to handle a flow of 35.08 cfs during the 25-yr, 24-hour storm event.

Channel:

Slope:	0.0126 ft/ft
Q_{des}	35.08 cfs
n	0.035

Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)
Bottom width	5 feet
Channel Depth	2 feet (minimum)

FlowMaster results are presented at the end of this summary.

Phase 3B Ditch Calculations

Intermediate landfill stormwater ditches will be constructed and used to transport stormwater that falls on the landfill during operations in phase 3B. The following calculations will illustrate that the ditches are sized adequately.

Phase 3B will use intermediate landfill stormwater ditches to route stormwater from phase 3B to the permanent perimeter. This ditch will be sized to handle a flow of 65.33 cfs during the 25-yr, 24-hour storm event.

Channel:

Slope:	0.0076 ft/ft
Q_{des}	65.33 cfs
n	0.035

Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)
Bottom width	5 feet
Channel Depth	2 feet (minimum)

FlowMaster results are presented at the end of this summary.

Phase 3C Ditch Calculations

Intermediate landfill stormwater ditches will be constructed and used to transport stormwater that falls on the landfill during operations in phase 3C. The following calculations will illustrate that the ditches are sized adequately.

Phase 3C will use intermediate landfill stormwater ditches to route stormwater from phase 3C to the permanent perimeter. This ditch will be sized to handle a flow of 20.44 cfs during the 25-yr, 24-hour storm event.

Channel:

Slope:	0.0075 ft/ft
Q_{des}	20.44 cfs
n	0.035

Based on FlowMaster Results the ditch will have the following dimensions:

Side slopes	3:1 (H:V)
Bottom width	5 feet
Channel Depth	2 feet (minimum)

FlowMaster results are presented at the end of this summary.

Drop Inlets

In order to route the stormwater off of the cap down to the sedimentation pond a series of drop inlets will be used. These drop inlets will be sized using a Neenah R-4215-C or equivalent grate and a 36-inch RCP. The following calculations were used to determine the number of grates for each pipe.

Drop Inlet Sediment Pond 1

For Sediment Pond 1 three separate drop inlets will be used. The first drop inlet will be a single drop inlet system and will capture and transport runoff from a part of Area 4 (in the routings this area will be 4A). The second drop inlet will be a single drop inlet system and will capture and transport runoff from Area 3. The third will consist of a double drop inlet system and will capture and transport runoff from the other part of Area 4 (4B in the routing) and Area 5.

Area 4A

Drop Inlet #1

Assume 30-inch pipe will have a minimum of 7 feet of head and a minimum slope of 0.50%, this will allow 53.36 cfs to pass through.

Assuming that the area of grate is 70% open and 30% grate, the total area open is:

$$\text{Area} = ((34\text{inch} * 34\text{inch})/144) * .70 = 5.62\ \text{ft}^2/\text{grate}$$

Want to pass 53.36 cfs per drop inlet w/ a maximum head above the grate equal to 3'.

Using orifice equation:

$$Q = C_d*A*(2*g*h)^{1/2}$$

Where:

C_d = Coefficient of discharge
A = Cross-sectional area of flow at the orifice opening (ft^2)
g = Acceleration of gravity (32.2 ft/sec^2)
h = Drving head (ft)

$$53.36\ \text{cfs} = .6*A*(2*32.2\ \text{ft/sec}^2*3\ \text{ft})^{1/2}$$

$$A = 6.40\ \text{ft}^2$$

Therefore need 2 drop inlet grates per pipe.

Area 3

Drop Inlet #2

Assume 36-inch pipe will have a minimum of 7 feet of head and a minimum slope of 0.50%, this will allow 87.13 cfs to pass through.

Assuming that the area of grate is 70% open and 30% grate, the total area open is:

$$\text{Area} = ((34\text{inch} * 34\text{inch})/144) * .70 = 5.62\ \text{ft}^2/\text{grate}$$

Want to pass 87.13 cfs per drop inlet w/ a maximum head above the grate equal to 3'.

Using orifice equation:

$$Q = C_d*A*(2*g*h)^{1/2}$$

Where:

C_d = Coefficient of discharge
A = Cross-sectional area of flow at the orifice opening (ft^2)
g = Acceleration of gravity (32.2 ft/sec^2)
h = Drving head (ft)

$$87.13\ \text{cfs} = .6*A*(2*32.2\ \text{ft/sec}^2*3\ \text{ft})^{1/2}$$

$$A = 10.45\ \text{ft}^2$$

Therefore need 2 drop inlet grates per pipe.

Area 4B & 5

Drop Inlet #3

Assume 36-inch pipe will have a minimum of 7 feet of head and a minimum slope of 0.5%, this will allow 72.43 cfs to pass through.

Assuming that the area of grate is 70% open and 30% grate, the total area open is:

$$\text{Area} = ((34\text{inch} * 34\text{inch})/144) * .70 = 5.62\ \text{ft}^2/\text{grate}$$

Want to pass 72.43 cfs per drop inlet w/ a maximum head above the grate equal to 3'.

Using orifice equation:

$$Q = C_d*A*(2*g*h)^{1/2}$$

Where:

C_d = Coefficient of discharge
A = Cross-sectional area of flow at the orifice opening (ft^2)
g = Acceleration of gravity (32.2 ft/sec^2)
h = Drving head (ft)

$$72.43\ \text{cfs} = .6*A*(2*32.2\ \text{ft/sec}^2*3\ \text{ft})^{1/2}$$

$A = 8.68\ ft^2$

Therefore need 2 drop inlet grates per pipe.

Drop Inlet Sediment Pond 2

For Sediment Pond 2 one double drop inlet will be used and will capture runoff from Area 1 and 2.

Area 1 & 2

Drop Inlet #4

Assume 36-inch pipe will have a minimum of 7 feet of head and a minimum slope of 1.66%, this will allow 87.93 cfs to pass through.

Assuming that the area of grate is 70% open and 30% grate, the total area open is:

$$Area = ((34inch * 34inch)/144) * .70 = 5.62\ ft^2/grate$$

Want to pass 87.93 cfs per drop inlet w/ a maximum head above the grate equal to 3'.

Using orifice equation:

$$Q = C_d*A*(2*g*h)^{1/2}$$

Where:

C_d = Coefficient of discharge
A = Cross-sectional area of flow at the orifice opening (ft^2)
g = Acceleration of gravity (32.2 ft/sec^2)
h = Drving head (ft)

$$87.93\ cfs = .6*A*(2*32.2\ ft/sec^2*3\ ft)^{1/2}$$

$$A = 10.54\ ft^2$$

Therefore need 2 drop inlet grates per pipe.

Number of Drop Inlets Required

Sediment Pond 1 Drop Inlet #1 Area 4A

Q = 44.88 cfs (Condition prior to waste placement, after waste placement a flow of 21.05 cfs will be captured)

Using Culvert Master a single drop inlet with a 30-inch RCP pipe will be required.

Sediment Pond 1 Drop Inlet #2 Area 3

Q = 74.76 cfs

Using Culvert Master a single drop inlet with a 36-inch RCP pipe will be required.

Sediment Pond 1 Drop Inlet #3 Areas 4B & 5

Q = 134.80 cfs

Using Culvert Master a double drop inlet with a 36-inch RCP pipe will be required.

Sediment Pond 2 Drop Inlet Areas 1 & 2

Q = 131.86 cfs

Using Culvert Master a double drop inlet with a 36-inch RCP pipe will be required.

Culvert Master results are attached at the end of this summary.

Locations of the drop inlets are illustrated on the engineering drawings.

Channel Lining Calculations

Using the flow rates obtained in the runoff calculations, the flow depth and flow velocity the desired channel lining was determined. The permissible velocity of the channel lining was then compared with the predicted channel velocity and unit shear stress, and if it was not satisfactory, a new lining was evaluated. Channel Lining calculations are provided in the following section. For each ditch specified below a temporary channel lining of North American Green Blanket, C125/C150BN with a maximum permissible shear stress of 2.25 psf was used. The permanent lining will be Tall Fescue with a permissible velocity of 4.5 ft/sec.

Ditch Area 1

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4 \text{ lbs/ft}^3 * 2.00 \text{ ft} * 0.0052 \text{ ft/ft}$$
$$T = 0.65 \text{ lbs/ft}^2$$

The velocity is 3.36 ft/sec as calculated by Flow Master.

Ditch Area 2

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4 \text{ lbs/ft}^3 * 2.13 \text{ ft} * 0.0054 \text{ ft/ft}$$
$$T = 0.72 \text{ lbs/ft}^2$$

The velocity is 3.56 ft/sec as calculated by Flow Master.

Ditch Area 3

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$T = 62.4\ lbs/ft^3 * 1.93\ ft * 0.0093\ ft/ft$
$T = 1.12\ lbs/ft^2$

The velocity is 4.40 ft/sec as calculated by Flow Master.

Ditch Area 4

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$T = 62.4\ lbs/ft^3 * 1.58\ ft * 0.0050\ ft/ft$
$T = 0.49\ lbs/ft^2$

The velocity is 2.88 ft/sec as calculated by Flow Master.

Ditch Area 5

Ditch Area 5 was broken up into two calculations due to a slope change. Area 5A captures stormwater from a portion of phase 3 and has a slope of 0.84%. Area 5 is used to calculate what flow is expected at the drop inlet. This area 5 uses a slope of 0.52%.

Permissible Shear Stress Area 5A

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$T = 62.4\ lbs/ft^3 * 1.95ft * 0.0084\ ft/ft$
$T = 1.02\ lbs/ft^2$

The velocity is 4.21 ft/sec as calculated by Flow Master.

Permissible Shear Stress Area 5

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4\ lbs/ft^3 * 2.71ft * 0.0052\ ft/ft$$
$$T = 0.88\ lbs/ft^2$$

The velocity is 4.01 ft/sec as calculated by Flow Master.

Ditch Phase 3A-1

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4\ lbs/ft^3 * 1.59\ ft * 0.0016\ ft/ft$$
$$T = 0.16\ lbs/ft^2$$

The velocity is 1.73 ft/sec as calculated by Flow Master.

Ditch Phase 3A-2

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4\ lbs/ft^3 * 1.08\ ft * 0.0126\ ft/ft$$
$$T = 0.85\ lbs/ft^2$$

The velocity is 3.94 ft/sec as calculated by Flow Master

Ditch Phase 3B

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4\ lbs/ft^3 * 1.68\ ft * 0.0076\ ft/ft$$
$$T = 0.80\ lbs/ft^2$$

The velocity is 3.89 ft/sec as calculated by Flow Master

Ditch Phase 3C

Permissible Shear Stress

$$T = y*d*s$$

Where:

y = unit weight of water, 62.4 lbs/ft^3
d = flow depth (ft)
s = channel gradient (ft/ft)

Based on FlowMaster Results attached to this summary following Permissible shear stress was calculated:

$$T = 62.4\ lbs/ft^3 * 0.93\ ft * 0.0075\ ft/ft$$
$$T = 0.44\ lbs/ft^2$$

The velocity is 2.81 ft/sec as calculated by Flow Master

Sediment Pond Calculations

A full routing for each sediment pond was performed using the PondPack program from Haestad Methods. The pond was modeled using design contours and outlet structures. Sediment ponds have adequate storage to detain the first 1-inch of runoff from the site.

The Sediment Pond routing is attached at the end of this report including minimum drain time and first flush volumes.

Storm Runoff Using SCS Peak Discharge Method

Sediment Pond 1

The drainage areas going to sediment pond 1 was divided into 3 drainage areas. The first drainage area is for Area 3, the second drainage area is Areas 4 & 5 and the third drainage area is for the sediment pond. Based on this the following input results were entered into PondPack.

Sediment Pond 1 was also routed to include 1 foot of sediment buildup on the bottom of the pond and continue to function properly without overtopping.

The following are input parameters entered into PondPack for Areas 3:

CN: 86
Area: 9.91 acres

Time of Concentration

Using Kirpich (TN)

Hydraulic Length	1888 ft
Slope	0.094 ft/ft

The time of concentration for Area 3 is 0.1076 hrs or 6.46 minutes.

The following are input parameters entered into PondPack for Areas 4&5:

CN: 86
Area: 20.66 acres

Time of Concentration

Using Kirpich (TN)

Hydraulic Length	2200 ft
Slope	0.076 ft/ft

The time of concentration for Areas 4&5 is 0.1314 hrs or 7.88 minutes.

The following are input parameters entered into PondPack for Pond Area:

CN: 98
Area: 2.82 acres

The time of concentration for the pond area is 5 minutes.

Pond Volume Inputs:

Elevation (ft)	Area (acres)
337.5	1.40
338	1.45
339	1.55
340	1.66
341	1.76
342	1.87
343	1.98
344	2.09
345	2.20

Riser/Barrel Inputs:

Riser:

Invert Elevation	341.5 ft
Diameter	5.0 ft
Orifice Coefficient	0.6
Weir Coefficient	3.0

Barrel:

Diameter	3.0 ft
Upstream Invert	337.54 ft
Downstream Invert	337.0 ft
Horizontal Length	106.31 ft
Type Pipe	CMP
Mannings n	0.022

Orifice Inputs:

Orifice 1:

# of Openings	10
Invert Elevation	337.5 ft
Diameter	0.167 ft
Orifice Coeff.	0.6

Orifice 2:

# of Openings	10
Invert Elevation	338.5 ft
Diameter	0.167 ft
Orifice Coeff.	0.6

Weir Inputs:

Weir:

Invert Elevation	343.0 ft
Weir Coefficient	3.0
Weir Length	20 ft

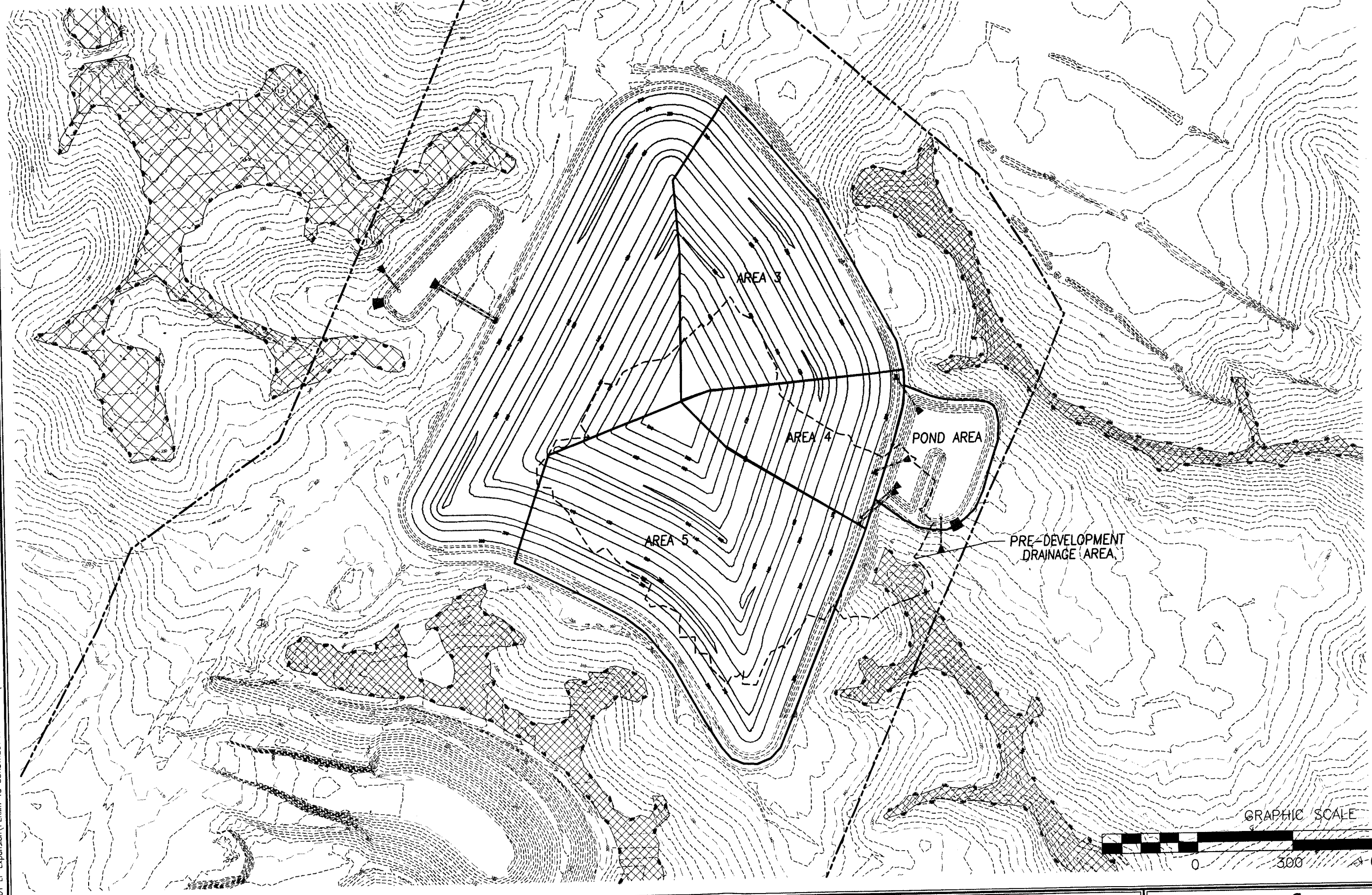
For pond with 1-foot of sediment buildup orifice 1 will not operate.

Detailed routings from PondPack are presented at the end of this summary.

Results Table

Structure #	Pre-Dev. Drainage Area (AC)	Post-Dev. Drainage Area (AC)	Pre-Dev. Flow Q 10 (cfs)	Post-Dev. Flow Q 10 (cfs)
Sediment Pond	19.42	33.39	44.36	13.46
Sediment Pond w/ 1 ft sediment buildup	-	-	44.36	30.66

Figure 2 shows the pre and post-development drainage areas for sediment pond 1.



No.	Revision	Date	By

Designer W&R	Scale 1" = 300'
Drawn By JCI	Date 3/22/05
Checked By JCI	Job No. 202334.01

CENTRAL CAROLINA TIRE
PHASE 3 SCRAP TIRE MONOFILL LANDFILL

HARNETT COUNTY NORTH CAROLINA

SEDIMENT POND #1
PRE & POST-DEVELOPMENT
DRAINAGE AREAS

WITHERS & RAVENEL
ENGINEERS | PLANNERS | SURVEYORS
111 MacKenan Drive Cary, North Carolina 27511
tel: 919-469-3340 www.withersravenel.com fax: 919-535-4545

Figure No. 2

K:\02\02-330\02334.01 LF Expansion\PERMIT TO CONSTRUCT APPLICATION\SW Permit to Construct\202334.01 Permit to Construct-REVISED SW Plan-(2-21-05).pro Tue Mar 22, 2005 10:43:47AM WITHERS & RAVENEL, ENGINEERS, PLANNERS & SURVEYOR

Sedimentology

Trapping Efficiency

Based on the studies performed by Barfield and Clark a trapping efficiency greater than 75% can be calculated as follows:

$$A = 0.01*q$$

Where:

A = basin surface area at crest of principal spillway
q= peak inflow from 2-year storm

$$A = 0.01 * 115.43 \text{ cfs} = 1.15 \text{ acres}$$

The pond has a surface area of 1.82 acres at the principal spillway height therefore the pond will meet the trapping efficiency of 75% during the 2-year storm event.

Sediment Storage Volume

The required sediment storage volume below the crest of the principal spillway is as follows:

$$\text{Sediment Storage Volume} = 1800 \text{ ft}^3/\text{acre} * \text{drainage area}$$

$$= 1800 \text{ ft}^3/\text{acre} * 33.39 \text{ acres} = 60{,}102 \text{ ft}^3$$

The current pond has a storage volume of 280,125 ft^3. Therefore there is enough storage below the riser to help ensure trapping efficiency.

Sediment Pond Dissipator Calculations

Riprap outlet protection or energy dissipators will be installed at the location the sediment pond discharges. The purpose for using the riprap outlet protection is to convert high-velocity, concentrated pipe flow into low-velocity, open channel flow. All energy dissipators will be sized based on the flows from the 25-year, 24-hour design storm. The following is the method used for sizing the energy dissipators.

From the North Carolina Erosion and Sediment Control Planning and Design Manual the following steps were used to size the energy dissipators.

Step 1: Determine the tailwater depth from channel characteristics below the pipe outlet for the design capacity of the pipe.

Step 2: Determine the d_{50} riprap size and minimum apron length (L_a) from either figure 8.06a or 8.06b depending on the results from step 1.

Step 3: Determine the apron width at the pipe outlet, the apron shape, and the apron width at the outlet end from the same figure used in step 2.

$$\text{Apron Width at Pipe Outlet} = 3 \times D_0$$

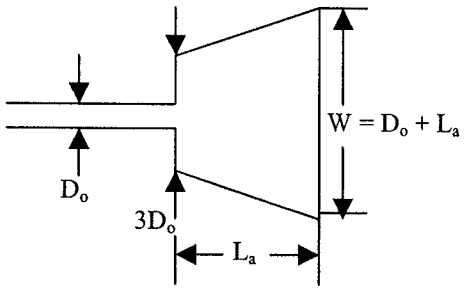
$$\text{Apron Width at Outlet End} = D_0 + L_a$$

Step 4: Determine the maximum stone diameter:

$$d_{max} = 1.5 \times d_{50}$$

Step 5: Determine the apron thickness:

$$\text{Apron thickness} = 1.5 \times d_{max}$$



D_0 = diameter of outlet pipe
L_a = length of apron
W = width of end of apron

Summary Table

Energy Dissipator #	Pipe Diameter (D_o) (in)	$Q_{25\text{-}yr}$ (cfs)	d_{50} (ft)	d_{max} (ft)	Apron Thickness (ft)	L_a (ft)	Width (ft)	$3D_0$ (ft)
1	36	50.03	0.5	0.75	1.125	20	23	9
2	30	53.36	0.7	1.05	1.575	24	26.5	7.5
3	36	87.13	1.0	1.5	2.25	30	33	9
4*	two-36	72.44	0.8	1.2	1.8	26	29	9

* Size of Energy Dissipator for each pipe

Buoyancy Force Under Riser/Barrel Structure

The following calculations will be used to verify that the riser will not float when placed in the pond on top of the proposed base.

The 5'-diam CMP riser is 4' tall. Assume $\gamma_{concrete}$ = 150 pcf. The proposed base for the riser is 7'*7'*2'.

$F_b = [\pi*(5.0')^2/4]*4'*62.4$ pcf = 4,900 lbs

W = (7'*7'*2')*150 pcf = 14,700 lb

FS = 14,700 lbs/4,900 lbs = 3

A table summarizing the outlet structures is presented on the engineering drawings.

Sediment Pond 2

The drainage areas going to sediment pond 2 was divided into 3 drainage areas. The first drainage area is for Area 1, the second drainage area is Area 2 and the third drainage area is for the sediment pond. Based on this the following input results were entered into PondPack.

Sediment Pond 2 was also routed to include 1 foot of sediment buildup on the bottom of the pond and continue to function properly without overtopping.

The following are input parameters entered into PondPack for Areas 1:

CN: 86
Area: 8.02 acres

Time of Concentration

The time of concentration for Area 1 is 5 minutes.

The following are input parameters entered into PondPack for Area 2:

CN: 86
Area: 9.46 acres

Time of Concentration

The time of concentration for Area 2 is 5 minutes.

The following are input parameters entered into PondPack for Pond Area:

CN: 98
Area: 1.89 acres

The time of concentration for the pond area is 5 minutes.

Pond Volume Inputs:

Elevation (ft)	Area (acres)
338	0.80
342	1.08
344	1.23
345	1.31

Riser/Barrel Inputs:

Riser:

Invert Elevation	343.0 ft
Diameter	5.0 ft
Orifice Coefficient	0.6
Weir Coefficient	3.0

Barrel:

Diameter	2.5 ft
Upstream Invert	338.04 ft
Downstream Invert	337.62 ft
Horizontal Length	83.38 ft
Type Pipe	CMP
Mannings n	0.022

Orifice Inputs:

Orifice 1:

# of Openings	10
Invert Elevation	338.04 ft

Diameter	0.167 ft
Orifice Coeff.	0.6

Orifice 2:

# of Openings	10
Invert Elevation	339.0 ft
Diameter	0.167 ft
Orifice Coeff.	0.6

Weir Inputs:

Weir:

Invert Elevation	343.5 ft
Weir Coefficient	3.0
Weir Length	30 ft

For pond with 1-foot of sediment buildup orifice 1 will not operate.

Detailed routings from PondPack are presented at the end of this summary.

Results Table

Structure #	Pre-Dev. Drainage Area (AC)	Post-Dev. Drainage Area (AC)	Pre-Dev. Flow Q 10 (cfs)	Post-Dev. Flow Q 10 (cfs)
Sediment Pond	1.63	19.37	6.09	3.90
Sediment Pond w/ 1-ft Sediment buildup	-	-	6.09	5.80

Figure 3 shows the pre and post-development drainage areas for sediment pond 2.

Sedimentology

Trapping Efficiency

Based on the studies performed by Barfield and Clark a trapping efficiency greater than 75% can be calculated as follows:

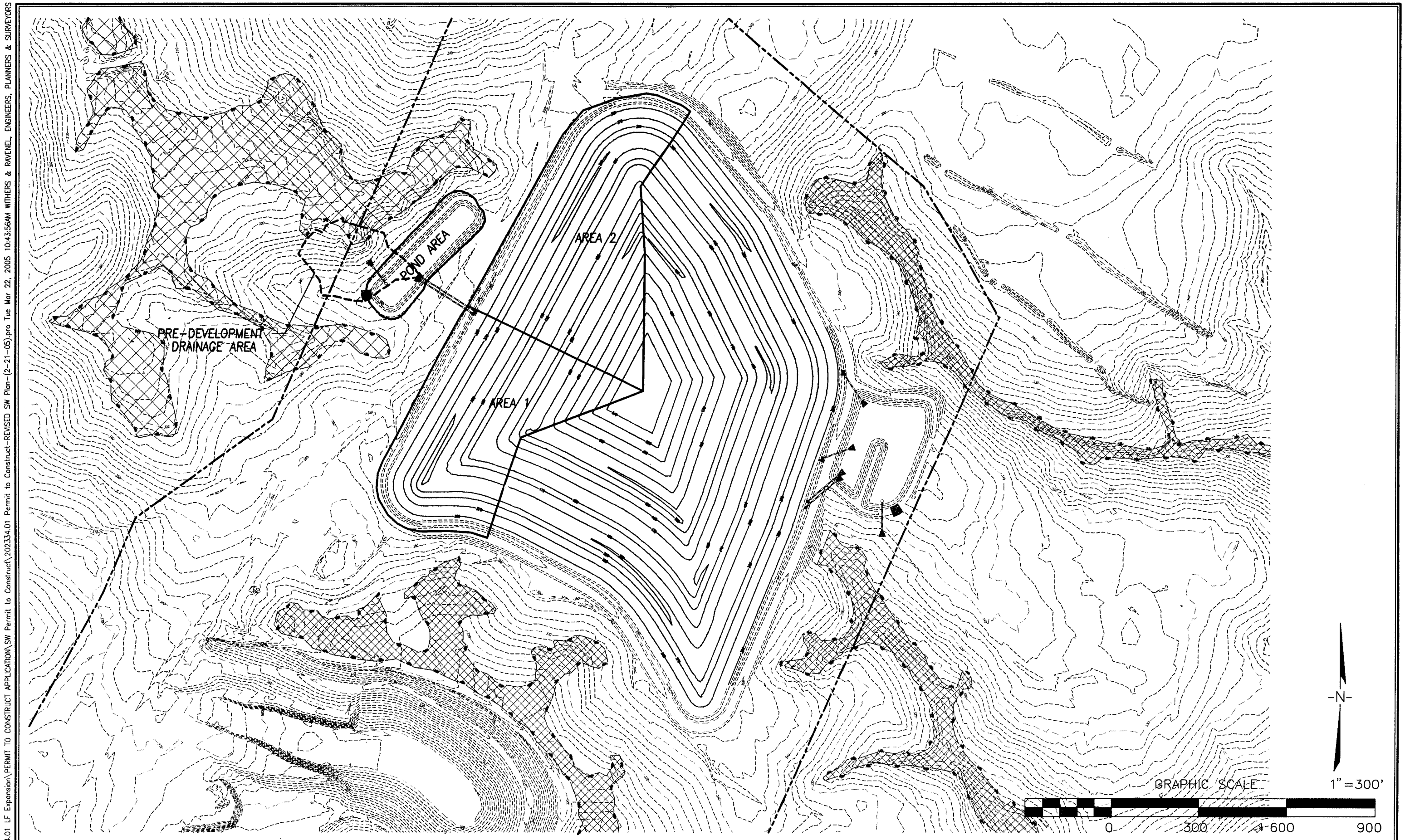
$$A = 0.01*q$$

Where:

A = basin surface area at crest of principal spillway
q= peak inflow from 2-year storm

$$A = 0.01 * 73.80 \text{ cfs} = 0.74 \text{ acres}$$

The pond has a surface area of 1.16 acres at the principal spillway height therefore the pond will meet the trapping efficiency of 75% during the 2-year storm event.



No.	Revision	Date	By

Designer W&R	Scale 1" = 300'
Drawn By JCI	Date 3/22/05
Checked By JCI	Job No. 202334.01

CENTRAL CAROLINA TIRE
PHASE 3 SCRAP TIRE MONOFILL LANDFILL

HARNETT COUNTY NORTH CAROLINA

SEDIMENT POND #2
PRE & POST-DEVELOPMENT
DRAINAGE AREAS

WITHERS & RAVENEL
ENGINEERS | PLANNERS | SURVEYORS
111 MacKenan Drive Cary, North Carolina 27511
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Figure No. 3

K:\02\02-330\02334.01 LF Expansion\PERMIT TO CONSTRUCT APPLICATION\SW Permit to Construct\202334.01 Permit to Construct-REVISED SW Plan-(2-21-05).pro Tue Mar 22, 2005 10:43:56AM WITHERS & RAVENEL, ENGINEERS, PLANNERS & SURVEYORS

Sediment Storage Volume

The required sediment storage volume below the crest of the principal spillway is as follows:

$$\text{Sediment Storage Volume} = 1800\ ft^3/acre * \text{drainage area}$$

$$= 1800\ ft^3/acre * 19.37\ acres = 34{,}866\ ft^3$$

The current pond has a storage volume of 212,301 ft^3. Therefore there is enough storage below the riser to help ensure trapping efficiency.

Sediment Pond Dissipator Calculations

Riprap outlet protection or energy dissipators will be installed at the location the sediment pond discharges. The purpose for using the riprap outlet protection is to convert high-velocity, concentrated pipe flow into low-velocity, open channel flow. All energy dissipators will be sized based on the flows from the 25-year, 24-hour design storm. The following is the method used for sizing the energy dissipators.

From the North Carolina Erosion and Sediment Control Planning and Design Manual the following steps were used to size the energy dissipators.

Step 1: Determine the tailwater depth from channel characteristics below the pipe outlet for the design capacity of the pipe.

Step 2: Determine the d_{50} riprap size and minimum apron length (L_a) from either figure 8.06a or 8.06b depending on the results from step 1.

Step 3: Determine the apron width at the pipe outlet, the apron shape, and the apron width at the outlet end from the same figure used in step 2.

$$\text{Apron Width at Pipe Outlet} = 3 \times D_0$$

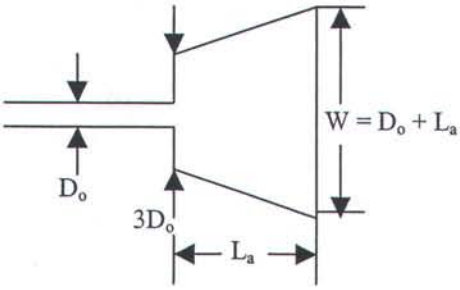
$$\text{Apron Width at Outlet End} = D_0 + L_a$$

Step 4: Determine the maximum stone diameter:

$$d_{max} = 1.5 \times d_{50}$$

Step 5: Determine the apron thickness:

$$\text{Apron thickness} = 1.5 \times d_{max}$$



D_0 = diameter of outlet pipe
L_a = length of apron
W = width of end of apron

Summary Table

Energy Dissipator #	Pipe Diameter (D_o) (in)	$Q_{25\text{-}yr}$ (cfs)	d_{50} (ft)	d_{max} (ft)	Apron Thickness (ft)	L_a (ft)	Width (ft)	$3D_0$ (ft)
5	30	18.12	0.5	0.75	1.125	20	23	9
6*	Two-36	87.93	1.0	1.5	2.25	30	33	9

* Size of Energy Dissipator for each pipe

Buoyancy Force Under Riser/Barrel Structure

The following calculations will be used to verify that the riser will not float when placed in the pond on top of the proposed base.

The 5'-diam CMP riser is 5' tall. Assume $\gamma_{concrete}$ = 150 pcf. The proposed base for the riser is 7'*7'*2'.

$F_b = [\pi*(5.0')^2/4]*5'*62.4$ pcf = 6,126 lbs

W = (7'*7'*2')*150 pcf = 14,700 lb

FS = 14,700 lbs/6,126 lbs = 2.4

A table summarizing the outlet structures is presented on the engineering drawings.

Downdrain Sizing

Downdrains and terrace ditches will be used to capture stormwater falling onto the closed landfill. All downdrains and terrace ditches have been sized as follows:

Terrace Ditch

The terrace dicth was sized to handle a maximum acre coming to it before being captured in a downdrain. The following is the calculation used to get the maximum acreage:

Ditch will have the following dimensions:

Side slopes	3.5:1 (H:V)
Right Side slope	3:1 (H:V)
Bottom width	3 feet
Channel Depth	1.5 feet

FlowMaster results calculate that the ditch will be able to convey 32.92 cfs. The depth of 1.5 feet will allow the ditch to have 0.5 feet of freeboard. The actual depth of the ditch will be 2 feet. FlowMaster results are presented at the end of this summary.

Once the flow was calculated the rational method was used to calculate the maximum area the ditch can capture.

$$Q = C*I*A$$

Where:

Q = desired flow (cfs)
C = rational coefficient corresponding to a CN = 86Based on a CN of 86 for newly graded areas

(converted using C_{hrm} = .0193 CN - .91 from Elements of Urban Stormwater Design by H. Rooney Malcom, P.E.)

I = Rainfall intensity (in/hr) (Using rainfall intensity from Figure 8.03d for Raleigh for the 10-yr storm with a Tc = 5minutes

A = area that can be captured

32.92 cfs = 0.75*8 in/hr*A

A = 5.49 acres

Downdrains will be placed such that each terrace ditch will have no greater than 5.49 acres draining to it.

Downdrain Sizing

Downdrains were sized using FlowMaster assuming open channel flow. Each downdrain will be 24-inches in diameter and will be placed such that the maximum flow going to anyone downdrain will not exceed 130.99 cfs. The following was entered into FlowMaster:

Mannings Coefficient	0.012
Channel Slope	0.2857 ft/ft
Diameter	24 in.

FlowMaster results are presented at the end of this summary.

Temporary Stormwater Calculations

Temporary Storm Runoff Calculations

Temporary erosion and sedimentation control measures will be used during the construction of each phase of the landfill. Temporary sedimentation traps and diversion ditches will be used. Each sediment trap will be limited to a maximum drainage area of 5-acres.

The following steps will be used to size the sediment traps for each phase:

Storage Capacity

Minimum Volume of the trap = 1800 ft^3/acre

Trap Efficiency

In order to get a trapping efficiency of 75% for the 2-yr storm event the following equation was used to size the sediment traps:

Surface area at design flow (acres) = (0.01)*peak inflow rate for the 2-yr, 24-hour storm (cfs)

The surface area will correspond to the surface area at the top of the weir. The design flow may be higher than the top of the weir but this will yield a better trapping efficiency. If the design storm when routed is lower than the top of the weir for the 2-yr, 24-hour storm it is assumed trapping efficiency will be met since all the flow is filtering through the rock.

Embankment

Maximum height of embankment = 5 ft

Side Slopes

2:1 or flatter

Depth

Crest of spillway a minimum of 1.5 ft below settled top of embankment

Weir Length and Depth

See Table 6.60a in the North Carolina Erosion and Sediment Control Planning and Design Manual

Phase 3A

Due to the proximity of wetlands and the topography of the existing grades, silt fence and 4 sediment traps will be used to prevent sediment from leaving site prior to stabilization. See the attached engineering drawings for the locations of each device and construction sequence.

Trap #	Drainage Area (acres)	Storage Volume Required (ft^3)	Actual Storage Volume at top of weir (ft^3)	Trap Dimensions at top of weir (L x W) (ft x ft)	Overall Trap Dimensions (L x W) (ft x ft)	Weir Length (ft)	Rock Dam Height (ft)
1	1.70	3,060	5,934	80 x 30	86 x 36	10	3.5
2	4.59	8,262	21,006	120 x 60	126 x 66	12	3.5
3	1.10	1,980	5,934	80 x 30	86 x 36	10	3.5

4	4.39	7,902	21,006	120 x 60	126 x 66	12	3.5

Trap Efficiency

After routing each of the sediment traps, all of the traps filter the 2-yr, 24-hour storm through the rock dam therefore it is assumed trapping efficiency of 75% or greater will be met for the 2-yr, 24-hour storm event.

Phase 3B

Due to the proximity of wetlands and the topography of the existing grades, silt fence and one sediment trap will be used to prevent sediment from leaving site prior to stabilization. See the attached engineering drawings for the locations of each device and the construction sequence.

Trap #	Drainage Area (acres)	Storage Volume Required (ft^3)	Actual Storage Volume at top of weir (ft^3)	Trap Dimensions at top of weir (L x W) (ft x ft)	Overall Trap Dimensions (L x W) (ft x ft)	Weir Length (ft)	Rock Dam Height (ft)
5	1.57	2,826	5,934	80 x 30	86 x 36	10	3.5

Trap Efficiency

After routing each of the sediment traps, all of the traps filter the 2-yr, 24-hour storm through the rock dam therefore it is assumed trapping efficiency of 75% or greater will be met for the 2-yr, 24-hour storm event.

Phase 3C

Due to the proximity of wetlands and the topography of the existing grades, silt fence will be used to prevent sediment from leaving site prior to stabilization. See the attached engineering drawings for the locations of each device and construction sequence.

Phase 3D

Silt Fence and 4 sediment trap will be used to prevent sediment from leaving site prior to stabilization. See the attached engineering drawings for the locations of each device and construction sequence.

Trap #	Drainage Area (acres)	Storage Volume Required (ft^3)	Actual Storage Volume at top of weir (ft^3)	Trap Dimensions at top of weir (L x W) (ft x ft)	Overall Trap Dimensions (L x W) (ft x ft)	Weir Length (ft)	Rock Dam Height (ft)
6	1.70	3,060	5,934	80 x 30	86 x 36	10	3.5
7	4.52	8,136	21,006	120 x 60	126 x 66	12	3.5
8	2.25	4,050	9,639	90 x 40	96 x 46	10	3.5
9	2.18	3,924	9,639	90 x 40	96 x 46	10	3.5

Trap Efficiency

After routing each of the sediment traps, all of the traps filter the 2-yr, 24-hour storm through the rock dam therefore it is assumed trapping efficiency of 75% or greater will be met for the 2-yr, 24-hour storm event.

A routing summary from PondPack is attached to the end of this summary. The CN used to calculate the runoff will be 86. This corresponds to a hydrologic soil group B, and a cover type/hydrologic condition of newly graded areas (pervious and no vegetation). Runoff is calculated using the SCS method in PondPack. The time of concentration used is 5 minutes.

MASTER DESIGN STORM SUMMARY

Network Storm Collection: Harnett County

Return Event	Total Depth in	Rainfall Type	RNF ID
2	3.8000	Synthetic Curve	TypeII 24hr
10	5.6000	Synthetic Curve	TypeII 24hr
25	6.6000	Synthetic Curve	TypeII 24hr
100	8.2000	Synthetic Curve	TypeII 24hr

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
*OUT 1	JCT	2	.327		12.0600	2.94		
*OUT 1	JCT	10	.556		11.9700	9.26		
*OUT 1	JCT	25	.686		11.9500	12.13		
*OUT 1	JCT	100	.903		11.9400	16.05		
*OUT 2	JCT	2	.804		12.1400	3.02		
*OUT 2	JCT	10	1.400		12.0200	21.04		
*OUT 2	JCT	25	1.741		11.9800	29.01		
*OUT 2	JCT	100	2.316		11.9600	40.44		
*OUT 3	JCT	2	.209		12.0700	1.62		
*OUT 3	JCT	10	.357		12.0500	3.87		
*OUT 3	JCT	25	.440		12.0100	6.45		
*OUT 3	JCT	100	.580		11.9600	9.69		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
*OUT 4	JCT	2	.765		12.1400	2.77		
*OUT 4	JCT	10	1.335		12.0300	19.44		
*OUT 4	JCT	25	1.661		11.9900	27.22		
*OUT 4	JCT	100	2.210		11.9600	38.44		
*OUT 40	JCT	2	.399		12.0900	2.15		
*OUT 40	JCT	10	.687		12.0200	10.02		
*OUT 40	JCT	25	.850		11.9800	13.97		
*OUT 40	JCT	100	1.125		11.9500	19.78		
*OUT 5	JCT	2	.301		12.0600	2.64		
*OUT 5	JCT	10	.513		11.9800	8.20		
*OUT 5	JCT	25	.633		11.9500	11.01		
*OUT 5	JCT	100	.833		11.9400	14.78		
*OUT 6	JCT	2	.327		12.0600	2.94		
*OUT 6	JCT	10	.556		11.9700	9.26		
*OUT 6	JCT	25	.686		11.9500	12.13		
*OUT 6	JCT	100	.903		11.9400	16.05		
*OUT 7	JCT	2	.790		12.1400	2.93		
*OUT 7	JCT	10	1.378		12.0200	20.45		
*OUT 7	JCT	25	1.713		11.9800	28.38		
*OUT 7	JCT	100	2.279		11.9600	39.75		
*OUT 8	JCT	2	.413		12.0900	2.26		
*OUT 8	JCT	10	.710		12.0200	10.58		
*OUT 8	JCT	25	.879		11.9700	14.67		
*OUT 8	JCT	100	1.163		11.9500	20.50		
TRAP #1	AREA	2	.335		11.9200	6.28		
TRAP #1	AREA	10	.571		11.9200	10.48		
TRAP #1	AREA	25	.706		11.9200	12.82		
TRAP #1	AREA	100	.924		11.9200	16.55		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
TRAP #1	IN	POND	2	.335		11.9200	6.28		
TRAP #1	IN	POND	10	.571		11.9200	10.48		
TRAP #1	IN	POND	25	.706		11.9200	12.82		
TRAP #1	IN	POND	100	.924		11.9200	16.55		
TRAP #1	OUT	POND	2	.327		12.0600	2.94	3.43	.118
TRAP #1	OUT	POND	10	.556		11.9700	9.26	3.85	.140
TRAP #1	OUT	POND	25	.686		11.9500	12.13	3.95	.146
TRAP #1	OUT	POND	100	.903		11.9400	16.05	4.07	.153
TRAP #2		AREA	2	.905		11.9200	16.94		
TRAP #2		AREA	10	1.542		11.9200	28.31		
TRAP #2		AREA	25	1.906		11.9200	34.62		
TRAP #2		AREA	100	2.496		11.9200	44.68		
TRAP #2	IN	POND	2	.905		11.9200	16.94		
TRAP #2	IN	POND	10	1.542		11.9200	28.31		
TRAP #2	IN	POND	25	1.906		11.9200	34.62		
TRAP #2	IN	POND	100	2.496		11.9200	44.68		
TRAP #2	OUT	POND	2	.804		12.1400	3.02	3.27	.439
TRAP #2	OUT	POND	10	1.400		12.0200	21.04	4.12	.581
TRAP #2	OUT	POND	25	1.741		11.9800	29.01	4.30	.612
TRAP #2	OUT	POND	100	2.316		11.9600	40.44	4.53	.653
TRAP #3		AREA	2	.217		11.9200	4.06		
TRAP #3		AREA	10	.369		11.9200	6.78		
TRAP #3		AREA	25	.457		11.9200	8.30		
TRAP #3		AREA	100	.598		11.9200	10.71		
TRAP #3	IN	POND	2	.217		11.9200	4.06		
TRAP #3	IN	POND	10	.369		11.9200	6.78		
TRAP #3	IN	POND	25	.457		11.9200	8.30		
TRAP #3	IN	POND	100	.598		11.9200	10.71		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
TRAP #3	OUT POND	2	.209		12.0700	1.62	2.70	.083
TRAP #3	OUT POND	10	.357		12.0500	3.87	3.58	.126
TRAP #3	OUT POND	25	.440		12.0100	6.45	3.73	.134
TRAP #3	OUT POND	100	.580		11.9600	9.69	3.86	.141
TRAP #4	AREA	2	.865		11.9200	16.20		
TRAP #4	AREA	10	1.474		11.9200	27.07		
TRAP #4	AREA	25	1.822		11.9200	33.11		
TRAP #4	AREA	100	2.387		11.9200	42.73		
TRAP #4	IN POND	2	.865		11.9200	16.20		
TRAP #4	IN POND	10	1.474		11.9200	27.07		
TRAP #4	IN POND	25	1.822		11.9200	33.11		
TRAP #4	IN POND	100	2.387		11.9200	42.73		
TRAP #4	OUT POND	2	.765		12.1400	2.77	3.17	.422
TRAP #4	OUT POND	10	1.335		12.0300	19.44	4.08	.574
TRAP #4	OUT POND	25	1.661		11.9900	27.22	4.26	.605
TRAP #4	OUT POND	100	2.210		11.9600	38.44	4.49	.646
TRAP #5	AREA	2	.310		11.9200	5.80		
TRAP #5	AREA	10	.527		11.9200	9.68		
TRAP #5	AREA	25	.652		11.9200	11.84		
TRAP #5	AREA	100	.854		11.9200	15.28		
TRAP #5	IN POND	2	.310		11.9200	5.80		
TRAP #5	IN POND	10	.527		11.9200	9.68		
TRAP #5	IN POND	25	.652		11.9200	11.84		
TRAP #5	IN POND	100	.854		11.9200	15.28		
TRAP #5	OUT POND	2	.301		12.0600	2.64	3.29	.110
TRAP #5	OUT POND	10	.513		11.9800	8.20	3.81	.138
TRAP #5	OUT POND	25	.633		11.9500	11.01	3.91	.144
TRAP #5	OUT POND	100	.833		11.9400	14.78	4.04	.151

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
TRAP #6	AREA	2	.335		11.9200	6.28		
TRAP #6	AREA	10	.571		11.9200	10.48		
TRAP #6	AREA	25	.706		11.9200	12.82		
TRAP #6	AREA	100	.924		11.9200	16.55		
TRAP #6	IN POND	2	.335		11.9200	6.28		
TRAP #6	IN POND	10	.571		11.9200	10.48		
TRAP #6	IN POND	25	.706		11.9200	12.82		
TRAP #6	IN POND	100	.924		11.9200	16.55		
TRAP #6	OUT POND	2	.327		12.0600	2.94	3.43	.118
TRAP #6	OUT POND	10	.556		11.9700	9.26	3.85	.140
TRAP #6	OUT POND	25	.686		11.9500	12.13	3.95	.146
TRAP #6	OUT POND	100	.903		11.9400	16.05	4.07	.153
TRAP #7	AREA	2	.891		11.9200	16.68		
TRAP #7	AREA	10	1.518		11.9200	27.88		
TRAP #7	AREA	25	1.876		11.9200	34.10		
TRAP #7	AREA	100	2.458		11.9200	43.99		
TRAP #7	IN POND	2	.891		11.9200	16.68		
TRAP #7	IN POND	10	1.518		11.9200	27.88		
TRAP #7	IN POND	25	1.876		11.9200	34.10		
TRAP #7	IN POND	100	2.458		11.9200	43.99		
TRAP #7	OUT POND	2	.790		12.1400	2.93	3.24	.433
TRAP #7	OUT POND	10	1.378		12.0200	20.45	4.11	.578
TRAP #7	OUT POND	25	1.713		11.9800	28.38	4.29	.610
TRAP #7	OUT POND	100	2.279		11.9600	39.75	4.51	.651
TRAP #8	AREA	2	.444		11.9200	8.31		
TRAP #8	AREA	10	.756		11.9200	13.88		
TRAP #8	AREA	25	.934		11.9200	16.97		
TRAP #8	AREA	100	1.223		11.9200	21.90		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
TRAP #8	IN	POND	2	.444		11.9200	8.31		
TRAP #8	IN	POND	10	.756		11.9200	13.88		
TRAP #8	IN	POND	25	.934		11.9200	16.97		
TRAP #8	IN	POND	100	1.223		11.9200	21.90		
TRAP #8	OUT	POND	2	.413		12.0900	2.26	3.09	.198
TRAP #8	OUT	POND	10	.710		12.0200	10.58	3.90	.265
TRAP #8	OUT	POND	25	.879		11.9700	14.67	4.03	.276
TRAP #8	OUT	POND	100	1.163		11.9500	20.50	4.20	.292
TRAP #9		AREA	2	.430		11.9200	8.05		
TRAP #9		AREA	10	.732		11.9200	13.44		
TRAP #9		AREA	25	.905		11.9200	16.44		
TRAP #9		AREA	100	1.185		11.9200	21.22		
TRAP #9	IN	POND	2	.430		11.9200	8.05		
TRAP #9	IN	POND	10	.732		11.9200	13.44		
TRAP #9	IN	POND	25	.905		11.9200	16.44		
TRAP #9	IN	POND	100	1.185		11.9200	21.22		
TRAP #9	OUT	POND	2	.399		12.0900	2.15	3.03	.192
TRAP #9	OUT	POND	10	.687		12.0200	10.02	3.88	.263
TRAP #9	OUT	POND	25	.850		11.9800	13.97	4.01	.275
TRAP #9	OUT	POND	100	1.125		11.9500	19.78	4.18	.290

FlowMaster Ditch Design Results

LF Ditch Area 1
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Area 1
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	005200	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	60.50	cfs

Results		
Depth	2.00	ft
Flow Area	18.0	ft²
Wetted Perim	15.65	ft
Top Width	15.00	ft
Critical Depth	1.48	ft
Critical Slope	0.019287	ft/ft
Velocity	3.36	ft/s
Velocity Head	0.18	ft
Specific Energ	2.18	ft
Froude Numb	0.54	
Flow Type	Subcritical	

LF Ditch Area 2
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Area 2
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	005400	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	71.36	cfs

Results		
Depth	2.13	ft
Flow Area	20.1	ft²
Wetted Perim	16.50	ft
Top Width	15.81	ft
Critical Depth	1.61	ft
Critical Slope	0.018866	ft/ft
Velocity	3.56	ft/s
Velocity Head	0.20	ft
Specific Energ	2.33	ft
Froude Numb	0.56	
Flow Type	Subcritical	

LF Ditch Area 3
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Area 3
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	009300	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	74.76	cfs

Results		
Depth	1.93	ft
Flow Area	17.0	ft²
Wetted Perim	15.21	ft
Top Width	14.58	ft
Critical Depth	1.65	ft
Critical Slope	0.018752	ft/ft
Velocity	4.40	ft/s
Velocity Head	0.30	ft
Specific Energ	2.23	ft
Froude Numb	0.72	
Flow Type	Subcritical	

LF Ditch Area 4
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Area 4
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	005000	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	35.23	cfs

Results		
Depth	1.58	ft
Flow Area	12.2	ft²
Wetted Perim	12.99	ft
Top Width	12.48	ft
Critical Depth	1.13	ft
Critical Slope	0.020740	ft/ft
Velocity	2.88	ft/s
Velocity Head	0.13	ft
Specific Energ	1.71	ft
Froude Numb	0.51	
Flow Type	Subcritical	

LF Ditch Area 5A
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Area 5A
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	008400	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	72.95	cfs

Results		
Depth	1.95	ft
Flow Area	17.3	ft²
Wetted Perim	15.35	ft
Top Width	14.72	ft
Critical Depth	1.63	ft
Critical Slope	0.018812	ft/ft
Velocity	4.21	ft/s
Velocity Head	0.28	ft
Specific Energ	2.23	ft
Froude Numb	0.69	
Flow Type	Subcritical	

LF Ditch Area 5
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Area 5
Flow Element	Trapezoidal Cha
Method	Manning's Formı
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	005200	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft
Discharge	120.62	cfs

Results		
Depth	2.71	ft
Flow Area	30.1	ft²
Wetted Perim	20.12	ft
Top Width	19.25	ft
Critical Depth	2.07	ft
Critical Slope	0.017596	ft/ft
Velocity	4.01	ft/s
Velocity Head	0.25	ft
Specific Energ	2.96	ft
Froude Numb	0.56	
Flow Type	Subcritical	

LF Phase 3A-1 Ditch
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Ph3A-1
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	001600	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	5.00	ft
Discharge	26.86	cfs

Results		
Depth	1.59	ft
Flow Area	15.5	ft²
Wetted Perim	15.04	ft
Top Width	14.53	ft
Critical Depth	0.81	ft
Critical Slope	0.021774	ft/ft
Velocity	1.73	ft/s
Velocity Head	0.05	ft
Specific Energ	1.63	ft
Froude Numb	0.30	
Flow Type	Subcritical	

LF Phase 3A-2 Ditch
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Ph3A-2
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	012600	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	5.00	ft
Discharge	35.08	cfs

Results		
Depth	1.08	ft
Flow Area	8.9	ft²
Wetted Perim	11.83	ft
Top Width	11.48	ft
Critical Depth	0.95	ft
Critical Slope	0.020924	ft/ft
Velocity	3.94	ft/s
Velocity Head	0.24	ft
Specific Energ	1.32	ft
Froude Numb	0.79	
Flow Type	Subcritical	

Terrace Ditches
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	Down Drains
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Discharge

Input Data		
Mannings Coeffic	0.035	
Channel Slope	005000	ft/ft
Depth	1.50	ft
Left Side Slope	3.50	H : V
Right Side Slope	3.00	H : V
Bottom Width	3.00	ft

Results		
Discharge	32.92	cfs
Flow Area	11.8	ft²
Wetted Perim	13.20	ft
Top Width	12.75	ft
Critical Depth	1.07	ft
Critical Slope	0.021007	ft/ft
Velocity	2.79	ft/s
Velocity Head	0.12	ft
Specific Energ	1.62	ft
Froude Numb	0.51	
Flow Type	Subcritical	

Down Drain Pipe
Worksheet for Circular Channel

Project Description	
Worksheet	Down Drain Pip
Flow Element	Circular Channe
Method	Manning's Form
Solve For	Full Flow Capac

Input Data		
Mannings Coeffic	0.012	
Channel Slope	285700	ft/ft
Diameter	24.0	in

Results		
Depth	2.00	ft
Discharge	130.99	cfs
Flow Area	3.1	ft²
Wetted Perime	6.28	ft
Top Width	0.00	ft
Critical Depth	2.00	ft
Percent Full	100.0	%
Critical Slope	281143	ft/ft
Velocity	41.69	ft/s
Velocity Head	27.02	ft
Specific Energ	29.02	ft
Froude Numbe	0.00	
Maximum Disc	140.90	cfs
Discharge Full	130.99	cfs
Slope Full	285700	ft/ft
Flow Type	N/A	

LF Phase 3A-1 Intermediate Ditch
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Ph3A-1
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	001600	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	5.00	ft
Discharge	26.86	cfs

Results		
Depth	1.59	ft
Flow Area	15.5	ft²
Wetted Perim	15.04	ft
Top Width	14.53	ft
Critical Depth	0.81	ft
Critical Slope	0.021774	ft/ft
Velocity	1.73	ft/s
Velocity Head	0.05	ft
Specific Energ	1.63	ft
Froude Numb	0.30	
Flow Type	Subcritical	

LF Phase 3A-2 Intermediate Ditch
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Ph3A-2
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	012600	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	5.00	ft
Discharge	35.08	cfs

Results		
Depth	1.08	ft
Flow Area	8.9	ft²
Wetted Perim	11.83	ft
Top Width	11.48	ft
Critical Depth	0.95	ft
Critical Slope	0.020924	ft/ft
Velocity	3.94	ft/s
Velocity Head	0.24	ft
Specific Energ	1.32	ft
Froude Numb	0.79	
Flow Type	Subcritical	

LF Ph3B Intermediate Ditch
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Ph3B
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	007600	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	5.00	ft
Discharge	65.33	cfs

Results		
Depth	1.68	ft
Flow Area	16.8	ft²
Wetted Perim	15.60	ft
Top Width	15.05	ft
Critical Depth	1.33	ft
Critical Slope	0.019155	ft/ft
Velocity	3.89	ft/s
Velocity Head	0.24	ft
Specific Energ	1.91	ft
Froude Numb	0.65	
Flow Type	Subcritical	

LF Ph3C Intermediate Ditch
Worksheet for Trapezoidal Channel

Project Description	
Worksheet	LF Ph3C
Flow Element	Trapezoidal Cha
Method	Manning's Form
Solve For	Channel Depth

Input Data		
Mannings Coeffic	0.035	
Channel Slope	007500	ft/ft
Left Side Slope	3.00	H : V
Right Side Slope	3.00	H : V
Bottom Width	5.00	ft
Discharge	20.44	cfs

Results		
Depth	0.93	ft
Flow Area	7.3	ft²
Wetted Perim	10.90	ft
Top Width	10.60	ft
Critical Depth	0.69	ft
Critical Slope	0.022690	ft/ft
Velocity	2.81	ft/s
Velocity Head	0.12	ft
Specific Energ	1.06	ft
Froude Numb	0.60	
Flow Type	Subcritical	

Culvert Master Results

Culvert Calculator Report
DROP INLET #1

Solve For: Discharge

Culvert Summary					
Allowable HW Elevation	346.37	ft	Headwater Depth/Height	2.80	
Computed Headwater Elev	346.37	ft	Discharge	53.36	cfs
Inlet Control HW Elev.	344.83	ft	Tailwater Elevation	342.82	ft
Outlet Control HW Elev.	346.37	ft	Control Type	Outlet Control	

Grades					
Upstream Invert	339.37	ft	Downstream Invert	338.97	ft
Length	79.57	ft	Constructed Slope	0.005027	ft/ft

Hydraulic Profile					
Profile	PressureProfile		Depth, Downstream	3.85	ft
Slope Type	N/A		Normal Depth	N/A	ft
Flow Regime	N/A		Critical Depth	2.34	ft
Velocity Downstream	10.87	ft/s	Critical Slope	0.014627	ft/ft

Section					
Section Shape	Circular		Mannings Coefficient	0.013	
Section Material	Concrete		Span	2.50	ft
Section Size	30 inch		Rise	2.50	ft
Number Sections	1				

Outlet Control Properties					
Outlet Control HW Elev.	346.37	ft	Upstream Velocity Head	1.84	ft
Ke	0.20		Entrance Loss	0.37	ft

Inlet Control Properties					
Inlet Control HW Elev.	344.83	ft	Flow Control	Submerged	
Inlet Type	Groove end projecting		Area Full	4.9	ft^2
K	0.00450		HDS 5 Chart	1	
M	2.00000		HDS 5 Scale	3	
C	0.03170		Equation Form	1	
Y	0.69000				

Culvert Calculator Report
DROP INLET #2

Solve For: Discharge

Culvert Summary					
Allowable HW Elevation	347.98	ft	Headwater Depth/Height	2.33	
Computed Headwater Elev	347.98	ft	Discharge	87.13	cfs
Inlet Control HW Elev.	347.86	ft	Tailwater Elevation	342.82	ft
Outlet Control HW Elev.	347.98	ft	Control Type	Outlet Control	

Grades					
Upstream Invert	340.98	ft	Downstream Invert	340.47	ft
Length	102.22	ft	Constructed Slope	0.004989	ft/ft

Hydraulic Profile					
Profile	CompositeM2PressureProfile		Depth, Downstream	2.83	ft
Slope Type	Mild		Normal Depth	N/A	ft
Flow Regime	Subcritical		Critical Depth	2.83	ft
Velocity Downstream	12.62	ft/s	Critical Slope	0.014754	ft/ft

Section					
Section Shape	Circular		Mannings Coefficient	0.013	
Section Material	Concrete		Span	3.00	ft
Section Size	36 inch		Rise	3.00	ft
Number Sections	1				

Outlet Control Properties					
Outlet Control HW Elev.	347.98	ft	Upstream Velocity Head	2.36	ft
Ke	0.20		Entrance Loss	0.47	ft

Inlet Control Properties					
Inlet Control HW Elev.	347.86	ft	Flow Control	N/A	
Inlet Type	Groove end projecting		Area Full	7.1	ft^2
K	0.00450		HDS 5 Chart	1	
M	2.00000		HDS 5 Scale	3	
C	0.03170		Equation Form	1	
Y	0.69000				

Culvert Calculator Report
DROP INLET #3

Solve For: Discharge

Culvert Summary					
Allowable HW Elevation	346.30	ft	Headwater Depth/Height	2.33	
Computed Headwater Elev:	346.30	ft	Discharge	144.87	cfs
Inlet Control HW Elev.	344.69	ft	Tailwater Elevation	342.82	ft
Outlet Control HW Elev.	346.30	ft	Control Type	Outlet Control	

Grades					
Upstream Invert	339.30	ft	Downstream Invert	338.65	ft
Length	129.02	ft	Constructed Slope	0.005038	ft/ft

Hydraulic Profile					
Profile	PressureProfile		Depth, Downstream	4.17	ft
Slope Type	N/A		Normal Depth	N/A	ft
Flow Regime	N/A		Critical Depth	2.69	ft
Velocity Downstream	10.25	ft/s	Critical Slope	0.010419	ft/ft

Section					
Section Shape	Circular		Mannings Coefficient	0.013	
Section Material	Concrete		Span	3.00	ft
Section Size	36 inch		Rise	3.00	ft
Number Sections	2				

Outlet Control Properties					
Outlet Control HW Elev.	346.30	ft	Upstream Velocity Head	1.63	ft
Ke	0.20		Entrance Loss	0.33	ft

Inlet Control Properties					
Inlet Control HW Elev.	344.69	ft	Flow Control	N/A	
Inlet Type	Groove end projecting		Area Full	14.1	ft^2
K	0.00450		HDS 5 Chart	1	
M	2.00000		HDS 5 Scale	3	
C	0.03170		Equation Form	1	
Y	0.69000				

Culvert Calculator Report
DROP INLET #4

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	350.47 ft	Headwater Depth/Height	2.32
Computed Headwater Elev	350.47 ft	Discharge	175.85 cfs
Inlet Control HW Elev.	350.47 ft	Tailwater Elevation	343.49 ft
Outlet Control HW Elev.	350.05 ft	Control Type	Inlet Control

Grades			
Upstream Invert	343.52 ft	Downstream Invert	340.00 ft
Length	211.61 ft	Constructed Slope	0.016634 ft/ft

Hydraulic Profile			
Profile	PressureProfile	Depth, Downstream	2.54 ft
Slope Type	N/A	Normal Depth	2.52 ft
Flow Regime	N/A	Critical Depth	2.83 ft
Velocity Downstream	13.78 ft/s	Critical Slope	0.015029 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	3.00 ft
Section Size	36 inch	Rise	3.00 ft
Number Sections	2		

Outlet Control Properties			
Outlet Control HW Elev.	350.05 ft	Upstream Velocity Head	2.51 ft
Ke	0.20	Entrance Loss	0.48 ft

Inlet Control Properties			
Inlet Control HW Elev.	350.47 ft	Flow Control	N/A
Inlet Type	Groove end projecting	Area Full	14.1 ft^2
K	0.00450	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	3
C	0.03170	Equation Form	1
Y	0.69000		

Sediment Pond Routings

Sediment Pond 1

==========
JOB TITLE
==========

Project Date: 9/28/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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MASTER DESIGN STORM SUMMARY

Network Storm Collection: Harnett County

Return Event	Total Depth in	Rainfall Type	RNF ID
2	3.7500	Synthetic Curve	TypeII 24hr
10	5.6000	Synthetic Curve	TypeII 24hr
25	6.6000	Synthetic Curve	TypeII 24hr
100	8.2000	Synthetic Curve	TypeII 24hr

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
LF AREA 3	AREA	2	1.917		11.9350	34.33		
LF AREA 3	AREA	10	3.328		11.9350	58.51		
LF AREA 3	AREA	25	4.114		11.9350	71.58		
LF AREA 3	AREA	100	5.389		11.9350	92.41		
LF AREA 4&5	AREA	2	3.996		11.9500	68.52		
LF AREA 4&5	AREA	10	6.939		11.9450	116.95		
LF AREA 4&5	AREA	25	8.577		11.9450	143.21		
LF AREA 4&5	AREA	100	11.235		11.9450	185.02		
LF AREA3	AREA	2	1.917		11.9350	34.33		
LF AREA3	AREA	10	3.328		11.9350	58.51		
LF AREA3	AREA	25	4.114		11.9350	71.58		
LF AREA3	AREA	100	5.389		11.9350	92.41		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
LF AREA45	AREA	2	3.996		11.9500	68.52		
LF AREA45	AREA	10	6.939		11.9450	116.95		
LF AREA45	AREA	25	8.577		11.9450	143.21		
LF AREA45	AREA	100	11.235		11.9450	185.02		
*OUT 1	JCT	2	2.961		15.3350	2.94		
*OUT 1	JCT	10	5.736		12.7650	13.46		
*OUT 1	JCT	25	8.178		12.2500	37.24		
*OUT 1	JCT	100	12.237		12.1750	64.97		
*OUT 1 W/ 1FT	JCT	2	1.939		18.2800	1.92		
*OUT 1 W/ 1FT	JCT	10	6.541		12.2500	30.66		
*OUT 1 W/ 1FT	JCT	25	9.164		12.1800	50.03		
*OUT 1 W/ 1FT	JCT	100	13.428		12.1250	95.59		
POND 1 IN	POND	2	6.739		11.9450	115.43		
POND 1 IN	POND	10	11.528		11.9450	194.21		
POND 1 IN	POND	25	14.186		11.9450	236.83		
POND 1 IN	POND	100	18.494		11.9400	304.82		
POND 1 OUT	POND	2	2.961		15.3350	2.94	340.45	4.581
POND 1 OUT	POND	10	5.736		12.7650	13.46	341.86	7.082
POND 1 OUT	POND	25	8.178		12.2500	37.24	342.32	7.946
POND 1 OUT	POND	100	12.237		12.1750	64.97	343.33	9.931
POND 1 W/ 1FTIN	POND	2	6.739		11.9450	115.43		
POND 1 W/ 1FTIN	POND	10	11.528		11.9450	194.21		
POND 1 W/ 1FTIN	POND	25	14.186		11.9450	236.83		
POND 1 W/ 1FTIN	POND	100	18.494		11.9400	304.82		
POND 1 W/ 1FTOUT	POND	2	1.939		18.2800	1.92	341.52	6.455
POND 1 W/ 1FTOUT	POND	10	6.541		12.2500	30.66	342.23	7.773
POND 1 W/ 1FTOUT	POND	25	9.164		12.1800	50.03	342.82	8.914
POND 1 W/ 1FTOUT	POND	100	13.428		12.1250	95.59	343.76	10.799

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
POND AREA	AREA	2	.826		11.9150	13.67		
POND AREA	AREA	10	1.260		11.9150	20.52		
POND AREA	AREA	25	1.495		11.9150	24.22		
POND AREA	AREA	100	1.871		11.9150	30.13		
PRE-DEV	AREA	2	1.041		12.0200	15.25		
PRE-DEV	AREA	10	2.695		12.0000	44.37		
PRE-DEV	AREA	25	3.762		11.9850	63.00		
PRE-DEV	AREA	100	5.638		11.9650	95.73		
*PRE-OUT	JCT	2	1.041		12.0200	15.25		
*PRE-OUT	JCT	10	2.695		12.0000	44.37		
*PRE-OUT	JCT	25	3.762		11.9850	63.00		
*PRE-OUT	JCT	100	5.638		11.9650	95.73		
SP-1DA	AREA	2	.826		11.9150	13.67		
SP-1DA	AREA	10	1.260		11.9150	20.52		
SP-1DA	AREA	25	1.495		11.9150	24.22		
SP-1DA	AREA	100	1.871		11.9150	30.13		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 2

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 2 yr
Total Rainfall Depth= 3.7500 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	1.917		11.9350	34.33	LF AREA 3	
		DL	1.917		11.9350	34.33		
		DN	6.739		11.9450	115.43	POND 1 W/ 1FTIN	
ADDLINK 20	ADD	UN	3.996		11.9500	68.52	LF AREA 4&5	
		DL	3.996		11.9500	68.52		
		DN	6.739		11.9450	115.43	POND 1 W/ 1FTIN	
ADDLINK 30	ADD	UN	.826		11.9150	13.67	POND AREA	
		DL	.826		11.9150	13.67		
		DN	6.739		11.9450	115.43	POND 1 W/ 1FTIN	
ADDLINK 40	ADD	UN	1.041		12.0200	15.25	PRE-DEV	
		DL	1.041		12.0200	15.25		
		DN	1.041		12.0200	15.25	PRE-OUT	
ADDLINK 50	ADD	UN	1.917		11.9350	34.33	LF AREA3	
		DL	1.917		11.9350	34.33		
		DN	6.739		11.9450	115.43	POND 1	IN
ADDLINK 60	ADD	UN	.826		11.9150	13.67	SP-1DA	
		DL	.826		11.9150	13.67		
		DN	6.739		11.9450	115.43	POND 1	IN
ADDLINK 70	ADD	UN	3.996		11.9500	68.52	LF AREA45	
		DL	3.996		11.9500	68.52		
		DN	6.739		11.9450	115.43	POND 1	IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	6.739		11.9450	115.43	POND 1 W/ 1FTIN	
ROUTE 10			1.939		18.2800	1.92	POND 1 W/ 1FTOUT	
		DL	1.939		18.2800	1.92		
		DN	1.939		18.2800	1.92	OUT 1 W/ 1FT	
ROUTE 20	PONDrt	UN	6.739		11.9450	115.43	POND 1	IN
ROUTE 20			2.961		15.3350	2.94	POND 1	OUT
		DL	2.961		15.3350	2.94		
		DN	2.961		15.3350	2.94	OUT 1	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 10

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 10 yr
Total Rainfall Depth= 5.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	3.328		11.9350	58.51	LF AREA 3	
		DL	3.328		11.9350	58.51		
		DN	11.528		11.9450	194.21	POND 1 W/ 1FTIN	
ADDLINK 20	ADD	UN	6.939		11.9450	116.95	LF AREA 4&5	
		DL	6.939		11.9450	116.95		
		DN	11.528		11.9450	194.21	POND 1 W/ 1FTIN	
ADDLINK 30	ADD	UN	1.260		11.9150	20.52	POND AREA	
		DL	1.260		11.9150	20.52		
		DN	11.528		11.9450	194.21	POND 1 W/ 1FTIN	
ADDLINK 40	ADD	UN	2.695		12.0000	44.37	PRE-DEV	
		DL	2.695		12.0000	44.37		
		DN	2.695		12.0000	44.37	PRE-OUT	
ADDLINK 50	ADD	UN	3.328		11.9350	58.51	LF AREA3	
		DL	3.328		11.9350	58.51		
		DN	11.528		11.9450	194.21	POND 1	IN
ADDLINK 60	ADD	UN	1.260		11.9150	20.52	SP-1DA	
		DL	1.260		11.9150	20.52		
		DN	11.528		11.9450	194.21	POND 1	IN
ADDLINK 70	ADD	UN	6.939		11.9450	116.95	LF AREA45	
		DL	6.939		11.9450	116.95		
		DN	11.528		11.9450	194.21	POND 1	IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	11.528		11.9450	194.21	POND 1 W/ 1FTIN	
ROUTE 10			6.541		12.2500	30.66	POND 1 W/ 1FTOUT	
		DL	6.541		12.2500	30.66		
		DN	6.541		12.2500	30.66	OUT 1 W/ 1FT	
ROUTE 20	PONDrt	UN	11.528		11.9450	194.21	POND 1	IN
ROUTE 20			5.736		12.7650	13.46	POND 1	OUT
		DL	5.736		12.7650	13.46		
		DN	5.736		12.7650	13.46	OUT 1	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 25

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 25 yr
Total Rainfall Depth= 6.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	4.114		11.9350	71.58	LF AREA 3
		DL	4.114		11.9350	71.58	
		DN	14.186		11.9450	236.83	POND 1 W/ 1FTIN
ADDLINK 20	ADD	UN	8.577		11.9450	143.21	LF AREA 4&5
		DL	8.577		11.9450	143.21	
		DN	14.186		11.9450	236.83	POND 1 W/ 1FTIN
ADDLINK 30	ADD	UN	1.495		11.9150	24.22	POND AREA
		DL	1.495		11.9150	24.22	
		DN	14.186		11.9450	236.83	POND 1 W/ 1FTIN
ADDLINK 40	ADD	UN	3.762		11.9850	63.00	PRE-DEV
		DL	3.762		11.9850	63.00	
		DN	3.762		11.9850	63.00	PRE-OUT
ADDLINK 50	ADD	UN	4.114		11.9350	71.58	LF AREA3
		DL	4.114		11.9350	71.58	
		DN	14.186		11.9450	236.83	POND 1 IN
ADDLINK 60	ADD	UN	1.495		11.9150	24.22	SP-1DA
		DL	1.495		11.9150	24.22	
		DN	14.186		11.9450	236.83	POND 1 IN
ADDLINK 70	ADD	UN	8.577		11.9450	143.21	LF AREA45
		DL	8.577		11.9450	143.21	
		DN	14.186		11.9450	236.83	POND 1 IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	14.186		11.9450	236.83	POND 1 W/ 1FTIN	
ROUTE 10			9.164		12.1800	50.03	POND 1 W/ 1FTOUT	
		DL	9.164		12.1800	50.03		
		DN	9.164		12.1800	50.03	OUT 1 W/ 1FT	
ROUTE 20	PONDrt	UN	14.186		11.9450	236.83	POND 1	IN
ROUTE 20			8.178		12.2500	37.24	POND 1	OUT
		DL	8.178		12.2500	37.24		
		DN	8.178		12.2500	37.24	OUT 1	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 100 yr
Total Rainfall Depth= 8.2000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	5.389		11.9350	92.41	LF AREA 3	
		DL	5.389		11.9350	92.41		
		DN	18.494		11.9400	304.82	POND 1 W/ 1FTIN	
ADDLINK 20	ADD	UN	11.235		11.9450	185.02	LF AREA 4&5	
		DL	11.235		11.9450	185.02		
		DN	18.494		11.9400	304.82	POND 1 W/ 1FTIN	
ADDLINK 30	ADD	UN	1.871		11.9150	30.13	POND AREA	
		DL	1.871		11.9150	30.13		
		DN	18.494		11.9400	304.82	POND 1 W/ 1FTIN	
ADDLINK 40	ADD	UN	5.638		11.9650	95.73	PRE-DEV	
		DL	5.638		11.9650	95.73		
		DN	5.638		11.9650	95.73	PRE-OUT	
ADDLINK 50	ADD	UN	5.389		11.9350	92.41	LF AREA3	
		DL	5.389		11.9350	92.41		
		DN	18.494		11.9400	304.82	POND 1	IN
ADDLINK 60	ADD	UN	1.871		11.9150	30.13	SP-1DA	
		DL	1.871		11.9150	30.13		
		DN	18.494		11.9400	304.82	POND 1	IN
ADDLINK 70	ADD	UN	11.235		11.9450	185.02	LF AREA45	
		DL	11.235		11.9450	185.02		
		DN	18.494		11.9400	304.82	POND 1	IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	18.494		11.9400	304.82	POND 1 W/ 1FTIN	
ROUTE 10			13.428		12.1250	95.59	POND 1 W/ 1FTOUT	
		DL	13.428		12.1250	95.59		
		DN	13.428		12.1250	95.59	OUT 1 W/ 1FT	
ROUTE 20	PONDrt	UN	18.494		11.9400	304.82	POND 1	IN
ROUTE 20			12.237		12.1750	64.97	POND 1	OUT
		DL	12.237		12.1750	64.97		
		DN	12.237		12.1750	64.97	OUT 1	

File.... K:\02\02-330\02334.01 LF Expansion\H&H\
Title... Project Date: 9/28/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

DESIGN STORMS SUMMARY

Design Storm File,ID = Harnett County

Storm Tag Name = 2

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 2 yr
Total Rainfall Depth= 3.7500 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 10

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 10 yr
Total Rainfall Depth= 5.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 25

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 25 yr
Total Rainfall Depth= 6.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 100 yr
Total Rainfall Depth= 8.2000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: Kirpich (TN)

Hydraulic Length	1888.00	ft
Slope	.094000	ft/ft
Multiplier	1.000	(Tc Adjustment)

Avg.Velocity 4.87 ft/sec

Segment #1 Time: .1076 hrs

==========
Total Tc: .1076 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== Kirpich (TN) ==========

Tc = 0.00013 * (Lf**0.77) * (Sf**-0.385) * Mt

Where: Tc = Time of concentration, hrs
Lf = Flow length, ft
Sf = Slope, ft/ft
Mt = Tc Multiplier (Tc adjustment)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: Kirpich (TN)

Hydraulic Length	2200.00	ft
Slope	.076000	ft/ft
Multiplier	1.000	(Tc Adjustment)

Avg.Velocity 4.65 ft/sec

Segment #1 Time: .1314 hrs

==========
Total Tc: .1314 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== Kirpich (TN) ==========

Tc = 0.00013 * (Lf**0.77) * (Sf**-0.385) * Mt

Where: Tc = Time of concentration, hrs
Lf = Flow length, ft
Sf = Slope, ft/ft
Mt = Tc Multiplier (Tc adjustment)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: Kirpich (TN)

Hydraulic Length	1888.00	ft
Slope	.094000	ft/ft
Multiplier	1.000	(Tc Adjustment)

Avg.Velocity 4.87 ft/sec

Segment #1 Time: .1076 hrs

==========
Total Tc: .1076 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== Kirpich (TN) ==========

Tc = 0.00013 * (Lf**0.77) * (Sf**-0.385) * Mt

Where: Tc = Time of concentration, hrs
Lf = Flow length, ft
Sf = Slope, ft/ft
Mt = Tc Multiplier (Tc adjustment)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

::

TIME OF CONCENTRATION CALCULATOR

::

Segment #1: Tc: Kirpich (TN)

Hydraulic Length	2200.00 ft
Slope	.076000 ft/ft
Multiplier	1.000 (Tc Adjustment)
Avg.Velocity	4.65 ft/sec

Segment #1 Time: .1314 hrs

========================

Total Tc: .1314 hrs

========================

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== Kirpich (TN) ==========

Tc = 0.00013 * (Lf**0.77) * (Sf**-0.385) * Mt

Where: Tc = Time of concentration, hrs
Lf = Flow length, ft
Sf = Slope, ft/ft
Mt = Tc Multiplier (Tc adjustment)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

TIME OF CONCENTRATION CALCULATOR

Segment #1: Tc: Kirpich (TN)

Hydraulic Length	1454.00	ft
Slope	.029300	ft/ft
Multiplier	1.000	(Tc Adjustment)
Avg.Velocity	2.93	ft/sec

Segment #1 Time: .1378 hrs

Total Tc: .1378 hrs

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== Kirpich (TN) ==========

Tc = 0.00013 * (Lf**0.77) * (Sf**-0.385) * Mt

Where: Tc = Time of concentration, hrs
Lf = Flow length, ft
Sf = Slope, ft/ft
Mt = Tc Multiplier (Tc adjustment)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	86	9.910			86.00
COMPOSITE AREA & WEIGHTED CN --->		9.910			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	20.660			86.00

COMPOSITE AREA & WEIGHTED CN ---> 20.660 86.00 (86)
::

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	9.910			86.00
COMPOSITE AREA & WEIGHTED CN --->		9.910			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	20.660			86.00

COMPOSITE AREA & WEIGHTED CN ---> 20.660 86.00 (86)
::

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	98	2.820			98.00
COMPOSITE AREA & WEIGHTED CN --->		2.820			98.00 (98)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	60	19.420			60.00

COMPOSITE AREA & WEIGHTED CN ---> 19.420 60.00 (60)
::

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	98	2.820			98.00
COMPOSITE AREA & WEIGHTED CN --->		2.820			98.00 (98)

SCS UNIT HYDROGRAPH METHOD
(Computational Notes)

DEFINITION OF TERMS: ----------

```
At    = Total area (acres):  At = Ai+Ap
Ai    = Impervious area (acres)
Ap    = Pervious area (acres)
CNi   = Runoff curve number for impervious area
CNp   = Runoff curve number for pervious area
fLoss = f loss constant infiltration (depth/time)
gKs   = Saturated Hydraulic Conductivity (depth/time)
Md    = Volumetric Moisture Deficit
Psi   = Capillary Suction (length)
hK    = Horton Infiltration Decay Rate (time^-1)
fo    = Initial Infiltration Rate (depth/time)
fc    = Ultimate(capacity)Infiltration Rate (depth/time)
Ia    = Initial Abstraction (length)
dt    = Computational increment (duration of unit excess rainfall)
        Default dt is smallest value of 0.1333Tc, rtm, and th
        (Smallest dt is then adjusted to match up with Tp)
UDdt  = User specified override computational main time increment
        (only used if UDdt is => .1333Tc)
D(t)  = Point on distribution curve (fraction of P) for time step t

K     = 2 / (1 + (Tr/Tp)):  default K = 0.75: (for Tr/Tp = 1.67)
Ks    = Hydrograph shape factor
      = Unit Conversions * K:
      = ((1hr/3600sec) * (1ft/12in) * ((5280ft)**2/sq.mi)) * K
        Default Ks = 645.333 * 0.75 = 484

Lag   = Lag time from center of excess runoff (dt) to Tp:  Lag = 0.6Tc
P     = Total precipitation depth, inches
Pa(t) = Accumulated rainfall at time step t
Pi(t) = Incremental rainfall at time step t
qp    = Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi.
      = (Ks * A * Q) / Tp   (where Q = 1in. runoff, A=sq.mi.)
Qu(t) = Unit hydrograph ordinate (cfs) at time step t
Q(t)  = Final hydrograph ordinate (cfs) at time step t
Rai(t)= Accumulated runoff (inches) at time step t for impervious area
Rap(t)= Accumulated runoff (inches) at time step t for pervious area
Rii(t)= Incremental runoff (inches) at time step t for impervious area
Rip(t)= Incremental runoff (inches) at time step t for pervious area
R(t)  = Incremental weighted total runoff (inches)
Rtm   = Time increment for rainfall table
Si    = S for impervious area:  Si = (1000/CNi) - 10
Sp    = S for pervious area:    Sp = (1000/CNp) - 10
t     = Time step (row) number
Tc    = Time of concentration
Tb    = Time (hrs) of entire unit hydrograph:  Tb = Tp + Tr
Tp    = Time (hrs) to peak of a unit hydrograph:  Tp = (dt/2) + Lag
Tr    = Time (hrs) of receding limb of unit hydrograph:  Tr = ratio of Tp
```

SCS UNIT HYDROGRAPH METHOD
(Computational Notes)

```
PRECIPITATION: ------------------------------------------------------------
Column (1):  Time for time step t
Column (2):  D(t)  = Point on distribution curve for time step t
Column (3):  Pi(t) = Pa(t) - Pa(t-1): Col.(4) - Preceding Col.(4)
Column (4):  Pa(t) = D(t) x P:        Col.(2) x P


PERVIOUS AREA RUNOFF (using SCS Runoff CN Method) -------------------------
Column (5):  Rap(t) = Accumulated pervious runoff for time step t
             If (Pa(t) is <= 0.2Sp) then use: Rap(t) = 0.0
             If (Pa(t) is >  0.2Sp) then use:

             Rap(t) = (Col.(4)-0.2Sp)**2 / (Col.(4)+0.8Sp)

Column (6):  Rip(t) = Incremental pervious runoff for time step t
             Rip(t) =         Rap(t)          -         Rap(t-1)
             Rip(t) = Col.(5) for current row - Col.(5) for preceding row.

IMPERVIOUS AREA RUNOFF ----------------------------------------------------
Column (7 & 8)... Did not specify to use impervious areas.


INCREMENTAL WEIGHTED RUNOFF: ----------------------------------------------
Column (9):  R(t)  = (Ap/At) x Rip(t)   +  (Ai/At) x Rii(t)
             R(t)  = (Ap/At) x Col.(6)  +  (Ai/At) x Col.(8)


SCS UNIT HYDROGRAPH METHOD: -----------------------------------------------
Column (10): Q(t)   is computed with the SCS unit hydrograph method
                    using R() and Qu().
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 2
Tc            = .1076 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01435 hrs
Computed Peak Time             =  11.9364 hrs
Computed Peak Flow             =    34.37 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9352 hrs
Peak Flow, Interpolated Output =    34.33 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 3
               CN   =   86
               Area =       9.910 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        1.917 ac-ft

HYG Volume...          1.917 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .10760 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01435 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   104.35 cfs
Unit peak time      Tp =   .07173 hrs
Unit receding limb, Tr =   .28693 hrs
Total unit time,    Tb =   .35867 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 10
Tc            = .1076 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

=============================================
Computational Time Increment   =   .01435 hrs
Computed Peak Time             =  11.9364 hrs
Computed Peak Flow             =    58.55 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9352 hrs
Peak Flow, Interpolated Output =    58.51 cfs
=============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 3
              CN   =   86
              Area =       9.910 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       3.328 ac-ft

HYG Volume...          3.328 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .10760 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01435 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   104.35 cfs
Unit peak time      Tp =   .07173 hrs
Unit receding limb, Tr =   .28693 hrs
Total unit time,    Tb =   .35867 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 25
Tc            = .1076 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01435 hrs
Computed Peak Time             =  11.9364 hrs
Computed Peak Flow             =    71.62 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9352 hrs
Peak Flow, Interpolated Output =    71.58 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 3
               CN   =   86
               Area =       9.910 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        4.114 ac-ft

HYG Volume...           4.114 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .10760 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01435 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   104.35 cfs
Unit peak time      Tp =   .07173 hrs
Unit receding limb, Tr =   .28693 hrs
Total unit time,    Tb =   .35867 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 100
Tc            = .1076 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01435 hrs
Computed Peak Time             =  11.9364 hrs
Computed Peak Flow             =    92.45 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9352 hrs
Peak Flow, Interpolated Output =    92.41 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:LF AREA 3
             CN   =   86
             Area =       9.910 acres
             S    =   1.6279 in
             0.2S =    .3256 in

              Cumulative Runoff
             -------------------
                     6.5254 in
                      5.389 ac-ft

HYG Volume...          5.389 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .10760 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01435 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   104.35 cfs
Unit peak time      Tp =   .07173 hrs
Unit receding limb, Tr =   .28693 hrs
Total unit time,    Tb =   .35867 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4&5 2
Tc            = .1314 hrs
Drainage Area = 20.660 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01752 hrs
Computed Peak Time             =  11.9459 hrs
Computed Peak Flow             =    68.53 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9502 hrs
Peak Flow, Interpolated Output =    68.52 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 4&5
               CN   =   86
               Area =      20.660 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        3.996 ac-ft

HYG Volume...          3.996 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13137 hrs (ID: LF AREA 4&5)
Computational Incr, Tm =   .01752 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   178.19 cfs
Unit peak time      Tp =   .08758 hrs
Unit receding limb, Tr =   .35032 hrs
Total unit time,    Tb =   .43790 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4&5 10
Tc            = .1314 hrs
Drainage Area = 20.660 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01752 hrs
Computed Peak Time             =  11.9459 hrs
Computed Peak Flow             =   117.06 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9452 hrs
Peak Flow, Interpolated Output =   116.95 cfs
============================================

                 DRAINAGE AREA
              -------------------
              ID:LF AREA 4&5
              CN   =   86
              Area =      20.660 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       6.939 ac-ft

HYG Volume...          6.939 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13137 hrs (ID: LF AREA 4&5)
Computational Incr, Tm =   .01752 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   178.19 cfs
Unit peak time      Tp =   .08758 hrs
Unit receding limb, Tr =   .35032 hrs
Total unit time,    Tb =   .43790 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4&5 25
Tc            = .1314 hrs
Drainage Area = 20.660 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01752 hrs
Computed Peak Time             =  11.9459 hrs
Computed Peak Flow             =   143.32 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9452 hrs
Peak Flow, Interpolated Output =   143.21 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 4&5
               CN   =   86
               Area =      20.660 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        8.577 ac-ft

HYG Volume...           8.577 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13137 hrs (ID: LF AREA 4&5)
Computational Incr, Tm =   .01752 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   178.19 cfs
Unit peak time      Tp =   .08758 hrs
Unit receding limb, Tr =   .35032 hrs
Total unit time,    Tb =   .43790 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4&5 100
Tc            = .1314 hrs
Drainage Area = 20.660 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01752 hrs
Computed Peak Time             =  11.9459 hrs
Computed Peak Flow             =   185.15 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9452 hrs
Peak Flow, Interpolated Output =   185.02 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 4&5
              CN   =   86
              Area =      20.660 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                      11.235 ac-ft

HYG Volume...         11.235 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13137 hrs (ID: LF AREA 4&5)
Computational Incr, Tm =   .01752 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   178.19 cfs
Unit peak time      Tp =   .08758 hrs
Unit receding limb, Tr =   .35032 hrs
Total unit time,    Tb =   .43790 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - POND AREA 2
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.820 acres  Runoff CN= 98

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    13.69 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9152 hrs
Peak Flow, Interpolated Output =    13.67 cfs
=============================================

                 DRAINAGE AREA
               -------------------
               ID:POND AREA
               CN   =   98
               Area =       2.820 acres
               S    =    .2041 in
               0.2S =    .0408 in

                Cumulative Runoff
               -------------------
                       3.5157 in
                         .826 ac-ft

HYG Volume...           .826 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: POND AREA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    38.36 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - POND AREA 10
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.820 acres  Runoff CN= 98

==========================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    20.55 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9152 hrs
Peak Flow, Interpolated Output =    20.52 cfs
==========================================

                DRAINAGE AREA
             -------------------
             ID:POND AREA
             CN   =   98
             Area =       2.820 acres
             S    =    .2041 in
             0.2S =    .0408 in

              Cumulative Runoff
             -------------------
                     5.3623 in
                      1.260 ac-ft

HYG Volume...          1.260 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: POND AREA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    38.36 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - POND AREA 25
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.820 acres  Runoff CN= 98

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    24.25 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9152 hrs
Peak Flow, Interpolated Output =    24.22 cfs
=============================================

                 DRAINAGE AREA
               -------------------
               ID:POND AREA
               CN   =   98
               Area =       2.820 acres
               S    =    .2041 in
               0.2S =    .0408 in

                Cumulative Runoff
               -------------------
                       6.3613 in
                        1.495 ac-ft

HYG Volume...           1.495 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: POND AREA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    38.36 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - POND AREA 100
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.820 acres  Runoff CN= 98

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    30.17 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9152 hrs
Peak Flow, Interpolated Output =    30.13 cfs
=============================================

                DRAINAGE AREA
             --------------------
             ID:POND AREA
             CN   =   98
             Area =       2.820 acres
             S    =    .2041 in
             0.2S =    .0408 in

              Cumulative Runoff
             --------------------
                     7.9601 in
                      1.871 ac-ft

HYG Volume...          1.871 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: POND AREA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    38.36 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE-DEV 2
Tc            = .1378 hrs
Drainage Area = 19.420 acres  Runoff CN= 60

=============================================
Computational Time Increment   =   .01838 hrs
Computed Peak Time             =  12.0196 hrs
Computed Peak Flow             =    15.26 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  12.0202 hrs
Peak Flow, Interpolated Output =    15.25 cfs
=============================================

                 DRAINAGE AREA
               -------------------
               ID:PRE-DEV
               CN   =   60
               Area =      19.420 acres
               S    =   6.6667 in
               0.2S =   1.3333 in

                Cumulative Runoff
               -------------------
                        .6430 in
                        1.041 ac-ft

HYG Volume...           1.041 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13784 hrs (ID: PRE-DEV)
Computational Incr, Tm =   .01838 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   159.63 cfs
Unit peak time      Tp =   .09189 hrs
Unit receding limb, Tr =   .36757 hrs
Total unit time,    Tb =   .45947 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE-DEV 10
Tc            = .1378 hrs
Drainage Area = 19.420 acres  Runoff CN= 60

============================================
Computational Time Increment   =   .01838 hrs
Computed Peak Time             =  12.0012 hrs
Computed Peak Flow             =    44.38 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  12.0002 hrs
Peak Flow, Interpolated Output =    44.37 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:PRE-DEV
             CN   =   60
             Area =      19.420 acres
             S    =   6.6667 in
             0.2S =   1.3333 in

              Cumulative Runoff
             -------------------
                     1.6650 in
                      2.695 ac-ft

HYG Volume...          2.695 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13784 hrs (ID: PRE-DEV)
Computational Incr, Tm =   .01838 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   159.63 cfs
Unit peak time      Tp =   .09189 hrs
Unit receding limb, Tr =   .36757 hrs
Total unit time,    Tb =   .45947 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE-DEV 25
Tc            = .1378 hrs
Drainage Area = 19.420 acres  Runoff CN= 60

==========================================
Computational Time Increment   =   .01838 hrs
Computed Peak Time             =  11.9829 hrs
Computed Peak Flow             =    63.02 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9852 hrs
Peak Flow, Interpolated Output =    63.00 cfs
==========================================

                 DRAINAGE AREA
               -------------------
               ID:PRE-DEV
               CN   =   60
               Area =      19.420 acres
               S    =   6.6667 in
               0.2S =   1.3333 in

                Cumulative Runoff
               -------------------
                       2.3244 in
                        3.762 ac-ft

HYG Volume...           3.762 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13784 hrs (ID: PRE-DEV)
Computational Incr, Tm =   .01838 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   159.63 cfs
Unit peak time      Tp =   .09189 hrs
Unit receding limb, Tr =   .36757 hrs
Total unit time,    Tb =   .45947 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE-DEV 100
Tc            = .1378 hrs
Drainage Area = 19.420 acres  Runoff CN= 60

============================================
Computational Time Increment   =   .01838 hrs
Computed Peak Time             =  11.9645 hrs
Computed Peak Flow             =    95.73 cfs

Time Increment for HYG File    =    .0050 hrs
Peak Time, Interpolated Output =  11.9652 hrs
Peak Flow, Interpolated Output =    95.73 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:PRE-DEV
              CN   =   60
              Area =      19.420 acres
              S    =   6.6667 in
              0.2S =   1.3333 in

               Cumulative Runoff
              -------------------
                      3.4841 in
                       5.638 ac-ft

HYG Volume...          5.638 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .13784 hrs (ID: PRE-DEV)
Computational Incr, Tm =   .01838 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   159.63 cfs
Unit peak time      Tp =   .09189 hrs
Unit receding limb, Tr =   .36757 hrs
Total unit time,    Tb =   .45947 hrs
```

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
337.50	-----	1.4000	.0000	.000	.000
338.00	-----	1.4500	4.2748	.712	.712
339.00	-----	1.5500	4.4992	1.500	2.212
340.00	-----	1.6600	4.8141	1.605	3.817
341.00	-----	1.7600	5.1293	1.710	5.527
342.00	-----	1.8700	5.4442	1.815	7.341
343.00	-----	1.9800	5.7742	1.925	9.266
344.00	-----	2.0900	6.1043	2.035	11.301
345.00	-----	2.2000	6.4343	2.145	13.446

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Volume = (1/3) * (EL2-EL1) * (Area1 + Area2 + sq.rt.(Area1*Area2))

where: EL1, EL2 = Lower and upper elevations of the increment
Area1,Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
337.50	-----	1.4000	.0000	.000	.000
338.00	-----	1.4500	4.2748	.712	.712
339.00	-----	1.5500	4.4992	1.500	2.212
340.00	-----	1.6600	4.8141	1.605	3.817
341.00	-----	1.7600	5.1293	1.710	5.527
342.00	-----	1.8700	5.4442	1.815	7.341
343.00	-----	1.9800	5.7742	1.925	9.266
344.00	-----	2.0900	6.1043	2.035	11.301
345.00	-----	2.2000	6.4343	2.145	13.446

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Volume = (1/3) * (EL2-EL1) * (Area1 + Area2 + sq.rt.(Area1*Area2))

where: EL1, EL2 = Lower and upper elevations of the increment
Area1,Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

REQUESTED POND WS ELEVATIONS:

Min. Elev.= 337.50 ft
Increment = .01 ft
Max. Elev.= 345.00 ft

****
OUTLET CONNECTIVITY
****

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.		Outfall	E1, ft	E2, ft
Weir-Rectangular	WR	--->	TW	343.000	345.000
Stand Pipe	SP	--->	CV	341.500	345.000
Orifice-Circular	O2	--->	CV	338.500	345.000
Orifice-Circular	O1	--->	CV	337.540	345.000
Culvert-Circular	CV	--->	TW	337.540	345.000
TW SETUP, DS Channel					

OUTLET STRUCTURE INPUT DATA

```
Structure ID       = WR
Structure Type     = Weir-Rectangular
-----------------------------------
# of Openings      =         1
Crest Elev.        =    343.00 ft
Weir Length        =     20.00 ft
Weir Coeff.        =  3.000000

Weir TW effects      (Use adjustment equation)
```

```
Structure ID       = SP
Structure Type     = Stand Pipe
-----------------------------------
# of Openings      =         1
Invert Elev.       =    341.50 ft
Diameter           =    5.0000 ft
Orifice Area       =   19.6350 sq.ft
Orifice Coeff.     =      .600
Weir Length        =     15.71 ft
Weir Coeff.        =     3.000
K, Submerged       =      .000
K, Reverse         =     1.000
Kb,Barrel          =   .000000  (per ft of full flow)
Barrel Length      =       .00 ft
Mannings n         =     .0000
```

```
Structure ID       = O2
Structure Type     = Orifice-Circular
-----------------------------------
# of Openings      =        10
Invert Elev.       =    338.50 ft
Diameter           =     .1667 ft
Orifice Coeff.     =      .600
```

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

OUTLET STRUCTURE INPUT DATA

```
Structure ID      = O1
Structure Type    = Orifice-Circular
-----------------------------------
# of Openings     =        10
Invert Elev.      =    337.54 ft
Diameter          =     .1667 ft
Orifice Coeff.    =      .600


Structure ID      = CV
Structure Type    = Culvert-Circular
-----------------------------------
No. Barrels       =         1
Barrel Diameter   =    3.0000 ft
Upstream Invert   =    337.54 ft
Dnstream Invert   =    337.00 ft
Horiz. Length     =    106.31 ft
Barrel Length     =    106.31 ft
Barrel Slope      =    .00508 ft/ft

OUTLET CONTROL DATA...
Mannings n        =     .0220
Ke                =     .9000  (forward entrance loss)
Kb                =   .020700  (per ft of full flow)
Kr                =     .9000  (reverse entrance loss)
HW Convergence    =      .001  +/- ft

INLET CONTROL DATA...
Equation form     =         1
Inlet Control K   =     .0340
Inlet Control M   =    1.5000
Inlet Control c   =    .05530
Inlet Control Y   =     .5400
T1 ratio (HW/D)   =     1.260
T2 ratio (HW/D)   =     1.422
Slope Factor      =     -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use   submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...
At T1 Elev =    341.32 ft  --->  Flow =     42.85 cfs
At T2 Elev =    341.81 ft  --->  Flow =     48.97 cfs
```

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OUTLET STRUCTURE INPUT DATA

Structure ID = TW
Structure Type = TW SETUP, DS Channel

FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...
Maximum Iterations= 30
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .10 cfs
Max. Q tolerance = .10 cfs

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
337.50	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
337.51	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
337.52	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
337.53	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
337.54	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
337.55	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.56	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.57	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.58	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.59	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.60	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.61	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.62	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.63	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.64	.12	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.65	.14	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.66	.14	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.67	.16	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.68	.16	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.69	.16	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.70	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.71	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.72	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.73	.20	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.74	.22	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.75	.22	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.76	.22	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.77	.22	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.78	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.79	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.80	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.81	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.82	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.83	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.84	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.85	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.86	.51	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.87	.52	Free Outfall		O1,CV (no Q: WR,SP,O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
337.88	.52	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.89	.54	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.90	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.91	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.92	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.93	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.94	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.95	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.96	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.97	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.98	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
337.99	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.00	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.01	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.02	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.03	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.04	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.05	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.06	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.07	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.08	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.09	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.10	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.11	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.12	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.13	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.14	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.15	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.16	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.17	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.18	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.19	.56	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.20	.56	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.21	.57	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.22	.57	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.23	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.24	.59	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.25	.60	Free Outfall		O1,CV (no Q: WR,SP,O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: Converge TW Elev ft	Converge Error +/-ft	Contributing Structures
338.26	.60	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.27	.61	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.28	.62	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.29	.63	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.30	.63	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.31	.64	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.32	.65	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.33	.65	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.34	.66	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.35	.67	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.36	.67	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.37	.68	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.38	.69	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.39	.69	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.40	.70	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.41	.71	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.42	.71	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.43	.72	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.44	.73	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.45	.73	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.46	.74	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.47	.75	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.48	.75	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.49	.76	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.50	.76	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.51	.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.52	.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.53	.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.54	.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.55	.82	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.56	.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.57	.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.58	.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.59	.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.60	.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.61	.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.62	.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.63	1.01	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
Elev. ft	Q cfs	TW Elev ft	Converge Error +/-ft	Contributing Structures
338.64	1.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.65	1.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.66	1.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.67	1.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.68	1.15	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.69	1.17	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.70	1.19	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.71	1.21	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.72	1.23	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.73	1.25	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.74	1.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.75	1.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.76	1.30	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.77	1.32	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.78	1.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.79	1.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.80	1.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.81	1.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.82	1.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.83	1.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.84	1.42	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.85	1.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.86	1.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.87	1.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.88	1.48	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.89	1.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.90	1.51	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.91	1.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.92	1.54	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.93	1.55	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.94	1.56	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.95	1.57	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.96	1.59	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.97	1.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.98	1.61	Free Outfall		O2,O1,CV (no Q: WR,SP)
338.99	1.63	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.00	1.64	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.01	1.65	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: Converge TW Elev ft	Converge Error +/-ft	Contributing Structures
339.02	1.66	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.03	1.68	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.04	1.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.05	1.70	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.06	1.71	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.07	1.72	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.08	1.73	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.09	1.75	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.10	1.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.11	1.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.12	1.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.13	1.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.14	1.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.15	1.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.16	1.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.17	1.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.18	1.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.19	1.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.20	1.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.21	1.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.22	1.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.23	1.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.24	1.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.25	1.92	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.26	1.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.27	1.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.28	1.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.29	1.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.30	1.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.31	1.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.32	2.00	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.33	2.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.34	2.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.35	2.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.36	2.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.37	2.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.38	2.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.39	2.07	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.40	2.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.41	2.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.42	2.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.43	2.11	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.44	2.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.45	2.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.46	2.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.47	2.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.48	2.15	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.49	2.16	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.50	2.17	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.51	2.18	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.52	2.19	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.53	2.20	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.54	2.21	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.55	2.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.56	2.23	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.57	2.24	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.58	2.25	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.59	2.26	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.60	2.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.61	2.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.62	2.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.63	2.29	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.64	2.30	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.65	2.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.66	2.32	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.67	2.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.68	2.34	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.69	2.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.70	2.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.71	2.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.72	2.37	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.73	2.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.74	2.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.75	2.40	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.76	2.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.77	2.42	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.78	2.42	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.79	2.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.80	2.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.81	2.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.82	2.46	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.83	2.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.84	2.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.85	2.48	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.86	2.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.87	2.50	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.88	2.51	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.89	2.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.90	2.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.91	2.53	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.92	2.54	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.93	2.55	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.94	2.56	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.95	2.56	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.96	2.57	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.97	2.58	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.98	2.59	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.99	2.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.00	2.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.01	2.61	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.02	2.62	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.03	2.63	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.04	2.64	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.05	2.64	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.06	2.65	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.07	2.66	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.08	2.67	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.09	2.68	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.10	2.68	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.11	2.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.12	2.70	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.13	2.71	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.14	2.71	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.15	2.72	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.16	2.73	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.17	2.74	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.18	2.74	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.19	2.75	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.20	2.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.21	2.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.22	2.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.23	2.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.24	2.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.25	2.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.26	2.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.27	2.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.28	2.82	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.29	2.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.30	2.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.31	2.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.32	2.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.33	2.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.34	2.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.35	2.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.36	2.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.37	2.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.38	2.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.39	2.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.40	2.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.41	2.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.42	2.92	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.43	2.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.44	2.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.45	2.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.46	2.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.47	2.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.48	2.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.49	2.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.50	2.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.51	2.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.52	2.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.53	3.00	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: Converge TW Elev ft	Converge Error +/-ft	Contributing Structures
340.54	3.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.55	3.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.56	3.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.57	3.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.58	3.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.59	3.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.60	3.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.61	3.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.62	3.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.63	3.07	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.64	3.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.65	3.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.66	3.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.67	3.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.68	3.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.69	3.11	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.70	3.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.71	3.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.72	3.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.73	3.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.74	3.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.75	3.15	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.76	3.16	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.77	3.16	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.78	3.17	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.79	3.18	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.80	3.18	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.81	3.19	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.82	3.20	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.83	3.20	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.84	3.21	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.85	3.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.86	3.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.87	3.23	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.88	3.24	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.89	3.24	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.90	3.25	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.91	3.25	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.92	3.26	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.93	3.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.94	3.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.95	3.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.96	3.29	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.97	3.29	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.98	3.30	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.99	3.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.00	3.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.01	3.32	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.02	3.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.03	3.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.04	3.34	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.05	3.34	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.06	3.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.07	3.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.08	3.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.09	3.37	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.10	3.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.11	3.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.12	3.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.13	3.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.14	3.40	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.15	3.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.16	3.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.17	3.42	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.18	3.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.19	3.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.20	3.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.21	3.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.22	3.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.23	3.46	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.24	3.46	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.25	3.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.26	3.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.27	3.48	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.28	3.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.29	3.49	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.30	3.50	Free Outfall		02,01,CV (no Q: WR,SP)
341.31	3.50	Free Outfall		02,01,CV (no Q: WR,SP)
341.32	3.51	Free Outfall		02,01,CV (no Q: WR,SP)
341.33	3.52	Free Outfall		02,01,CV (no Q: WR,SP)
341.34	3.52	Free Outfall		02,01,CV (no Q: WR,SP)
341.35	3.53	Free Outfall		02,01,CV (no Q: WR,SP)
341.36	3.53	Free Outfall		02,01,CV (no Q: WR,SP)
341.37	3.54	Free Outfall		02,01,CV (no Q: WR,SP)
341.38	3.55	Free Outfall		02,01,CV (no Q: WR,SP)
341.39	3.55	Free Outfall		02,01,CV (no Q: WR,SP)
341.40	3.56	Free Outfall		02,01,CV (no Q: WR,SP)
341.41	3.56	Free Outfall		02,01,CV (no Q: WR,SP)
341.42	3.57	Free Outfall		02,01,CV (no Q: WR,SP)
341.43	3.58	Free Outfall		02,01,CV (no Q: WR,SP)
341.44	3.58	Free Outfall		02,01,CV (no Q: WR,SP)
341.45	3.59	Free Outfall		02,01,CV (no Q: WR,SP)
341.46	3.59	Free Outfall		02,01,CV (no Q: WR,SP)
341.47	3.60	Free Outfall		02,01,CV (no Q: WR,SP)
341.48	3.61	Free Outfall		02,01,CV (no Q: WR,SP)
341.49	3.61	Free Outfall		02,01,CV (no Q: WR,SP)
341.50	3.62	Free Outfall		02,01,CV (no Q: WR,SP)
341.51	3.67	Free Outfall		SP,02,01,CV (no Q: WR)
341.52	3.76	Free Outfall		SP,02,01,CV (no Q: WR)
341.53	3.87	Free Outfall		SP,02,01,CV (no Q: WR)
341.54	4.00	Free Outfall		SP,02,01,CV (no Q: WR)
341.55	4.15	Free Outfall		SP,02,01,CV (no Q: WR)
341.56	4.32	Free Outfall		SP,02,01,CV (no Q: WR)
341.57	4.49	Free Outfall		SP,02,01,CV (no Q: WR)
341.58	4.68	Free Outfall		SP,02,01,CV (no Q: WR)
341.59	4.87	Free Outfall		SP,02,01,CV (no Q: WR)
341.60	5.08	Free Outfall		SP,02,01,CV (no Q: WR)
341.61	5.30	Free Outfall		SP,02,01,CV (no Q: WR)
341.62	5.53	Free Outfall		SP,02,01,CV (no Q: WR)
341.63	5.77	Free Outfall		SP,02,01,CV (no Q: WR)
341.64	6.02	Free Outfall		SP,02,01,CV (no Q: WR)
341.65	6.28	Free Outfall		SP,02,01,CV (no Q: WR)
341.66	6.54	Free Outfall		SP,02,01,CV (no Q: WR)
341.67	6.82	Free Outfall		SP,02,01,CV (no Q: WR)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
Elev. ft	Q cfs	TW Elev ft	Converge Error +/-ft	Contributing Structures
341.68	7.10	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.69	7.40	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.70	7.70	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.71	8.00	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.72	8.32	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.73	8.64	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.74	8.97	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.75	9.31	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.76	9.65	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.77	10.00	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.78	10.36	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.79	10.72	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.80	11.09	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.81	11.47	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.82	11.85	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.83	12.24	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.84	12.63	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.85	13.03	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.86	13.44	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.87	13.85	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.88	14.27	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.89	14.69	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.90	15.12	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.91	15.55	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.92	15.99	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.93	16.43	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.94	16.88	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.95	17.34	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.96	17.80	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.97	18.26	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.98	18.73	Free Outfall		SP,O2,O1,CV (no Q: WR)
341.99	19.21	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.00	19.69	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.01	20.17	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.02	20.66	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.03	21.15	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.04	21.65	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.05	22.15	Free Outfall		SP,O2,O1,CV (no Q: WR)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.06	22.66	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.07	23.17	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.08	23.69	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.09	24.21	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.10	24.73	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.11	25.26	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.12	25.80	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.13	26.33	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.14	26.87	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.15	27.42	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.16	27.97	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.17	28.52	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.18	29.08	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.19	29.64	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.20	30.20	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.21	30.77	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.22	31.34	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.23	31.92	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.24	32.49	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.25	33.07	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.26	33.66	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.27	34.24	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.28	34.83	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.29	35.43	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.30	36.02	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.31	36.62	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.32	37.22	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.33	37.82	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.34	38.42	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.35	39.02	Free Outfall		SP,O2,O1,CV (no Q: WR)
342.36	40.58	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.37	41.20	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.38	41.82	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.39	42.44	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.40	43.06	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.41	43.68	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.42	44.30	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.43	44.92	Free Outfall		SP,CV (no Q: WR,O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.44	45.53	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.45	45.78	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.46	45.97	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.47	46.56	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.48	47.63	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.49	47.71	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.50	47.78	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.51	47.84	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.52	47.91	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.53	47.99	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.54	48.06	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.55	48.13	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.56	48.19	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.57	48.26	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.58	48.34	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.59	48.41	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.60	48.48	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.61	48.54	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.62	48.61	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.63	48.70	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.64	48.78	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.65	48.83	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.66	48.91	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.67	48.98	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.68	49.06	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.69	49.12	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.70	49.19	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.71	49.26	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.72	49.33	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.73	49.40	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.74	49.47	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.75	49.53	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.76	49.60	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.77	49.68	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.78	49.74	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.79	49.81	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.80	49.88	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.81	49.95	Free Outfall		SP,CV (no Q: WR,O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.82	50.02	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.83	50.09	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.84	50.15	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.85	50.23	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.86	50.29	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.87	50.36	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.88	50.43	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.89	50.50	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.90	50.57	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.91	50.63	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.92	50.71	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.93	50.76	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.94	50.84	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.95	50.90	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.96	50.98	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.97	51.03	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.98	51.11	Free Outfall		SP,CV (no Q: WR,O2,O1)
342.99	51.17	Free Outfall		SP,CV (no Q: WR,O2,O1)
343.00	51.24	Free Outfall		SP,CV (no Q: WR,O2,O1)
343.01	51.37	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.02	51.56	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.03	51.77	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.04	52.01	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.05	52.26	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.06	52.55	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.07	52.84	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.08	53.16	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.09	53.48	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.10	53.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.11	54.19	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.12	54.55	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.13	54.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.14	55.34	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.15	55.75	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.16	56.17	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.17	56.60	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.18	57.04	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.19	57.50	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.20	57.96	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.21	58.43	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.22	58.92	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.23	59.41	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.24	59.90	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.25	60.41	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.26	60.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.27	61.46	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.28	62.00	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.29	62.54	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.30	63.09	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.31	63.66	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.32	64.22	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.33	64.80	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.34	65.38	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.35	65.97	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.36	66.57	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.37	67.18	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.38	67.79	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.39	68.42	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.40	69.04	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.41	69.67	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.42	70.31	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.43	70.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.44	71.61	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.45	72.28	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.46	72.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.47	73.61	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.48	74.30	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.49	74.98	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.50	75.68	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.51	76.37	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.52	77.08	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.53	77.78	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.54	78.50	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.55	79.22	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.56	79.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.57	80.69	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.58	81.43	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.59	82.18	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.60	82.93	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.61	83.68	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.62	84.43	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.63	85.24	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.64	86.01	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.65	86.79	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.66	87.57	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.67	88.37	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.68	89.16	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.69	89.96	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.70	90.76	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.71	91.58	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.72	92.40	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.73	93.21	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.74	94.04	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.75	94.87	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.76	95.71	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.77	96.54	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.78	97.40	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.79	98.25	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.80	99.10	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.81	99.96	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.82	100.82	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.83	101.69	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.84	102.57	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.85	103.45	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.86	104.33	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.87	105.22	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.88	106.11	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.89	107.01	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.90	107.91	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.91	108.81	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.92	109.72	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.93	110.64	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.94	111.55	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.95	112.48	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.96	113.40	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.97	114.34	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.98	115.28	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.99	116.22	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.00	117.17	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.01	118.11	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.02	119.07	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.03	120.03	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.04	120.99	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.05	121.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.06	122.92	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.07	123.90	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.08	124.88	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.09	125.86	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.10	126.85	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.11	127.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.12	128.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.13	129.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.14	130.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.15	131.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.16	132.84	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.17	133.86	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.18	134.88	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.19	135.90	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.20	136.93	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.21	137.96	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.22	138.99	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.23	140.03	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.24	141.06	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.25	142.10	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.26	143.15	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.27	144.21	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.28	145.27	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.29	146.32	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.30	147.38	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.31	148.46	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.32	149.52	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.33	150.60	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.34	151.68	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.35	152.76	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.36	153.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.37	154.93	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.38	156.02	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.39	157.12	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.40	158.21	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.41	159.32	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.42	160.42	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.43	161.53	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.44	162.65	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.45	163.76	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.46	164.88	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.47	166.00	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.48	167.13	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.49	168.25	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.50	169.39	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.51	170.53	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.52	171.67	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.53	172.80	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.54	173.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.55	175.10	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.56	176.25	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.57	177.42	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.58	178.57	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.59	179.73	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.60	180.90	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.61	182.06	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.62	183.24	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.63	184.41	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.64	185.60	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.65	186.77	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.66	187.96	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.67	189.14	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.68	190.34	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.69	191.53	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.70	192.73	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.71	193.92	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.72	195.13	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.73	196.33	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.74	197.54	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.75	198.75	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.76	199.97	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.77	201.30	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.78	202.52	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.79	203.74	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.80	204.97	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.81	206.20	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.82	207.44	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.83	208.68	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.84	209.92	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.85	211.16	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.86	212.41	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.87	213.66	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.88	214.92	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.89	216.17	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.90	217.43	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.91	218.68	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.92	219.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.93	221.21	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.94	222.48	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.95	223.75	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.96	225.03	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.97	226.31	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.98	227.60	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.99	228.87	Free Outfall		WR,SP,CV (no Q: O2,O1)
345.00	230.17	Free Outfall		WR,SP,CV (no Q: O2,O1)

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REQUESTED POND WS ELEVATIONS:

Min. Elev.= 337.50 ft
Increment = .01 ft
Max. Elev.= 345.00 ft

****
OUTLET CONNECTIVITY
****

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.		Outfall	E1, ft	E2, ft
Weir-Rectangular	WR	--->	TW	343.000	345.000
Stand Pipe	SP	--->	CV	341.500	345.000
Orifice-Circular	O2	--->	CV	338.500	345.000
Culvert-Circular	CV	--->	TW	337.540	345.000

TW SETUP, DS Channel

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OUTLET STRUCTURE INPUT DATA

```
Structure ID      = WR
Structure Type    = Weir-Rectangular
-----------------------------------
# of Openings     =         1
Crest Elev.       =    343.00 ft
Weir Length       =     20.00 ft
Weir Coeff.       =  3.000000

Weir TW effects     (Use adjustment equation)
```

```
Structure ID      = SP
Structure Type    = Stand Pipe
-----------------------------------
# of Openings     =         1
Invert Elev.      =    341.50 ft
Diameter          =    5.0000 ft
Orifice Area      =   19.6350 sq.ft
Orifice Coeff.    =      .600
Weir Length       =     15.71 ft
Weir Coeff.       =     3.000
K, Submerged      =      .000
K, Reverse        =     1.000
Kb,Barrel         =   .000000  (per ft of full flow)
Barrel Length     =       .00 ft
Mannings n        =     .0000
```

```
Structure ID      = O2
Structure Type    = Orifice-Circular
-----------------------------------
# of Openings     =        10
Invert Elev.      =    338.50 ft
Diameter          =     .1667 ft
Orifice Coeff.    =      .600
```

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OUTLET STRUCTURE INPUT DATA

```
Structure ID       = CV
Structure Type     = Culvert-Circular
-----------------------------------
No. Barrels        =         1
Barrel Diameter    =    3.0000 ft
Upstream Invert    =    337.54 ft
Dnstream Invert    =    337.00 ft
Horiz. Length      =    106.31 ft
Barrel Length      =    106.31 ft
Barrel Slope       =    .00508 ft/ft

OUTLET CONTROL DATA...
Mannings n         =     .0220
Ke                 =     .9000  (forward entrance loss)
Kb                 =   .020700  (per ft of full flow)
Kr                 =     .9000  (reverse entrance loss)
HW Convergence     =      .001  +/- ft

INLET CONTROL DATA...
Equation form      =         1
Inlet Control K    =     .0340
Inlet Control M    =    1.5000
Inlet Control c    =    .05530
Inlet Control Y    =     .5400
T1 ratio (HW/D)    =     1.260
T2 ratio (HW/D)    =     1.422
Slope Factor       =     -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use   submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...
At T1 Elev =    341.32 ft  --->  Flow =     42.85 cfs
At T2 Elev =    341.81 ft  --->  Flow =     48.97 cfs
```

```
Structure ID       = TW
Structure Type     = TW SETUP, DS Channel
-----------------------------------
FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...
Maximum Iterations=    30
Min. TW tolerance =    .01 ft
Max. TW tolerance =    .01 ft
Min. HW tolerance =    .01 ft
Max. HW tolerance =    .01 ft
Min.  Q tolerance =    .00 cfs
Max.  Q tolerance =    .00 cfs
```

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q, Q cfs	Converge, TW Elev ft	Converge, Error +/-ft	Notes, Contributing Structures
337.50	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.51	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.52	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.53	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.54	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.55	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.56	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.57	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.58	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.59	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.60	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.61	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.62	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.63	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.64	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.65	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.66	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.67	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.68	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.69	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.70	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.71	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.72	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.73	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.74	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.75	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.76	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.77	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.78	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.79	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.80	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.81	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.82	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.83	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.84	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.85	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.86	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.87	.00	Free Outfall		(no Q: WR,SP,O2,CV)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
337.88	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.89	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.90	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.91	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.92	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.93	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.94	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.95	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.96	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.97	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.98	.00	Free Outfall		(no Q: WR,SP,O2,CV)
337.99	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.00	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.01	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.02	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.03	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.04	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.05	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.06	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.07	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.08	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.09	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.10	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.11	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.12	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.13	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.14	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.15	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.16	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.17	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.18	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.19	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.20	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.21	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.22	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.23	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.24	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.25	.00	Free Outfall		(no Q: WR,SP,O2,CV)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
338.26	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.27	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.28	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.29	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.30	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.31	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.32	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.33	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.34	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.35	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.36	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.37	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.38	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.39	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.40	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.41	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.42	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.43	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.44	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.45	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.46	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.47	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.48	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.49	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.50	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.51	.00	Free Outfall		O2,CV (no Q: WR,SP)
338.52	.00	Free Outfall		O2,CV (no Q: WR,SP)
338.53	.01	Free Outfall		O2,CV (no Q: WR,SP)
338.54	.01	Free Outfall		O2,CV (no Q: WR,SP)
338.55	.03	Free Outfall		O2,CV (no Q: WR,SP)
338.56	.05	Free Outfall		O2,CV (no Q: WR,SP)
338.57	.06	Free Outfall		O2,CV (no Q: WR,SP)
338.58	.08	Free Outfall		O2,CV (no Q: WR,SP)
338.59	.10	Free Outfall		O2,CV (no Q: WR,SP)
338.60	.12	Free Outfall		O2,CV (no Q: WR,SP)
338.61	.14	Free Outfall		O2,CV (no Q: WR,SP)
338.62	.17	Free Outfall		O2,CV (no Q: WR,SP)
338.63	.19	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
338.64	.22	Free Outfall		O2,CV (no Q: WR,SP)
338.65	.25	Free Outfall		O2,CV (no Q: WR,SP)
338.66	.27	Free Outfall		O2,CV (no Q: WR,SP)
338.67	.31	Free Outfall		O2,CV (no Q: WR,SP)
338.68	.33	Free Outfall		O2,CV (no Q: WR,SP)
338.69	.34	Free Outfall		O2,CV (no Q: WR,SP)
338.70	.36	Free Outfall		O2,CV (no Q: WR,SP)
338.71	.37	Free Outfall		O2,CV (no Q: WR,SP)
338.72	.39	Free Outfall		O2,CV (no Q: WR,SP)
338.73	.40	Free Outfall		O2,CV (no Q: WR,SP)
338.74	.42	Free Outfall		O2,CV (no Q: WR,SP)
338.75	.43	Free Outfall		O2,CV (no Q: WR,SP)
338.76	.44	Free Outfall		O2,CV (no Q: WR,SP)
338.77	.45	Free Outfall		O2,CV (no Q: WR,SP)
338.78	.47	Free Outfall		O2,CV (no Q: WR,SP)
338.79	.48	Free Outfall		O2,CV (no Q: WR,SP)
338.80	.49	Free Outfall		O2,CV (no Q: WR,SP)
338.81	.50	Free Outfall		O2,CV (no Q: WR,SP)
338.82	.51	Free Outfall		O2,CV (no Q: WR,SP)
338.83	.52	Free Outfall		O2,CV (no Q: WR,SP)
338.84	.53	Free Outfall		O2,CV (no Q: WR,SP)
338.85	.54	Free Outfall		O2,CV (no Q: WR,SP)
338.86	.55	Free Outfall		O2,CV (no Q: WR,SP)
338.87	.56	Free Outfall		O2,CV (no Q: WR,SP)
338.88	.57	Free Outfall		O2,CV (no Q: WR,SP)
338.89	.58	Free Outfall		O2,CV (no Q: WR,SP)
338.90	.59	Free Outfall		O2,CV (no Q: WR,SP)
338.91	.60	Free Outfall		O2,CV (no Q: WR,SP)
338.92	.61	Free Outfall		O2,CV (no Q: WR,SP)
338.93	.62	Free Outfall		O2,CV (no Q: WR,SP)
338.94	.63	Free Outfall		O2,CV (no Q: WR,SP)
338.95	.64	Free Outfall		O2,CV (no Q: WR,SP)
338.96	.64	Free Outfall		O2,CV (no Q: WR,SP)
338.97	.65	Free Outfall		O2,CV (no Q: WR,SP)
338.98	.66	Free Outfall		O2,CV (no Q: WR,SP)
338.99	.67	Free Outfall		O2,CV (no Q: WR,SP)
339.00	.68	Free Outfall		O2,CV (no Q: WR,SP)
339.01	.69	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.02	.69	Free Outfall		O2,CV (no Q: WR,SP)
339.03	.70	Free Outfall		O2,CV (no Q: WR,SP)
339.04	.71	Free Outfall		O2,CV (no Q: WR,SP)
339.05	.72	Free Outfall		O2,CV (no Q: WR,SP)
339.06	.73	Free Outfall		O2,CV (no Q: WR,SP)
339.07	.73	Free Outfall		O2,CV (no Q: WR,SP)
339.08	.74	Free Outfall		O2,CV (no Q: WR,SP)
339.09	.75	Free Outfall		O2,CV (no Q: WR,SP)
339.10	.76	Free Outfall		O2,CV (no Q: WR,SP)
339.11	.76	Free Outfall		O2,CV (no Q: WR,SP)
339.12	.77	Free Outfall		O2,CV (no Q: WR,SP)
339.13	.78	Free Outfall		O2,CV (no Q: WR,SP)
339.14	.78	Free Outfall		O2,CV (no Q: WR,SP)
339.15	.79	Free Outfall		O2,CV (no Q: WR,SP)
339.16	.80	Free Outfall		O2,CV (no Q: WR,SP)
339.17	.80	Free Outfall		O2,CV (no Q: WR,SP)
339.18	.81	Free Outfall		O2,CV (no Q: WR,SP)
339.19	.82	Free Outfall		O2,CV (no Q: WR,SP)
339.20	.82	Free Outfall		O2,CV (no Q: WR,SP)
339.21	.83	Free Outfall		O2,CV (no Q: WR,SP)
339.22	.84	Free Outfall		O2,CV (no Q: WR,SP)
339.23	.84	Free Outfall		O2,CV (no Q: WR,SP)
339.24	.85	Free Outfall		O2,CV (no Q: WR,SP)
339.25	.86	Free Outfall		O2,CV (no Q: WR,SP)
339.26	.86	Free Outfall		O2,CV (no Q: WR,SP)
339.27	.87	Free Outfall		O2,CV (no Q: WR,SP)
339.28	.88	Free Outfall		O2,CV (no Q: WR,SP)
339.29	.88	Free Outfall		O2,CV (no Q: WR,SP)
339.30	.89	Free Outfall		O2,CV (no Q: WR,SP)
339.31	.90	Free Outfall		O2,CV (no Q: WR,SP)
339.32	.90	Free Outfall		O2,CV (no Q: WR,SP)
339.33	.91	Free Outfall		O2,CV (no Q: WR,SP)
339.34	.91	Free Outfall		O2,CV (no Q: WR,SP)
339.35	.92	Free Outfall		O2,CV (no Q: WR,SP)
339.36	.93	Free Outfall		O2,CV (no Q: WR,SP)
339.37	.93	Free Outfall		O2,CV (no Q: WR,SP)
339.38	.94	Free Outfall		O2,CV (no Q: WR,SP)
339.39	.94	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.40	.95	Free Outfall		O2,CV (no Q: WR,SP)
339.41	.96	Free Outfall		O2,CV (no Q: WR,SP)
339.42	.96	Free Outfall		O2,CV (no Q: WR,SP)
339.43	.97	Free Outfall		O2,CV (no Q: WR,SP)
339.44	.97	Free Outfall		O2,CV (no Q: WR,SP)
339.45	.98	Free Outfall		O2,CV (no Q: WR,SP)
339.46	.98	Free Outfall		O2,CV (no Q: WR,SP)
339.47	.99	Free Outfall		O2,CV (no Q: WR,SP)
339.48	.99	Free Outfall		O2,CV (no Q: WR,SP)
339.49	1.00	Free Outfall		O2,CV (no Q: WR,SP)
339.50	1.01	Free Outfall		O2,CV (no Q: WR,SP)
339.51	1.01	Free Outfall		O2,CV (no Q: WR,SP)
339.52	1.02	Free Outfall		O2,CV (no Q: WR,SP)
339.53	1.02	Free Outfall		O2,CV (no Q: WR,SP)
339.54	1.03	Free Outfall		O2,CV (no Q: WR,SP)
339.55	1.03	Free Outfall		O2,CV (no Q: WR,SP)
339.56	1.04	Free Outfall		O2,CV (no Q: WR,SP)
339.57	1.04	Free Outfall		O2,CV (no Q: WR,SP)
339.58	1.05	Free Outfall		O2,CV (no Q: WR,SP)
339.59	1.05	Free Outfall		O2,CV (no Q: WR,SP)
339.60	1.06	Free Outfall		O2,CV (no Q: WR,SP)
339.61	1.06	Free Outfall		O2,CV (no Q: WR,SP)
339.62	1.07	Free Outfall		O2,CV (no Q: WR,SP)
339.63	1.07	Free Outfall		O2,CV (no Q: WR,SP)
339.64	1.08	Free Outfall		O2,CV (no Q: WR,SP)
339.65	1.08	Free Outfall		O2,CV (no Q: WR,SP)
339.66	1.09	Free Outfall		O2,CV (no Q: WR,SP)
339.67	1.10	Free Outfall		O2,CV (no Q: WR,SP)
339.68	1.10	Free Outfall		O2,CV (no Q: WR,SP)
339.69	1.11	Free Outfall		O2,CV (no Q: WR,SP)
339.70	1.11	Free Outfall		O2,CV (no Q: WR,SP)
339.71	1.11	Free Outfall		O2,CV (no Q: WR,SP)
339.72	1.12	Free Outfall		O2,CV (no Q: WR,SP)
339.73	1.12	Free Outfall		O2,CV (no Q: WR,SP)
339.74	1.13	Free Outfall		O2,CV (no Q: WR,SP)
339.75	1.13	Free Outfall		O2,CV (no Q: WR,SP)
339.76	1.14	Free Outfall		O2,CV (no Q: WR,SP)
339.77	1.14	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.78	1.15	Free Outfall		O2,CV (no Q: WR,SP)
339.79	1.15	Free Outfall		O2,CV (no Q: WR,SP)
339.80	1.16	Free Outfall		O2,CV (no Q: WR,SP)
339.81	1.16	Free Outfall		O2,CV (no Q: WR,SP)
339.82	1.17	Free Outfall		O2,CV (no Q: WR,SP)
339.83	1.17	Free Outfall		O2,CV (no Q: WR,SP)
339.84	1.18	Free Outfall		O2,CV (no Q: WR,SP)
339.85	1.18	Free Outfall		O2,CV (no Q: WR,SP)
339.86	1.19	Free Outfall		O2,CV (no Q: WR,SP)
339.87	1.19	Free Outfall		O2,CV (no Q: WR,SP)
339.88	1.20	Free Outfall		O2,CV (no Q: WR,SP)
339.89	1.20	Free Outfall		O2,CV (no Q: WR,SP)
339.90	1.21	Free Outfall		O2,CV (no Q: WR,SP)
339.91	1.21	Free Outfall		O2,CV (no Q: WR,SP)
339.92	1.21	Free Outfall		O2,CV (no Q: WR,SP)
339.93	1.22	Free Outfall		O2,CV (no Q: WR,SP)
339.94	1.22	Free Outfall		O2,CV (no Q: WR,SP)
339.95	1.23	Free Outfall		O2,CV (no Q: WR,SP)
339.96	1.23	Free Outfall		O2,CV (no Q: WR,SP)
339.97	1.24	Free Outfall		O2,CV (no Q: WR,SP)
339.98	1.24	Free Outfall		O2,CV (no Q: WR,SP)
339.99	1.25	Free Outfall		O2,CV (no Q: WR,SP)
340.00	1.25	Free Outfall		O2,CV (no Q: WR,SP)
340.01	1.25	Free Outfall		O2,CV (no Q: WR,SP)
340.02	1.26	Free Outfall		O2,CV (no Q: WR,SP)
340.03	1.26	Free Outfall		O2,CV (no Q: WR,SP)
340.04	1.27	Free Outfall		O2,CV (no Q: WR,SP)
340.05	1.27	Free Outfall		O2,CV (no Q: WR,SP)
340.06	1.28	Free Outfall		O2,CV (no Q: WR,SP)
340.07	1.28	Free Outfall		O2,CV (no Q: WR,SP)
340.08	1.29	Free Outfall		O2,CV (no Q: WR,SP)
340.09	1.29	Free Outfall		O2,CV (no Q: WR,SP)
340.10	1.29	Free Outfall		O2,CV (no Q: WR,SP)
340.11	1.30	Free Outfall		O2,CV (no Q: WR,SP)
340.12	1.30	Free Outfall		O2,CV (no Q: WR,SP)
340.13	1.31	Free Outfall		O2,CV (no Q: WR,SP)
340.14	1.31	Free Outfall		O2,CV (no Q: WR,SP)
340.15	1.31	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.16	1.32	Free Outfall		O2,CV (no Q: WR,SP)
340.17	1.32	Free Outfall		O2,CV (no Q: WR,SP)
340.18	1.33	Free Outfall		O2,CV (no Q: WR,SP)
340.19	1.33	Free Outfall		O2,CV (no Q: WR,SP)
340.20	1.34	Free Outfall		O2,CV (no Q: WR,SP)
340.21	1.34	Free Outfall		O2,CV (no Q: WR,SP)
340.22	1.34	Free Outfall		O2,CV (no Q: WR,SP)
340.23	1.35	Free Outfall		O2,CV (no Q: WR,SP)
340.24	1.35	Free Outfall		O2,CV (no Q: WR,SP)
340.25	1.36	Free Outfall		O2,CV (no Q: WR,SP)
340.26	1.36	Free Outfall		O2,CV (no Q: WR,SP)
340.27	1.36	Free Outfall		O2,CV (no Q: WR,SP)
340.28	1.37	Free Outfall		O2,CV (no Q: WR,SP)
340.29	1.37	Free Outfall		O2,CV (no Q: WR,SP)
340.30	1.38	Free Outfall		O2,CV (no Q: WR,SP)
340.31	1.38	Free Outfall		O2,CV (no Q: WR,SP)
340.32	1.38	Free Outfall		O2,CV (no Q: WR,SP)
340.33	1.39	Free Outfall		O2,CV (no Q: WR,SP)
340.34	1.39	Free Outfall		O2,CV (no Q: WR,SP)
340.35	1.40	Free Outfall		O2,CV (no Q: WR,SP)
340.36	1.40	Free Outfall		O2,CV (no Q: WR,SP)
340.37	1.40	Free Outfall		O2,CV (no Q: WR,SP)
340.38	1.41	Free Outfall		O2,CV (no Q: WR,SP)
340.39	1.41	Free Outfall		O2,CV (no Q: WR,SP)
340.40	1.42	Free Outfall		O2,CV (no Q: WR,SP)
340.41	1.42	Free Outfall		O2,CV (no Q: WR,SP)
340.42	1.42	Free Outfall		O2,CV (no Q: WR,SP)
340.43	1.43	Free Outfall		O2,CV (no Q: WR,SP)
340.44	1.43	Free Outfall		O2,CV (no Q: WR,SP)
340.45	1.44	Free Outfall		O2,CV (no Q: WR,SP)
340.46	1.44	Free Outfall		O2,CV (no Q: WR,SP)
340.47	1.44	Free Outfall		O2,CV (no Q: WR,SP)
340.48	1.45	Free Outfall		O2,CV (no Q: WR,SP)
340.49	1.45	Free Outfall		O2,CV (no Q: WR,SP)
340.50	1.45	Free Outfall		O2,CV (no Q: WR,SP)
340.51	1.46	Free Outfall		O2,CV (no Q: WR,SP)
340.52	1.46	Free Outfall		O2,CV (no Q: WR,SP)
340.53	1.47	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.54	1.47	Free Outfall		O2,CV (no Q: WR,SP)
340.55	1.47	Free Outfall		O2,CV (no Q: WR,SP)
340.56	1.48	Free Outfall		O2,CV (no Q: WR,SP)
340.57	1.48	Free Outfall		O2,CV (no Q: WR,SP)
340.58	1.48	Free Outfall		O2,CV (no Q: WR,SP)
340.59	1.49	Free Outfall		O2,CV (no Q: WR,SP)
340.60	1.49	Free Outfall		O2,CV (no Q: WR,SP)
340.61	1.50	Free Outfall		O2,CV (no Q: WR,SP)
340.62	1.50	Free Outfall		O2,CV (no Q: WR,SP)
340.63	1.50	Free Outfall		O2,CV (no Q: WR,SP)
340.64	1.51	Free Outfall		O2,CV (no Q: WR,SP)
340.65	1.51	Free Outfall		O2,CV (no Q: WR,SP)
340.66	1.51	Free Outfall		O2,CV (no Q: WR,SP)
340.67	1.52	Free Outfall		O2,CV (no Q: WR,SP)
340.68	1.52	Free Outfall		O2,CV (no Q: WR,SP)
340.69	1.52	Free Outfall		O2,CV (no Q: WR,SP)
340.70	1.53	Free Outfall		O2,CV (no Q: WR,SP)
340.71	1.53	Free Outfall		O2,CV (no Q: WR,SP)
340.72	1.54	Free Outfall		O2,CV (no Q: WR,SP)
340.73	1.54	Free Outfall		O2,CV (no Q: WR,SP)
340.74	1.54	Free Outfall		O2,CV (no Q: WR,SP)
340.75	1.55	Free Outfall		O2,CV (no Q: WR,SP)
340.76	1.55	Free Outfall		O2,CV (no Q: WR,SP)
340.77	1.55	Free Outfall		O2,CV (no Q: WR,SP)
340.78	1.56	Free Outfall		O2,CV (no Q: WR,SP)
340.79	1.56	Free Outfall		O2,CV (no Q: WR,SP)
340.80	1.56	Free Outfall		O2,CV (no Q: WR,SP)
340.81	1.57	Free Outfall		O2,CV (no Q: WR,SP)
340.82	1.57	Free Outfall		O2,CV (no Q: WR,SP)
340.83	1.57	Free Outfall		O2,CV (no Q: WR,SP)
340.84	1.58	Free Outfall		O2,CV (no Q: WR,SP)
340.85	1.58	Free Outfall		O2,CV (no Q: WR,SP)
340.86	1.58	Free Outfall		O2,CV (no Q: WR,SP)
340.87	1.59	Free Outfall		O2,CV (no Q: WR,SP)
340.88	1.59	Free Outfall		O2,CV (no Q: WR,SP)
340.89	1.60	Free Outfall		O2,CV (no Q: WR,SP)
340.90	1.60	Free Outfall		O2,CV (no Q: WR,SP)
340.91	1.60	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.92	1.61	Free Outfall		O2,CV (no Q: WR,SP)
340.93	1.61	Free Outfall		O2,CV (no Q: WR,SP)
340.94	1.61	Free Outfall		O2,CV (no Q: WR,SP)
340.95	1.62	Free Outfall		O2,CV (no Q: WR,SP)
340.96	1.62	Free Outfall		O2,CV (no Q: WR,SP)
340.97	1.62	Free Outfall		O2,CV (no Q: WR,SP)
340.98	1.63	Free Outfall		O2,CV (no Q: WR,SP)
340.99	1.63	Free Outfall		O2,CV (no Q: WR,SP)
341.00	1.63	Free Outfall		O2,CV (no Q: WR,SP)
341.01	1.64	Free Outfall		O2,CV (no Q: WR,SP)
341.02	1.64	Free Outfall		O2,CV (no Q: WR,SP)
341.03	1.64	Free Outfall		O2,CV (no Q: WR,SP)
341.04	1.65	Free Outfall		O2,CV (no Q: WR,SP)
341.05	1.65	Free Outfall		O2,CV (no Q: WR,SP)
341.06	1.65	Free Outfall		O2,CV (no Q: WR,SP)
341.07	1.66	Free Outfall		O2,CV (no Q: WR,SP)
341.08	1.66	Free Outfall		O2,CV (no Q: WR,SP)
341.09	1.66	Free Outfall		O2,CV (no Q: WR,SP)
341.10	1.67	Free Outfall		O2,CV (no Q: WR,SP)
341.11	1.67	Free Outfall		O2,CV (no Q: WR,SP)
341.12	1.67	Free Outfall		O2,CV (no Q: WR,SP)
341.13	1.68	Free Outfall		O2,CV (no Q: WR,SP)
341.14	1.68	Free Outfall		O2,CV (no Q: WR,SP)
341.15	1.68	Free Outfall		O2,CV (no Q: WR,SP)
341.16	1.69	Free Outfall		O2,CV (no Q: WR,SP)
341.17	1.69	Free Outfall		O2,CV (no Q: WR,SP)
341.18	1.69	Free Outfall		O2,CV (no Q: WR,SP)
341.19	1.70	Free Outfall		O2,CV (no Q: WR,SP)
341.20	1.70	Free Outfall		O2,CV (no Q: WR,SP)
341.21	1.70	Free Outfall		O2,CV (no Q: WR,SP)
341.22	1.71	Free Outfall		O2,CV (no Q: WR,SP)
341.23	1.71	Free Outfall		O2,CV (no Q: WR,SP)
341.24	1.71	Free Outfall		O2,CV (no Q: WR,SP)
341.25	1.72	Free Outfall		O2,CV (no Q: WR,SP)
341.26	1.72	Free Outfall		O2,CV (no Q: WR,SP)
341.27	1.72	Free Outfall		O2,CV (no Q: WR,SP)
341.28	1.73	Free Outfall		O2,CV (no Q: WR,SP)
341.29	1.73	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.30	1.73	Free Outfall		O2,CV (no Q: WR,SP)
341.31	1.73	Free Outfall		O2,CV (no Q: WR,SP)
341.32	1.74	Free Outfall		O2,CV (no Q: WR,SP)
341.33	1.74	Free Outfall		O2,CV (no Q: WR,SP)
341.34	1.74	Free Outfall		O2,CV (no Q: WR,SP)
341.35	1.75	Free Outfall		O2,CV (no Q: WR,SP)
341.36	1.75	Free Outfall		O2,CV (no Q: WR,SP)
341.37	1.75	Free Outfall		O2,CV (no Q: WR,SP)
341.38	1.76	Free Outfall		O2,CV (no Q: WR,SP)
341.39	1.76	Free Outfall		O2,CV (no Q: WR,SP)
341.40	1.76	Free Outfall		O2,CV (no Q: WR,SP)
341.41	1.77	Free Outfall		O2,CV (no Q: WR,SP)
341.42	1.77	Free Outfall		O2,CV (no Q: WR,SP)
341.43	1.77	Free Outfall		O2,CV (no Q: WR,SP)
341.44	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.45	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.46	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.47	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.48	1.79	Free Outfall		O2,CV (no Q: WR,SP)
341.49	1.79	Free Outfall		O2,CV (no Q: WR,SP)
341.50	1.79	Free Outfall		O2,CV (no Q: WR,SP)
341.51	1.84	Free Outfall		SP,O2,CV (no Q: WR)
341.52	1.93	Free Outfall		SP,O2,CV (no Q: WR)
341.53	2.05	Free Outfall		SP,O2,CV (no Q: WR)
341.54	2.18	Free Outfall		SP,O2,CV (no Q: WR)
341.55	2.34	Free Outfall		SP,O2,CV (no Q: WR)
341.56	2.50	Free Outfall		SP,O2,CV (no Q: WR)
341.57	2.69	Free Outfall		SP,O2,CV (no Q: WR)
341.58	2.88	Free Outfall		SP,O2,CV (no Q: WR)
341.59	3.09	Free Outfall		SP,O2,CV (no Q: WR)
341.60	3.31	Free Outfall		SP,O2,CV (no Q: WR)
341.61	3.55	Free Outfall		SP,O2,CV (no Q: WR)
341.62	3.79	Free Outfall		SP,O2,CV (no Q: WR)
341.63	4.04	Free Outfall		SP,O2,CV (no Q: WR)
341.64	4.31	Free Outfall		SP,O2,CV (no Q: WR)
341.65	4.57	Free Outfall		SP,O2,CV (no Q: WR)
341.66	4.84	Free Outfall		SP,O2,CV (no Q: WR)
341.67	5.12	Free Outfall		SP,O2,CV (no Q: WR)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.68	5.41	Free Outfall		SP,O2,CV (no Q: WR)
341.69	5.71	Free Outfall		SP,O2,CV (no Q: WR)
341.70	6.01	Free Outfall		SP,O2,CV (no Q: WR)
341.71	6.32	Free Outfall		SP,O2,CV (no Q: WR)
341.72	6.64	Free Outfall		SP,O2,CV (no Q: WR)
341.73	6.97	Free Outfall		SP,O2,CV (no Q: WR)
341.74	7.31	Free Outfall		SP,O2,CV (no Q: WR)
341.75	7.65	Free Outfall		SP,O2,CV (no Q: WR)
341.76	8.00	Free Outfall		SP,O2,CV (no Q: WR)
341.77	8.35	Free Outfall		SP,O2,CV (no Q: WR)
341.78	8.72	Free Outfall		SP,O2,CV (no Q: WR)
341.79	9.09	Free Outfall		SP,O2,CV (no Q: WR)
341.80	9.46	Free Outfall		SP,O2,CV (no Q: WR)
341.81	9.85	Free Outfall		SP,O2,CV (no Q: WR)
341.82	10.23	Free Outfall		SP,O2,CV (no Q: WR)
341.83	10.63	Free Outfall		SP,O2,CV (no Q: WR)
341.84	11.03	Free Outfall		SP,O2,CV (no Q: WR)
341.85	11.44	Free Outfall		SP,O2,CV (no Q: WR)
341.86	11.85	Free Outfall		SP,O2,CV (no Q: WR)
341.87	12.27	Free Outfall		SP,O2,CV (no Q: WR)
341.88	12.69	Free Outfall		SP,O2,CV (no Q: WR)
341.89	13.13	Free Outfall		SP,O2,CV (no Q: WR)
341.90	13.56	Free Outfall		SP,O2,CV (no Q: WR)
341.91	14.00	Free Outfall		SP,O2,CV (no Q: WR)
341.92	14.45	Free Outfall		SP,O2,CV (no Q: WR)
341.93	14.90	Free Outfall		SP,O2,CV (no Q: WR)
341.94	15.36	Free Outfall		SP,O2,CV (no Q: WR)
341.95	15.82	Free Outfall		SP,O2,CV (no Q: WR)
341.96	16.29	Free Outfall		SP,O2,CV (no Q: WR)
341.97	16.76	Free Outfall		SP,O2,CV (no Q: WR)
341.98	17.24	Free Outfall		SP,O2,CV (no Q: WR)
341.99	17.72	Free Outfall		SP,O2,CV (no Q: WR)
342.00	18.21	Free Outfall		SP,O2,CV (no Q: WR)
342.01	18.71	Free Outfall		SP,O2,CV (no Q: WR)
342.02	19.20	Free Outfall		SP,O2,CV (no Q: WR)
342.03	19.71	Free Outfall		SP,O2,CV (no Q: WR)
342.04	20.21	Free Outfall		SP,O2,CV (no Q: WR)
342.05	20.72	Free Outfall		SP,O2,CV (no Q: WR)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.06	21.24	Free Outfall		SP,O2,CV (no Q: WR)
342.07	21.76	Free Outfall		SP,O2,CV (no Q: WR)
342.08	22.29	Free Outfall		SP,O2,CV (no Q: WR)
342.09	22.82	Free Outfall		SP,O2,CV (no Q: WR)
342.10	23.36	Free Outfall		SP,O2,CV (no Q: WR)
342.11	23.89	Free Outfall		SP,O2,CV (no Q: WR)
342.12	24.44	Free Outfall		SP,O2,CV (no Q: WR)
342.13	24.99	Free Outfall		SP,O2,CV (no Q: WR)
342.14	25.54	Free Outfall		SP,O2,CV (no Q: WR)
342.15	26.09	Free Outfall		SP,O2,CV (no Q: WR)
342.16	26.65	Free Outfall		SP,O2,CV (no Q: WR)
342.17	27.22	Free Outfall		SP,O2,CV (no Q: WR)
342.18	27.79	Free Outfall		SP,O2,CV (no Q: WR)
342.19	28.36	Free Outfall		SP,O2,CV (no Q: WR)
342.20	28.94	Free Outfall		SP,O2,CV (no Q: WR)
342.21	29.52	Free Outfall		SP,O2,CV (no Q: WR)
342.22	30.10	Free Outfall		SP,O2,CV (no Q: WR)
342.23	30.69	Free Outfall		SP,O2,CV (no Q: WR)
342.24	31.28	Free Outfall		SP,O2,CV (no Q: WR)
342.25	31.88	Free Outfall		SP,O2,CV (no Q: WR)
342.26	32.48	Free Outfall		SP,O2,CV (no Q: WR)
342.27	33.08	Free Outfall		SP,O2,CV (no Q: WR)
342.28	33.69	Free Outfall		SP,O2,CV (no Q: WR)
342.29	34.30	Free Outfall		SP,O2,CV (no Q: WR)
342.30	34.91	Free Outfall		SP,O2,CV (no Q: WR)
342.31	35.53	Free Outfall		SP,O2,CV (no Q: WR)
342.32	36.15	Free Outfall		SP,O2,CV (no Q: WR)
342.33	36.77	Free Outfall		SP,O2,CV (no Q: WR)
342.34	37.39	Free Outfall		SP,O2,CV (no Q: WR)
342.35	38.02	Free Outfall		SP,O2,CV (no Q: WR)
342.36	38.65	Free Outfall		SP,O2,CV (no Q: WR)
342.37	39.28	Free Outfall		SP,O2,CV (no Q: WR)
342.38	39.91	Free Outfall		SP,O2,CV (no Q: WR)
342.39	40.54	Free Outfall		SP,CV (no Q: WR,O2)
342.40	41.17	Free Outfall		SP,CV (no Q: WR,O2)
342.41	41.80	Free Outfall		SP,CV (no Q: WR,O2)
342.42	42.43	Free Outfall		SP,CV (no Q: WR,O2)
342.43	43.06	Free Outfall		SP,CV (no Q: WR,O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.44	43.69	Free Outfall		SP,CV (no Q: WR,O2)
342.45	44.32	Free Outfall		SP,CV (no Q: WR,O2)
342.46	44.95	Free Outfall		SP,CV (no Q: WR,O2)
342.47	45.57	Free Outfall		SP,CV (no Q: WR,O2)
342.48	46.19	Free Outfall		SP,CV (no Q: WR,O2)
342.49	46.79	Free Outfall		SP,CV (no Q: WR,O2)
342.50	47.78	Free Outfall		SP,CV (no Q: WR,O2)
342.51	47.84	Free Outfall		SP,CV (no Q: WR,O2)
342.52	47.91	Free Outfall		SP,CV (no Q: WR,O2)
342.53	47.99	Free Outfall		SP,CV (no Q: WR,O2)
342.54	48.06	Free Outfall		SP,CV (no Q: WR,O2)
342.55	48.13	Free Outfall		SP,CV (no Q: WR,O2)
342.56	48.19	Free Outfall		SP,CV (no Q: WR,O2)
342.57	48.26	Free Outfall		SP,CV (no Q: WR,O2)
342.58	48.34	Free Outfall		SP,CV (no Q: WR,O2)
342.59	48.41	Free Outfall		SP,CV (no Q: WR,O2)
342.60	48.48	Free Outfall		SP,CV (no Q: WR,O2)
342.61	48.54	Free Outfall		SP,CV (no Q: WR,O2)
342.62	48.61	Free Outfall		SP,CV (no Q: WR,O2)
342.63	48.70	Free Outfall		SP,CV (no Q: WR,O2)
342.64	48.78	Free Outfall		SP,CV (no Q: WR,O2)
342.65	48.83	Free Outfall		SP,CV (no Q: WR,O2)
342.66	48.91	Free Outfall		SP,CV (no Q: WR,O2)
342.67	48.98	Free Outfall		SP,CV (no Q: WR,O2)
342.68	49.06	Free Outfall		SP,CV (no Q: WR,O2)
342.69	49.12	Free Outfall		SP,CV (no Q: WR,O2)
342.70	49.19	Free Outfall		SP,CV (no Q: WR,O2)
342.71	49.26	Free Outfall		SP,CV (no Q: WR,O2)
342.72	49.33	Free Outfall		SP,CV (no Q: WR,O2)
342.73	49.40	Free Outfall		SP,CV (no Q: WR,O2)
342.74	49.47	Free Outfall		SP,CV (no Q: WR,O2)
342.75	49.53	Free Outfall		SP,CV (no Q: WR,O2)
342.76	49.60	Free Outfall		SP,CV (no Q: WR,O2)
342.77	49.68	Free Outfall		SP,CV (no Q: WR,O2)
342.78	49.74	Free Outfall		SP,CV (no Q: WR,O2)
342.79	49.81	Free Outfall		SP,CV (no Q: WR,O2)
342.80	49.88	Free Outfall		SP,CV (no Q: WR,O2)
342.81	49.95	Free Outfall		SP,CV (no Q: WR,O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
Elev. ft	Q cfs	TW Elev ft	Converge Error +/-ft	Contributing Structures
342.82	50.02	Free Outfall		SP,CV (no Q: WR,O2)
342.83	50.09	Free Outfall		SP,CV (no Q: WR,O2)
342.84	50.15	Free Outfall		SP,CV (no Q: WR,O2)
342.85	50.23	Free Outfall		SP,CV (no Q: WR,O2)
342.86	50.29	Free Outfall		SP,CV (no Q: WR,O2)
342.87	50.36	Free Outfall		SP,CV (no Q: WR,O2)
342.88	50.43	Free Outfall		SP,CV (no Q: WR,O2)
342.89	50.50	Free Outfall		SP,CV (no Q: WR,O2)
342.90	50.57	Free Outfall		SP,CV (no Q: WR,O2)
342.91	50.63	Free Outfall		SP,CV (no Q: WR,O2)
342.92	50.71	Free Outfall		SP,CV (no Q: WR,O2)
342.93	50.76	Free Outfall		SP,CV (no Q: WR,O2)
342.94	50.84	Free Outfall		SP,CV (no Q: WR,O2)
342.95	50.90	Free Outfall		SP,CV (no Q: WR,O2)
342.96	50.98	Free Outfall		SP,CV (no Q: WR,O2)
342.97	51.03	Free Outfall		SP,CV (no Q: WR,O2)
342.98	51.11	Free Outfall		SP,CV (no Q: WR,O2)
342.99	51.17	Free Outfall		SP,CV (no Q: WR,O2)
343.00	51.24	Free Outfall		SP,CV (no Q: WR,O2)
343.01	51.37	Free Outfall		WR,SP,CV (no Q: O2)
343.02	51.56	Free Outfall		WR,SP,CV (no Q: O2)
343.03	51.77	Free Outfall		WR,SP,CV (no Q: O2)
343.04	52.01	Free Outfall		WR,SP,CV (no Q: O2)
343.05	52.26	Free Outfall		WR,SP,CV (no Q: O2)
343.06	52.55	Free Outfall		WR,SP,CV (no Q: O2)
343.07	52.84	Free Outfall		WR,SP,CV (no Q: O2)
343.08	53.16	Free Outfall		WR,SP,CV (no Q: O2)
343.09	53.48	Free Outfall		WR,SP,CV (no Q: O2)
343.10	53.83	Free Outfall		WR,SP,CV (no Q: O2)
343.11	54.19	Free Outfall		WR,SP,CV (no Q: O2)
343.12	54.55	Free Outfall		WR,SP,CV (no Q: O2)
343.13	54.94	Free Outfall		WR,SP,CV (no Q: O2)
343.14	55.34	Free Outfall		WR,SP,CV (no Q: O2)
343.15	55.75	Free Outfall		WR,SP,CV (no Q: O2)
343.16	56.17	Free Outfall		WR,SP,CV (no Q: O2)
343.17	56.60	Free Outfall		WR,SP,CV (no Q: O2)
343.18	57.04	Free Outfall		WR,SP,CV (no Q: O2)
343.19	57.50	Free Outfall		WR,SP,CV (no Q: O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.20	57.96	Free Outfall		WR,SP,CV (no Q: O2)
343.21	58.43	Free Outfall		WR,SP,CV (no Q: O2)
343.22	58.92	Free Outfall		WR,SP,CV (no Q: O2)
343.23	59.41	Free Outfall		WR,SP,CV (no Q: O2)
343.24	59.90	Free Outfall		WR,SP,CV (no Q: O2)
343.25	60.41	Free Outfall		WR,SP,CV (no Q: O2)
343.26	60.94	Free Outfall		WR,SP,CV (no Q: O2)
343.27	61.46	Free Outfall		WR,SP,CV (no Q: O2)
343.28	62.00	Free Outfall		WR,SP,CV (no Q: O2)
343.29	62.54	Free Outfall		WR,SP,CV (no Q: O2)
343.30	63.09	Free Outfall		WR,SP,CV (no Q: O2)
343.31	63.66	Free Outfall		WR,SP,CV (no Q: O2)
343.32	64.22	Free Outfall		WR,SP,CV (no Q: O2)
343.33	64.80	Free Outfall		WR,SP,CV (no Q: O2)
343.34	65.38	Free Outfall		WR,SP,CV (no Q: O2)
343.35	65.97	Free Outfall		WR,SP,CV (no Q: O2)
343.36	66.57	Free Outfall		WR,SP,CV (no Q: O2)
343.37	67.18	Free Outfall		WR,SP,CV (no Q: O2)
343.38	67.79	Free Outfall		WR,SP,CV (no Q: O2)
343.39	68.42	Free Outfall		WR,SP,CV (no Q: O2)
343.40	69.04	Free Outfall		WR,SP,CV (no Q: O2)
343.41	69.67	Free Outfall		WR,SP,CV (no Q: O2)
343.42	70.31	Free Outfall		WR,SP,CV (no Q: O2)
343.43	70.95	Free Outfall		WR,SP,CV (no Q: O2)
343.44	71.61	Free Outfall		WR,SP,CV (no Q: O2)
343.45	72.28	Free Outfall		WR,SP,CV (no Q: O2)
343.46	72.94	Free Outfall		WR,SP,CV (no Q: O2)
343.47	73.61	Free Outfall		WR,SP,CV (no Q: O2)
343.48	74.30	Free Outfall		WR,SP,CV (no Q: O2)
343.49	74.98	Free Outfall		WR,SP,CV (no Q: O2)
343.50	75.68	Free Outfall		WR,SP,CV (no Q: O2)
343.51	76.37	Free Outfall		WR,SP,CV (no Q: O2)
343.52	77.08	Free Outfall		WR,SP,CV (no Q: O2)
343.53	77.78	Free Outfall		WR,SP,CV (no Q: O2)
343.54	78.50	Free Outfall		WR,SP,CV (no Q: O2)
343.55	79.22	Free Outfall		WR,SP,CV (no Q: O2)
343.56	79.95	Free Outfall		WR,SP,CV (no Q: O2)
343.57	80.69	Free Outfall		WR,SP,CV (no Q: O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.58	81.43	Free Outfall		WR,SP,CV (no Q: O2)
343.59	82.18	Free Outfall		WR,SP,CV (no Q: O2)
343.60	82.93	Free Outfall		WR,SP,CV (no Q: O2)
343.61	83.68	Free Outfall		WR,SP,CV (no Q: O2)
343.62	84.43	Free Outfall		WR,SP,CV (no Q: O2)
343.63	85.24	Free Outfall		WR,SP,CV (no Q: O2)
343.64	86.01	Free Outfall		WR,SP,CV (no Q: O2)
343.65	86.79	Free Outfall		WR,SP,CV (no Q: O2)
343.66	87.57	Free Outfall		WR,SP,CV (no Q: O2)
343.67	88.37	Free Outfall		WR,SP,CV (no Q: O2)
343.68	89.16	Free Outfall		WR,SP,CV (no Q: O2)
343.69	89.96	Free Outfall		WR,SP,CV (no Q: O2)
343.70	90.76	Free Outfall		WR,SP,CV (no Q: O2)
343.71	91.58	Free Outfall		WR,SP,CV (no Q: O2)
343.72	92.40	Free Outfall		WR,SP,CV (no Q: O2)
343.73	93.21	Free Outfall		WR,SP,CV (no Q: O2)
343.74	94.04	Free Outfall		WR,SP,CV (no Q: O2)
343.75	94.87	Free Outfall		WR,SP,CV (no Q: O2)
343.76	95.71	Free Outfall		WR,SP,CV (no Q: O2)
343.77	96.54	Free Outfall		WR,SP,CV (no Q: O2)
343.78	97.40	Free Outfall		WR,SP,CV (no Q: O2)
343.79	98.25	Free Outfall		WR,SP,CV (no Q: O2)
343.80	99.10	Free Outfall		WR,SP,CV (no Q: O2)
343.81	99.96	Free Outfall		WR,SP,CV (no Q: O2)
343.82	100.82	Free Outfall		WR,SP,CV (no Q: O2)
343.83	101.69	Free Outfall		WR,SP,CV (no Q: O2)
343.84	102.57	Free Outfall		WR,SP,CV (no Q: O2)
343.85	103.45	Free Outfall		WR,SP,CV (no Q: O2)
343.86	104.33	Free Outfall		WR,SP,CV (no Q: O2)
343.87	105.22	Free Outfall		WR,SP,CV (no Q: O2)
343.88	106.11	Free Outfall		WR,SP,CV (no Q: O2)
343.89	107.01	Free Outfall		WR,SP,CV (no Q: O2)
343.90	107.91	Free Outfall		WR,SP,CV (no Q: O2)
343.91	108.81	Free Outfall		WR,SP,CV (no Q: O2)
343.92	109.72	Free Outfall		WR,SP,CV (no Q: O2)
343.93	110.64	Free Outfall		WR,SP,CV (no Q: O2)
343.94	111.55	Free Outfall		WR,SP,CV (no Q: O2)
343.95	112.48	Free Outfall		WR,SP,CV (no Q: O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.96	113.40	Free Outfall		WR,SP,CV (no Q: O2)
343.97	114.34	Free Outfall		WR,SP,CV (no Q: O2)
343.98	115.28	Free Outfall		WR,SP,CV (no Q: O2)
343.99	116.22	Free Outfall		WR,SP,CV (no Q: O2)
344.00	117.17	Free Outfall		WR,SP,CV (no Q: O2)
344.01	118.11	Free Outfall		WR,SP,CV (no Q: O2)
344.02	119.07	Free Outfall		WR,SP,CV (no Q: O2)
344.03	120.03	Free Outfall		WR,SP,CV (no Q: O2)
344.04	120.99	Free Outfall		WR,SP,CV (no Q: O2)
344.05	121.95	Free Outfall		WR,SP,CV (no Q: O2)
344.06	122.92	Free Outfall		WR,SP,CV (no Q: O2)
344.07	123.90	Free Outfall		WR,SP,CV (no Q: O2)
344.08	124.88	Free Outfall		WR,SP,CV (no Q: O2)
344.09	125.86	Free Outfall		WR,SP,CV (no Q: O2)
344.10	126.85	Free Outfall		WR,SP,CV (no Q: O2)
344.11	127.83	Free Outfall		WR,SP,CV (no Q: O2)
344.12	128.83	Free Outfall		WR,SP,CV (no Q: O2)
344.13	129.83	Free Outfall		WR,SP,CV (no Q: O2)
344.14	130.83	Free Outfall		WR,SP,CV (no Q: O2)
344.15	131.83	Free Outfall		WR,SP,CV (no Q: O2)
344.16	132.84	Free Outfall		WR,SP,CV (no Q: O2)
344.17	133.86	Free Outfall		WR,SP,CV (no Q: O2)
344.18	134.88	Free Outfall		WR,SP,CV (no Q: O2)
344.19	135.90	Free Outfall		WR,SP,CV (no Q: O2)
344.20	136.93	Free Outfall		WR,SP,CV (no Q: O2)
344.21	137.96	Free Outfall		WR,SP,CV (no Q: O2)
344.22	138.99	Free Outfall		WR,SP,CV (no Q: O2)
344.23	140.03	Free Outfall		WR,SP,CV (no Q: O2)
344.24	141.06	Free Outfall		WR,SP,CV (no Q: O2)
344.25	142.10	Free Outfall		WR,SP,CV (no Q: O2)
344.26	143.15	Free Outfall		WR,SP,CV (no Q: O2)
344.27	144.21	Free Outfall		WR,SP,CV (no Q: O2)
344.28	145.27	Free Outfall		WR,SP,CV (no Q: O2)
344.29	146.32	Free Outfall		WR,SP,CV (no Q: O2)
344.30	147.38	Free Outfall		WR,SP,CV (no Q: O2)
344.31	148.46	Free Outfall		WR,SP,CV (no Q: O2)
344.32	149.52	Free Outfall		WR,SP,CV (no Q: O2)
344.33	150.60	Free Outfall		WR,SP,CV (no Q: O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.34	151.68	Free Outfall		WR,SP,CV (no Q: O2)
344.35	152.76	Free Outfall		WR,SP,CV (no Q: O2)
344.36	153.83	Free Outfall		WR,SP,CV (no Q: O2)
344.37	154.93	Free Outfall		WR,SP,CV (no Q: O2)
344.38	156.02	Free Outfall		WR,SP,CV (no Q: O2)
344.39	157.12	Free Outfall		WR,SP,CV (no Q: O2)
344.40	158.21	Free Outfall		WR,SP,CV (no Q: O2)
344.41	159.32	Free Outfall		WR,SP,CV (no Q: O2)
344.42	160.42	Free Outfall		WR,SP,CV (no Q: O2)
344.43	161.53	Free Outfall		WR,SP,CV (no Q: O2)
344.44	162.65	Free Outfall		WR,SP,CV (no Q: O2)
344.45	163.76	Free Outfall		WR,SP,CV (no Q: O2)
344.46	164.88	Free Outfall		WR,SP,CV (no Q: O2)
344.47	166.00	Free Outfall		WR,SP,CV (no Q: O2)
344.48	167.13	Free Outfall		WR,SP,CV (no Q: O2)
344.49	168.25	Free Outfall		WR,SP,CV (no Q: O2)
344.50	169.39	Free Outfall		WR,SP,CV (no Q: O2)
344.51	170.53	Free Outfall		WR,SP,CV (no Q: O2)
344.52	171.67	Free Outfall		WR,SP,CV (no Q: O2)
344.53	172.80	Free Outfall		WR,SP,CV (no Q: O2)
344.54	173.95	Free Outfall		WR,SP,CV (no Q: O2)
344.55	175.10	Free Outfall		WR,SP,CV (no Q: O2)
344.56	176.25	Free Outfall		WR,SP,CV (no Q: O2)
344.57	177.42	Free Outfall		WR,SP,CV (no Q: O2)
344.58	178.57	Free Outfall		WR,SP,CV (no Q: O2)
344.59	179.73	Free Outfall		WR,SP,CV (no Q: O2)
344.60	180.90	Free Outfall		WR,SP,CV (no Q: O2)
344.61	182.06	Free Outfall		WR,SP,CV (no Q: O2)
344.62	183.24	Free Outfall		WR,SP,CV (no Q: O2)
344.63	184.41	Free Outfall		WR,SP,CV (no Q: O2)
344.64	185.60	Free Outfall		WR,SP,CV (no Q: O2)
344.65	186.77	Free Outfall		WR,SP,CV (no Q: O2)
344.66	187.96	Free Outfall		WR,SP,CV (no Q: O2)
344.67	189.14	Free Outfall		WR,SP,CV (no Q: O2)
344.68	190.34	Free Outfall		WR,SP,CV (no Q: O2)
344.69	191.53	Free Outfall		WR,SP,CV (no Q: O2)
344.70	192.73	Free Outfall		WR,SP,CV (no Q: O2)
344.71	193.92	Free Outfall		WR,SP,CV (no Q: O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Converge TW Elev ft	Converge Error +/-ft	Notes Contributing Structures
344.72	195.13	Free Outfall		WR,SP,CV (no Q: O2)
344.73	196.33	Free Outfall		WR,SP,CV (no Q: O2)
344.74	197.54	Free Outfall		WR,SP,CV (no Q: O2)
344.75	198.75	Free Outfall		WR,SP,CV (no Q: O2)
344.76	199.97	Free Outfall		WR,SP,CV (no Q: O2)
344.77	201.30	Free Outfall		WR,SP,CV (no Q: O2)
344.78	202.52	Free Outfall		WR,SP,CV (no Q: O2)
344.79	203.74	Free Outfall		WR,SP,CV (no Q: O2)
344.80	204.97	Free Outfall		WR,SP,CV (no Q: O2)
344.81	206.20	Free Outfall		WR,SP,CV (no Q: O2)
344.82	207.44	Free Outfall		WR,SP,CV (no Q: O2)
344.83	208.68	Free Outfall		WR,SP,CV (no Q: O2)
344.84	209.92	Free Outfall		WR,SP,CV (no Q: O2)
344.85	211.16	Free Outfall		WR,SP,CV (no Q: O2)
344.86	212.41	Free Outfall		WR,SP,CV (no Q: O2)
344.87	213.66	Free Outfall		WR,SP,CV (no Q: O2)
344.88	214.92	Free Outfall		WR,SP,CV (no Q: O2)
344.89	216.17	Free Outfall		WR,SP,CV (no Q: O2)
344.90	217.43	Free Outfall		WR,SP,CV (no Q: O2)
344.91	218.68	Free Outfall		WR,SP,CV (no Q: O2)
344.92	219.94	Free Outfall		WR,SP,CV (no Q: O2)
344.93	221.21	Free Outfall		WR,SP,CV (no Q: O2)
344.94	222.48	Free Outfall		WR,SP,CV (no Q: O2)
344.95	223.75	Free Outfall		WR,SP,CV (no Q: O2)
344.96	225.03	Free Outfall		WR,SP,CV (no Q: O2)
344.97	226.31	Free Outfall		WR,SP,CV (no Q: O2)
344.98	227.60	Free Outfall		WR,SP,CV (no Q: O2)
344.99	228.87	Free Outfall		WR,SP,CV (no Q: O2)
345.00	230.17	Free Outfall		WR,SP,CV (no Q: O2)

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
337.50	.00	.000	1.4000	.00	.00	.00
337.51	.00	.014	1.4010	.00	.00	67.85
337.52	.00	.028	1.4020	.00	.00	135.54
337.53	.00	.042	1.4030	.00	.00	203.49
337.54	.00	.056	1.4040	.00	.00	271.48
337.55	.00	.070	1.4050	.00	.00	339.32
337.56	.00	.084	1.4060	.00	.00	407.41
337.57	.00	.098	1.4069	.00	.00	475.55
337.58	.00	.112	1.4079	.00	.00	543.53
337.59	.00	.126	1.4089	.00	.00	611.76
337.60	.00	.141	1.4099	.00	.00	680.04
337.61	.00	.155	1.4109	.00	.00	748.17
337.62	.00	.169	1.4119	.00	.00	816.55
337.63	.00	.183	1.4129	.00	.00	884.97
337.64	.12	.197	1.4139	.00	.12	953.57
337.65	.14	.211	1.4149	.00	.14	1021.91
337.66	.14	.225	1.4159	.00	.14	1090.48
337.67	.16	.239	1.4169	.00	.16	1159.12
337.68	.16	.254	1.4179	.00	.16	1227.58
337.69	.16	.268	1.4189	.00	.16	1296.30

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
337.70	.17	.282	1.4199	.00	.17	1365.07
337.71	.17	.296	1.4209	.00	.17	1433.68
337.72	.17	.310	1.4219	.00	.17	1502.54
337.73	.20	.325	1.4229	.00	.20	1571.48
337.74	.22	.339	1.4239	.00	.22	1640.25
337.75	.22	.353	1.4249	.00	.22	1709.26
337.76	.22	.367	1.4259	.00	.22	1778.32
337.77	.22	.382	1.4269	.00	.22	1847.21
337.78	.26	.396	1.4279	.00	.26	1916.40
337.79	.26	.410	1.4289	.00	.26	1985.60
337.80	.26	.424	1.4299	.00	.26	2054.64
337.81	.26	.439	1.4309	.00	.26	2123.94
337.82	.26	.453	1.4319	.00	.26	2193.29
337.83	.26	.467	1.4329	.00	.26	2262.47
337.84	.26	.482	1.4339	.00	.26	2331.92
337.85	.26	.496	1.4349	.00	.26	2401.41
337.86	.51	.510	1.4359	.00	.51	2470.99
337.87	.52	.525	1.4369	.00	.52	2540.59
337.88	.52	.539	1.4379	.00	.52	2610.23
337.89	.54	.554	1.4389	.00	.54	2679.94

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
337.90	.55	.568	1.4399	.00	.55	2749.47
337.91	.55	.582	1.4409	.00	.55	2819.26
337.92	.55	.597	1.4419	.00	.55	2889.09
337.93	.55	.611	1.4429	.00	.55	2958.76
337.94	.55	.626	1.4440	.00	.55	3028.69
337.95	.55	.640	1.4450	.00	.55	3098.67
337.96	.55	.655	1.4460	.00	.55	3168.49
337.97	.55	.669	1.4470	.00	.55	3238.56
337.98	.55	.683	1.4480	.00	.55	3308.69
337.99	.55	.698	1.4490	.00	.55	3378.65
338.00	.55	.712	1.4500	.00	.55	3448.88
338.01	.55	.727	1.4510	.00	.55	3519.15
338.02	.55	.741	1.4520	.00	.55	3589.25
338.03	.55	.756	1.4530	.00	.55	3659.62
338.04	.55	.771	1.4539	.00	.55	3730.04
338.05	.55	.785	1.4549	.00	.55	3800.29
338.06	.55	.800	1.4559	.00	.55	3870.80
338.07	.55	.814	1.4569	.00	.55	3941.35
338.08	.55	.829	1.4579	.00	.55	4011.75
338.09	.55	.843	1.4589	.00	.55	4082.40

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.10	.55	.858	1.4599	.00	.55	4153.10
338.11	.55	.873	1.4608	.00	.55	4223.64
338.12	.55	.887	1.4618	.00	.55	4294.43
338.13	.55	.902	1.4628	.00	.55	4365.28
338.14	.55	.916	1.4638	.00	.55	4436.17
338.15	.55	.931	1.4648	.00	.55	4506.90
338.16	.55	.946	1.4658	.00	.55	4577.89
338.17	.55	.960	1.4668	.00	.55	4648.92
338.18	.55	.975	1.4678	.00	.55	4719.79
338.19	.56	.990	1.4687	.00	.56	4790.93
338.20	.56	1.004	1.4697	.00	.56	4862.11
338.21	.57	1.019	1.4707	.00	.57	4933.13
338.22	.57	1.034	1.4717	.00	.57	5004.41
338.23	.58	1.049	1.4727	.00	.58	5075.75
338.24	.59	1.063	1.4737	.00	.59	5146.91
338.25	.60	1.078	1.4747	.00	.60	5218.34
338.26	.60	1.093	1.4757	.00	.60	5289.81
338.27	.61	1.108	1.4767	.00	.61	5361.12
338.28	.62	1.122	1.4777	.00	.62	5432.69
338.29	.63	1.137	1.4787	.00	.63	5504.31

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

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Pond Volume Data = POND 1
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.30	.63	1.152	1.4796	.00	.63	5575.76
338.31	.64	1.167	1.4806	.00	.64	5647.48
338.32	.65	1.182	1.4816	.00	.65	5719.24
338.33	.65	1.196	1.4826	.00	.65	5790.83
338.34	.66	1.211	1.4836	.00	.66	5862.69
338.35	.67	1.226	1.4846	.00	.67	5934.60
338.36	.67	1.241	1.4856	.00	.67	6006.34
338.37	.68	1.256	1.4866	.00	.68	6078.34
338.38	.69	1.271	1.4876	.00	.69	6150.40
338.39	.69	1.285	1.4886	.00	.69	6222.50
338.40	.70	1.300	1.4896	.00	.70	6294.43
338.41	.71	1.315	1.4906	.00	.71	6366.63
338.42	.71	1.330	1.4916	.00	.71	6438.87
338.43	.72	1.345	1.4926	.00	.72	6510.94
338.44	.73	1.360	1.4936	.00	.73	6583.29
338.45	.73	1.375	1.4946	.00	.73	6655.68
338.46	.74	1.390	1.4956	.00	.74	6727.90
338.47	.75	1.405	1.4966	.00	.75	6800.38
338.48	.75	1.420	1.4976	.00	.75	6872.92
338.49	.76	1.435	1.4986	.00	.76	6945.28

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

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Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.50	.76	1.450	1.4996	.00	.76	7017.92
338.51	.77	1.465	1.5006	.00	.77	7090.60
338.52	.78	1.480	1.5016	.00	.78	7163.10
338.53	.79	1.495	1.5026	.00	.79	7235.89
338.54	.80	1.510	1.5036	.00	.80	7308.72
338.55	.82	1.525	1.5046	.00	.82	7381.39
338.56	.84	1.540	1.5056	.00	.84	7454.32
338.57	.85	1.555	1.5066	.00	.85	7527.30
338.58	.88	1.570	1.5076	.00	.88	7600.12
338.59	.90	1.585	1.5086	.00	.90	7673.21
338.60	.93	1.600	1.5096	.00	.93	7746.34
338.61	.95	1.615	1.5106	.00	.95	7819.30
338.62	.98	1.630	1.5116	.00	.98	7892.54
338.63	1.01	1.646	1.5126	.00	1.01	7965.83
338.64	1.03	1.661	1.5136	.00	1.03	8039.16
338.65	1.06	1.676	1.5146	.00	1.06	8112.32
338.66	1.09	1.691	1.5156	.00	1.09	8185.75
338.67	1.13	1.706	1.5166	.00	1.13	8259.24
338.68	1.15	1.721	1.5176	.00	1.15	8332.54
338.69	1.17	1.737	1.5186	.00	1.17	8406.11

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

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Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.70	1.19	1.752	1.5197	.00	1.19	8479.73
338.71	1.21	1.767	1.5207	.00	1.21	8553.17
338.72	1.23	1.782	1.5217	.00	1.23	8626.89
338.73	1.25	1.797	1.5227	.00	1.25	8700.65
338.74	1.27	1.813	1.5237	.00	1.27	8774.24
338.75	1.28	1.828	1.5247	.00	1.28	8848.10
338.76	1.30	1.843	1.5257	.00	1.30	8922.01
338.77	1.32	1.858	1.5267	.00	1.32	8995.74
338.78	1.33	1.874	1.5277	.00	1.33	9069.74
338.79	1.35	1.889	1.5287	.00	1.35	9143.80
338.80	1.36	1.904	1.5297	.00	1.36	9217.67
338.81	1.38	1.920	1.5307	.00	1.38	9291.82
338.82	1.39	1.935	1.5318	.00	1.39	9366.02
338.83	1.41	1.950	1.5328	.00	1.41	9440.05
338.84	1.42	1.965	1.5338	.00	1.42	9514.34
338.85	1.44	1.981	1.5348	.00	1.44	9588.69
338.86	1.45	1.996	1.5358	.00	1.45	9662.86
338.87	1.47	2.012	1.5368	.00	1.47	9737.30
338.88	1.48	2.027	1.5378	.00	1.48	9811.79
338.89	1.49	2.042	1.5388	.00	1.49	9886.34

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
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Starting Outflow = .00 cfs
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.90	1.51	2.058	1.5398	.00	1.51	9960.70
338.91	1.52	2.073	1.5409	.00	1.52	10035.34
338.92	1.54	2.089	1.5419	.00	1.54	10110.03
338.93	1.55	2.104	1.5429	.00	1.55	10184.54
338.94	1.56	2.119	1.5439	.00	1.56	10259.32
338.95	1.57	2.135	1.5449	.00	1.57	10334.16
338.96	1.59	2.150	1.5459	.00	1.59	10408.82
338.97	1.60	2.166	1.5470	.00	1.60	10483.75
338.98	1.61	2.181	1.5480	.00	1.61	10558.73
338.99	1.63	2.197	1.5490	.00	1.63	10633.54
339.00	1.64	2.212	1.5500	.00	1.64	10708.62
339.01	1.65	2.228	1.5511	.00	1.65	10783.75
339.02	1.66	2.243	1.5522	.00	1.66	10858.70
339.03	1.68	2.259	1.5532	.00	1.68	10933.94
339.04	1.69	2.274	1.5543	.00	1.69	11009.23
339.05	1.70	2.290	1.5554	.00	1.70	11084.34
339.06	1.71	2.305	1.5565	.00	1.71	11159.73
339.07	1.72	2.321	1.5576	.00	1.72	11235.18
339.08	1.73	2.337	1.5587	.00	1.73	11310.45
339.09	1.75	2.352	1.5597	.00	1.75	11386.00

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.10	1.76	2.368	1.5608	.00	1.76	11461.60
339.11	1.77	2.383	1.5619	.00	1.77	11537.03
339.12	1.78	2.399	1.5630	.00	1.78	11612.74
339.13	1.79	2.415	1.5641	.00	1.79	11688.50
339.14	1.80	2.430	1.5652	.00	1.80	11764.31
339.15	1.81	2.446	1.5663	.00	1.81	11839.95
339.16	1.83	2.462	1.5673	.00	1.83	11915.86
339.17	1.84	2.477	1.5684	.00	1.84	11991.83
339.18	1.85	2.493	1.5695	.00	1.85	12067.63
339.19	1.86	2.509	1.5706	.00	1.86	12143.70
339.20	1.87	2.524	1.5717	.00	1.87	12219.83
339.21	1.88	2.540	1.5728	.00	1.88	12295.78
339.22	1.89	2.556	1.5739	.00	1.89	12372.02
339.23	1.90	2.572	1.5750	.00	1.90	12448.30
339.24	1.91	2.587	1.5761	.00	1.91	12524.41
339.25	1.92	2.603	1.5771	.00	1.92	12600.80
339.26	1.93	2.619	1.5782	.00	1.93	12677.25
339.27	1.94	2.635	1.5793	.00	1.94	12753.51
339.28	1.96	2.650	1.5804	.00	1.96	12830.06
339.29	1.97	2.666	1.5815	.00	1.97	12906.67

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.30	1.98	2.682	1.5826	.00	1.98	12983.09
339.31	1.99	2.698	1.5837	.00	1.99	13059.80
339.32	2.00	2.714	1.5848	.00	2.00	13136.56
339.33	2.01	2.730	1.5859	.00	2.01	13213.14
339.34	2.02	2.745	1.5870	.00	2.02	13290.01
339.35	2.03	2.761	1.5881	.00	2.03	13366.93
339.36	2.04	2.777	1.5892	.00	2.04	13443.67
339.37	2.05	2.793	1.5903	.00	2.05	13520.70
339.38	2.06	2.809	1.5914	.00	2.06	13597.78
339.39	2.07	2.825	1.5925	.00	2.07	13674.91
339.40	2.08	2.841	1.5935	.00	2.08	13751.87
339.41	2.09	2.857	1.5946	.00	2.09	13829.10
339.42	2.10	2.873	1.5957	.00	2.10	13906.40
339.43	2.11	2.889	1.5968	.00	2.11	13983.51
339.44	2.12	2.905	1.5979	.00	2.12	14060.91
339.45	2.13	2.921	1.5990	.00	2.13	14138.36
339.46	2.13	2.937	1.6001	.00	2.13	14215.63
339.47	2.14	2.953	1.6012	.00	2.14	14293.18
339.48	2.15	2.969	1.6023	.00	2.15	14370.79
339.49	2.16	2.985	1.6034	.00	2.16	14448.22

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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Pond Volume Data = POND 1
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.50	2.17	3.001	1.6045	.00	2.17	14525.94
339.51	2.18	3.017	1.6056	.00	2.18	14603.71
339.52	2.19	3.033	1.6067	.00	2.19	14681.30
339.53	2.20	3.049	1.6078	.00	2.20	14759.18
339.54	2.21	3.065	1.6089	.00	2.21	14837.11
339.55	2.22	3.081	1.6100	.00	2.22	14914.86
339.56	2.23	3.097	1.6111	.00	2.23	14992.89
339.57	2.24	3.113	1.6122	.00	2.24	15070.98
339.58	2.25	3.129	1.6133	.00	2.25	15148.89
339.59	2.26	3.146	1.6144	.00	2.26	15227.09
339.60	2.27	3.162	1.6155	.00	2.27	15305.34
339.61	2.27	3.178	1.6167	.00	2.27	15383.40
339.62	2.28	3.194	1.6178	.00	2.28	15461.76
339.63	2.29	3.210	1.6189	.00	2.29	15540.17
339.64	2.30	3.227	1.6200	.00	2.30	15618.64
339.65	2.31	3.243	1.6211	.00	2.31	15696.92
339.66	2.32	3.259	1.6222	.00	2.32	15775.49
339.67	2.33	3.275	1.6233	.00	2.33	15854.12
339.68	2.34	3.291	1.6244	.00	2.34	15932.56
339.69	2.35	3.308	1.6255	.00	2.35	16011.29

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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Pond Volume Data = POND 1
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INITIAL CONDITIONS

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Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.70	2.35	3.324	1.6266	.00	2.35	16090.08
339.71	2.36	3.340	1.6277	.00	2.36	16168.67
339.72	2.37	3.356	1.6288	.00	2.37	16247.57
339.73	2.38	3.373	1.6299	.00	2.38	16326.52
339.74	2.39	3.389	1.6310	.00	2.39	16405.28
339.75	2.40	3.405	1.6321	.00	2.40	16484.33
339.76	2.41	3.422	1.6333	.00	2.41	16563.44
339.77	2.42	3.438	1.6344	.00	2.42	16642.36
339.78	2.42	3.454	1.6355	.00	2.42	16721.58
339.79	2.43	3.471	1.6366	.00	2.43	16800.85
339.80	2.44	3.487	1.6377	.00	2.44	16879.93
339.81	2.45	3.503	1.6388	.00	2.45	16959.30
339.82	2.46	3.520	1.6399	.00	2.46	17038.73
339.83	2.47	3.536	1.6410	.00	2.47	17117.98
339.84	2.47	3.553	1.6421	.00	2.47	17197.52
339.85	2.48	3.569	1.6433	.00	2.48	17277.11
339.86	2.49	3.586	1.6444	.00	2.49	17356.52
339.87	2.50	3.602	1.6455	.00	2.50	17436.21
339.88	2.51	3.618	1.6466	.00	2.51	17515.97
339.89	2.52	3.635	1.6477	.00	2.52	17595.78

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.90	2.52	3.651	1.6488	.00	2.52	17675.40
339.91	2.53	3.668	1.6499	.00	2.53	17755.31
339.92	2.54	3.684	1.6511	.00	2.54	17835.29
339.93	2.55	3.701	1.6522	.00	2.55	17915.07
339.94	2.56	3.717	1.6533	.00	2.56	17995.14
339.95	2.56	3.734	1.6544	.00	2.56	18075.28
339.96	2.57	3.751	1.6555	.00	2.57	18155.22
339.97	2.58	3.767	1.6566	.00	2.58	18235.46
339.98	2.59	3.784	1.6578	.00	2.59	18315.76
339.99	2.60	3.800	1.6589	.00	2.60	18395.86
340.00	2.60	3.817	1.6600	.00	2.60	18476.26
340.01	2.61	3.833	1.6610	.00	2.61	18556.72
340.02	2.62	3.850	1.6620	.00	2.62	18636.97
340.03	2.63	3.867	1.6630	.00	2.63	18717.52
340.04	2.64	3.883	1.6639	.00	2.64	18798.12
340.05	2.64	3.900	1.6649	.00	2.64	18878.52
340.06	2.65	3.917	1.6659	.00	2.65	18959.21
340.07	2.66	3.933	1.6669	.00	2.66	19039.96
340.08	2.67	3.950	1.6679	.00	2.67	19120.50
340.09	2.68	3.967	1.6689	.00	2.68	19201.33

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
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INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.10	2.68	3.983	1.6699	.00	2.68	19282.22
340.11	2.69	4.000	1.6709	.00	2.69	19362.90
340.12	2.70	4.017	1.6718	.00	2.70	19443.88
340.13	2.71	4.034	1.6728	.00	2.71	19524.91
340.14	2.71	4.050	1.6738	.00	2.71	19605.99
340.15	2.72	4.067	1.6748	.00	2.72	19686.87
340.16	2.73	4.084	1.6758	.00	2.73	19768.04
340.17	2.74	4.101	1.6768	.00	2.74	19849.25
340.18	2.74	4.117	1.6778	.00	2.74	19930.28
340.19	2.75	4.134	1.6788	.00	2.75	20011.59
340.20	2.76	4.151	1.6798	.00	2.76	20092.96
340.21	2.77	4.168	1.6808	.00	2.77	20174.12
340.22	2.77	4.184	1.6817	.00	2.77	20255.58
340.23	2.78	4.201	1.6827	.00	2.78	20337.09
340.24	2.79	4.218	1.6837	.00	2.79	20418.39
340.25	2.80	4.235	1.6847	.00	2.80	20500.00
340.26	2.80	4.252	1.6857	.00	2.80	20581.65
340.27	2.81	4.269	1.6867	.00	2.81	20663.10
340.28	2.82	4.286	1.6877	.00	2.82	20744.85
340.29	2.83	4.302	1.6887	.00	2.83	20826.64

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.30	2.83	4.319	1.6897	.00	2.83	20908.24
340.31	2.84	4.336	1.6907	.00	2.84	20990.13
340.32	2.85	4.353	1.6917	.00	2.85	21072.07
340.33	2.86	4.370	1.6927	.00	2.86	21153.81
340.34	2.86	4.387	1.6937	.00	2.86	21235.85
340.35	2.87	4.404	1.6947	.00	2.87	21317.93
340.36	2.88	4.421	1.6957	.00	2.88	21399.82
340.37	2.89	4.438	1.6967	.00	2.89	21482.00
340.38	2.89	4.455	1.6977	.00	2.89	21564.23
340.39	2.90	4.472	1.6987	.00	2.90	21646.51
340.40	2.91	4.489	1.6996	.00	2.91	21728.58
340.41	2.91	4.506	1.7006	.00	2.91	21810.96
340.42	2.92	4.523	1.7016	.00	2.92	21893.38
340.43	2.93	4.540	1.7026	.00	2.93	21975.60
340.44	2.94	4.557	1.7036	.00	2.94	22058.12
340.45	2.94	4.574	1.7046	.00	2.94	22140.69
340.46	2.95	4.591	1.7056	.00	2.95	22223.05
340.47	2.96	4.608	1.7066	.00	2.96	22305.71
340.48	2.96	4.625	1.7076	.00	2.96	22388.43
340.49	2.97	4.642	1.7086	.00	2.97	22470.94

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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Outflow HYG file = NONE STORED - POND 1 OUT 2

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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.50	2.98	4.659	1.7096	.00	2.98	22553.75
340.51	2.99	4.676	1.7106	.00	2.99	22636.61
340.52	2.99	4.693	1.7116	.00	2.99	22719.26
340.53	3.00	4.711	1.7126	.00	3.00	22802.21
340.54	3.01	4.728	1.7136	.00	3.01	22885.22
340.55	3.01	4.745	1.7146	.00	3.01	22968.02
340.56	3.02	4.762	1.7156	.00	3.02	23051.12
340.57	3.03	4.779	1.7166	.00	3.03	23134.27
340.58	3.03	4.796	1.7176	.00	3.03	23217.21
340.59	3.04	4.814	1.7186	.00	3.04	23300.46
340.60	3.05	4.831	1.7197	.00	3.05	23383.75
340.61	3.05	4.848	1.7207	.00	3.05	23466.84
340.62	3.06	4.865	1.7217	.00	3.06	23550.23
340.63	3.07	4.882	1.7227	.00	3.07	23633.67
340.64	3.08	4.900	1.7237	.00	3.08	23717.17
340.65	3.08	4.917	1.7247	.00	3.08	23800.45
340.66	3.09	4.934	1.7257	.00	3.09	23884.04
340.67	3.10	4.951	1.7267	.00	3.10	23967.67
340.68	3.10	4.969	1.7277	.00	3.10	24051.10
340.69	3.11	4.986	1.7287	.00	3.11	24134.83

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.70	3.12	5.003	1.7297	.00	3.12	24218.61
340.71	3.12	5.020	1.7307	.00	3.12	24302.19
340.72	3.13	5.038	1.7317	.00	3.13	24386.06
340.73	3.14	5.055	1.7327	.00	3.14	24469.99
340.74	3.14	5.072	1.7337	.00	3.14	24553.71
340.75	3.15	5.090	1.7347	.00	3.15	24637.74
340.76	3.16	5.107	1.7357	.00	3.16	24721.81
340.77	3.16	5.124	1.7367	.00	3.16	24805.68
340.78	3.17	5.142	1.7377	.00	3.17	24889.85
340.79	3.18	5.159	1.7388	.00	3.18	24974.07
340.80	3.18	5.177	1.7398	.00	3.18	25058.08
340.81	3.19	5.194	1.7408	.00	3.19	25142.40
340.82	3.20	5.211	1.7418	.00	3.20	25226.77
340.83	3.20	5.229	1.7428	.00	3.20	25310.93
340.84	3.21	5.246	1.7438	.00	3.21	25395.39
340.85	3.22	5.264	1.7448	.00	3.22	25479.90
340.86	3.22	5.281	1.7458	.00	3.22	25564.21
340.87	3.23	5.299	1.7468	.00	3.23	25648.82
340.88	3.24	5.316	1.7478	.00	3.24	25733.48
340.89	3.24	5.334	1.7489	.00	3.24	25818.19

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.90	3.25	5.351	1.7499	.00	3.25	25902.69
340.91	3.25	5.369	1.7509	.00	3.25	25987.49
340.92	3.26	5.386	1.7519	.00	3.26	26072.35
340.93	3.27	5.404	1.7529	.00	3.27	26157.00
340.94	3.27	5.421	1.7539	.00	3.27	26241.95
340.95	3.28	5.439	1.7549	.00	3.28	26326.96
340.96	3.29	5.456	1.7559	.00	3.29	26411.75
340.97	3.29	5.474	1.7570	.00	3.29	26496.85
340.98	3.30	5.491	1.7580	.00	3.30	26582.00
340.99	3.31	5.509	1.7590	.00	3.31	26666.94
341.00	3.31	5.527	1.7600	.00	3.31	26752.19
341.01	3.32	5.544	1.7611	.00	3.32	26837.49
341.02	3.33	5.562	1.7622	.00	3.33	26922.58
341.03	3.33	5.579	1.7633	.00	3.33	27007.99
341.04	3.34	5.597	1.7643	.00	3.34	27093.45
341.05	3.34	5.615	1.7654	.00	3.34	27178.70
341.06	3.35	5.632	1.7665	.00	3.35	27264.26
341.07	3.36	5.650	1.7676	.00	3.36	27349.87
341.08	3.36	5.668	1.7687	.00	3.36	27435.28
341.09	3.37	5.685	1.7698	.00	3.37	27521.00

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.10	3.38	5.703	1.7709	.00	3.38	27606.77
341.11	3.38	5.721	1.7719	.00	3.38	27692.33
341.12	3.39	5.739	1.7730	.00	3.39	27778.21
341.13	3.39	5.756	1.7741	.00	3.39	27864.14
341.14	3.40	5.774	1.7752	.00	3.40	27950.13
341.15	3.41	5.792	1.7763	.00	3.41	28035.90
341.16	3.41	5.810	1.7774	.00	3.41	28121.99
341.17	3.42	5.827	1.7785	.00	3.42	28208.13
341.18	3.43	5.845	1.7796	.00	3.43	28294.06
341.19	3.43	5.863	1.7806	.00	3.43	28380.31
341.20	3.44	5.881	1.7817	.00	3.44	28466.61
341.21	3.44	5.899	1.7828	.00	3.44	28552.70
341.22	3.45	5.916	1.7839	.00	3.45	28639.10
341.23	3.46	5.934	1.7850	.00	3.46	28725.56
341.24	3.46	5.952	1.7861	.00	3.46	28811.81
341.25	3.47	5.970	1.7872	.00	3.47	28898.38
341.26	3.47	5.988	1.7883	.00	3.47	28984.99
341.27	3.48	6.006	1.7894	.00	3.48	29071.40
341.28	3.49	6.024	1.7905	.00	3.49	29158.12
341.29	3.49	6.042	1.7916	.00	3.49	29244.90

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.30	3.50	6.059	1.7926	.00	3.50	29331.46
341.31	3.50	6.077	1.7937	.00	3.50	29418.34
341.32	3.51	6.095	1.7948	.00	3.51	29505.27
341.33	3.52	6.113	1.7959	.00	3.52	29592.00
341.34	3.52	6.131	1.7970	.00	3.52	29679.04
341.35	3.53	6.149	1.7981	.00	3.53	29766.13
341.36	3.53	6.167	1.7992	.00	3.53	29853.01
341.37	3.54	6.185	1.8003	.00	3.54	29940.21
341.38	3.55	6.203	1.8014	.00	3.55	30027.46
341.39	3.55	6.221	1.8025	.00	3.55	30114.77
341.40	3.56	6.239	1.8036	.00	3.56	30201.87
341.41	3.56	6.257	1.8047	.00	3.56	30289.27
341.42	3.57	6.275	1.8058	.00	3.57	30376.74
341.43	3.58	6.293	1.8069	.00	3.58	30463.99
341.44	3.58	6.312	1.8080	.00	3.58	30551.56
341.45	3.59	6.330	1.8091	.00	3.59	30639.19
341.46	3.59	6.348	1.8102	.00	3.59	30726.60
341 47	3.60	6.366	1.8113	.00	3.60	30814.33
341.48	3.61	6.384	1.8124	.00	3.61	30902.11
341.49	3.61	6.402	1.8135	.00	3.61	30989.68

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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Pond Volume Data = POND 1
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.50	3.62	6.420	1.8146	.00	3.62	31077.57
341.51	3.67	6.438	1.8157	.00	3.67	31165.56
341.52	3.76	6.457	1.8168	.00	3.76	31253.38
341.53	3.87	6.475	1.8179	.00	3.87	31341.54
341.54	4.00	6.493	1.8190	.00	4.00	31429.77
341.55	4.15	6.511	1.8201	.00	4.15	31517.79
341.56	4.32	6.529	1.8212	.00	4.32	31606.17
341.57	4.49	6.548	1.8223	.00	4.49	31694.60
341.58	4.68	6.566	1.8234	.00	4.68	31782.83
341.59	4.87	6.584	1.8245	.00	4.87	31871.39
341.60	5.08	6.602	1.8256	.00	5.08	31960.02
341.61	5.30	6.620	1.8267	.00	5.30	32048.44
341.62	5.53	6.639	1.8278	.00	5.53	32137.19
341.63	5.77	6.657	1.8289	.00	5.77	32226.01
341.64	6.02	6.675	1.8300	.00	6.02	32314.89
341.65	6.28	6.694	1.8311	.00	6.28	32403.56
341.66	6.54	6.712	1.8322	.00	6.54	32492.57
341.67	6.82	6.730	1.8333	.00	6.82	32581.64
341.68	7.10	6.749	1.8344	.00	7.10	32670.50
341.69	7.40	6.767	1.8355	.00	7.40	32759.70

LEVEL POOL ROUTING DATA

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Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.70	7.70	6.785	1.8367	.00	7.70	32848.95
341.71	8.00	6.804	1.8378	.00	8.00	32937.99
341.72	8.32	6.822	1.8389	.00	8.32	33027.37
341.73	8.64	6.841	1.8400	.00	8.64	33116.80
341.74	8.97	6.859	1.8411	.00	8.97	33206.03
341.75	9.31	6.877	1.8422	.00	9.31	33295.59
341.76	9.65	6.896	1.8433	.00	9.65	33385.21
341.77	10.00	6.914	1.8444	.00	10.00	33474.62
341.78	10.36	6.933	1.8455	.00	10.36	33564.35
341.79	10.72	6.951	1.8466	.00	10.72	33654.16
341.80	11.09	6.970	1.8477	.00	11.09	33743.74
341.81	11.47	6.988	1.8488	.00	11.47	33833.66
341.82	11.85	7.007	1.8500	.00	11.85	33923.64
341.83	12.24	7.025	1.8511	.00	12.24	34013.41
341.84	12.63	7.044	1.8522	.00	12.63	34103.51
341.85	13.03	7.062	1.8533	.00	13.03	34193.67
341.86	13.44	7.081	1.8544	.00	13.44	34283.62
341.87	13.85	7.099	1.8555	.00	13.85	34373.90
341.88	14.27	7.118	1.8566	.00	14.27	34464.23
341.89	14.69	7.136	1.8577	.00	14.69	34554.63

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.90	15.12	7.155	1.8588	.00	15.12	34644.82
341.91	15.55	7.174	1.8600	.00	15.55	34735.33
341.92	15.99	7.192	1.8611	.00	15.99	34825.91
341.93	16.43	7.211	1.8622	.00	16.43	34916.27
341.94	16.88	7.229	1.8633	.00	16.88	35006.96
341.95	17.34	7.248	1.8644	.00	17.34	35097.72
341.96	17.80	7.267	1.8655	.00	17.80	35188.26
341.97	18.26	7.285	1.8667	.00	18.26	35279.13
341.98	18.73	7.304	1.8678	.00	18.73	35370.06
341.99	19.21	7.323	1.8689	.00	19.21	35460.77
342.00	19.69	7.341	1.8700	.00	19.69	35551.82
342.01	20.17	7.360	1.8711	.00	20.17	35642.93
342.02	20.66	7.379	1.8722	.00	20.66	35733.82
342.03	21.15	7.397	1.8733	.00	21.15	35825.04
342.04	21.65	7.416	1.8743	.00	21.65	35916.32
342.05	22.15	7.435	1.8754	.00	22.15	36007.38
342.06	22.66	7.454	1.8765	.00	22.66	36098.77
342.07	23.17	7.473	1.8776	.00	23.17	36190.22
342.08	23.69	7.491	1.8787	.00	23.69	36281.45
342.09	24.21	7.510	1.8798	.00	24.21	36373.02

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.10	24.73	7.529	1.8809	.00	24.73	36464.63
342.11	25.26	7.548	1.8819	.00	25.26	36556.03
342.12	25.80	7.567	1.8830	.00	25.80	36647.77
342.13	26.33	7.585	1.8841	.00	26.33	36739.56
342.14	26.87	7.604	1.8852	.00	26.87	36831.41
342.15	27.42	7.623	1.8863	.00	27.42	36923.03
342.16	27.97	7.642	1.8874	.00	27.97	37014.99
342.17	28.52	7.661	1.8885	.00	28.52	37107.02
342.18	29.08	7.680	1.8896	.00	29.08	37198.81
342.19	29.64	7.699	1.8907	.00	29.64	37290.94
342.20	30.20	7.718	1.8918	.00	30.20	37383.13
342.21	30.77	7.736	1.8928	.00	30.77	37475.09
342.22	31.34	7.755	1.8939	.00	31.34	37567.39
342.23	31.92	7.774	1.8950	.00	31.92	37659.75
342.24	32.49	7.793	1.8961	.00	32.49	37751.88
342.25	33.07	7.812	1.8972	.00	33.07	37844.35
342.26	33.66	7.831	1.8983	.00	33.66	37936.87
342.27	34.24	7.850	1.8994	.00	34.24	38029.17
342.28	34.83	7.869	1.9005	.00	34.83	38121.81
342.29	35.43	7.888	1.9016	.00	35.43	38214.50

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.30	36.02	7.907	1.9027	.00	36.02	38306.97
342.31	36.62	7.926	1.9038	.00	36.62	38399.77
342.32	37.22	7.945	1.9049	.00	37.22	38492.63
342.33	37.82	7.964	1.9060	.00	37.82	38585.26
342.34	38.42	7.983	1.9070	.00	38.42	38678.22
342.35	39.02	8.003	1.9081	.00	39.02	38771.24
342.36	40.58	8.022	1.9092	.00	40.58	38864.99
342.37	41.20	8.041	1.9103	.00	41.20	38958.13
342.38	41.82	8.060	1.9114	.00	41.82	39051.33
342.39	42.44	8.079	1.9125	.00	42.44	39144.58
342.40	43.06	8.098	1.9136	.00	43.06	39237.61
342.41	43.68	8.117	1.9147	.00	43.68	39330.96
342.42	44.30	8.136	1.9158	.00	44.30	39424.37
342.43	44.92	8.156	1.9169	.00	44.92	39517.55
342.44	45.53	8.175	1.9180	.00	45.53	39611.05
342.45	45.78	8.194	1.9191	.00	45.78	39704.25
342.46	45.97	8.213	1.9202	.00	45.97	39797.16
342.47	46.56	8.232	1.9213	.00	46.56	39890.82
342.48	47.63	8.252	1.9224	.00	47.63	39984.99
342.49	47.71	8.271	1.9235	.00	47.71	40077.95

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.50	47.78	8.290	1.9246	.00	47.78	40171.23
342.51	47.84	8.309	1.9257	.00	47.84	40264.56
342.52	47.91	8.328	1.9268	.00	47.91	40357.67
342.53	47.99	8.348	1.9279	.00	47.99	40451.12
342.54	48.06	8.367	1.9290	.00	48.06	40544.62
342.55	48.13	8.386	1.9301	.00	48.13	40637.89
342.56	48.19	8.406	1.9312	.00	48.19	40731.49
342.57	48.26	8.425	1.9323	.00	48.26	40825.14
342.58	48.34	8.444	1.9334	.00	48.34	40918.58
342.59	48.41	8.464	1.9345	.00	48.41	41012.34
342.60	48.48	8.483	1.9356	.00	48.48	41106.17
342.61	48.54	8.502	1.9367	.00	48.54	41199.74
342.62	48.61	8.522	1.9378	.00	48.61	41293.67
342.63	48.70	8.541	1.9389	.00	48.70	41387.67
342.64	48.78	8.561	1.9400	.00	48.78	41481.71
342.65	48.83	8.580	1.9411	.00	48.83	41575.49
342.66	48.91	8.599	1.9422	.00	48.91	41669.64
342.67	48.98	8.619	1.9434	.00	48.98	41763.84
342.68	49.06	8.638	1.9445	.00	49.06	41857.80
342.69	49.12	8.658	1.9456	.00	49.12	41952.09

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.70	49.19	8.677	1.9467	.00	49.19	42046.45
342.71	49.26	8.697	1.9478	.00	49.26	42140.57
342.72	49.33	8.716	1.9489	.00	49.33	42235.02
342.73	49.40	8.736	1.9500	.00	49.40	42329.54
342.74	49.47	8.755	1.9511	.00	49.47	42423.82
342.75	49.53	8.775	1.9522	.00	49.53	42518.43
342.76	49.60	8.794	1.9533	.00	49.60	42613.12
342.77	49.68	8.814	1.9544	.00	49.68	42707.56
342.78	49.74	8.833	1.9555	.00	49.74	42802.33
342.79	49.81	8.853	1.9566	.00	49.81	42897.18
342.80	49.88	8.872	1.9577	.00	49.88	42991.78
342.81	49.95	8.892	1.9589	.00	49.95	43086.71
342.82	50.02	8.912	1.9600	.00	50.02	43181.71
342.83	50.09	8.931	1.9611	.00	50.09	43276.48
342.84	50.15	8.951	1.9622	.00	50.15	43371.58
342.85	50.23	8.970	1.9633	.00	50.23	43466.75
342.86	50.29	8.990	1.9644	.00	50.29	43561.66
342.87	50.36	9.010	1.9655	.00	50.36	43656.92
342.88	50.43	9.029	1.9666	.00	50.43	43752.24
342.89	50.50	9.049	1.9677	.00	50.50	43847.61

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.90	50.57	9.069	1.9689	.00	50.57	43942.76
342.91	50.63	9.088	1.9700	.00	50.63	44038.23
342.92	50.71	9.108	1.9711	.00	50.71	44133.77
342.93	50.76	9.128	1.9722	.00	50.76	44229.06
342.94	50.84	9.147	1.9733	.00	50.84	44324.71
342.95	50.90	9.167	1.9744	.00	50.90	44420.40
342.96	50.98	9.187	1.9755	.00	50.98	44515.87
342.97	51.03	9.207	1.9767	.00	51.03	44611.66
342.98	51.11	9.227	1.9778	.00	51.11	44707.53
342.99	51.17	9.246	1.9789	.00	51.17	44803.14
343.00	51.24	9.266	1.9800	.00	51.24	44899.11
343.01	51.37	9.286	1.9811	.00	51.37	44995.19
343.02	51.56	9.306	1.9822	.00	51.56	45091.09
343.03	51.77	9.326	1.9833	.00	51.77	45187.36
343.04	52.01	9.345	1.9843	.00	52.01	45283.71
343.05	52.26	9.365	1.9854	.00	52.26	45379.83
343.06	52.55	9.385	1.9865	.00	52.55	45476.33
343.07	52.84	9.405	1.9876	.00	52.84	45572.89
343.08	53.16	9.425	1.9887	.00	53.16	45669.24
343.09	53.48	9.445	1.9898	.00	53.48	45765.93

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.10	53.83	9.465	1.9909	.00	53.83	45862.71
343.11	54.19	9.485	1.9920	.00	54.19	45959.25
343.12	54.55	9.504	1.9930	.00	54.55	46056.14
343.13	54.94	9.524	1.9941	.00	54.94	46153.12
343.14	55.34	9.544	1.9952	.00	55.34	46250.15
343.15	55.75	9.564	1.9963	.00	55.75	46346.96
343.16	56.17	9.584	1.9974	.00	56.17	46444.11
343.17	56.60	9.604	1.9985	.00	56.60	46541.34
343.18	57.04	9.624	1.9996	.00	57.04	46638.34
343.19	57.50	9.644	2.0007	.00	57.50	46735.69
343.20	57.96	9.664	2.0018	.00	57.96	46833.11
343.21	58.43	9.684	2.0029	.00	58.43	46930.29
343.22	58.92	9.704	2.0039	.00	58.92	47027.84
343.23	59.41	9.724	2.0050	.00	59.41	47125.44
343.24	59.90	9.744	2.0061	.00	59.90	47222.80
343.25	60.41	9.764	2.0072	.00	60.41	47320.52
343.26	60.94	9.785	2.0083	.00	60.94	47418.33
343.27	61.46	9.805	2.0094	.00	61.46	47515.88
343.28	62.00	9.825	2.0105	.00	62.00	47613.79
343.29	62.54	9.845	2.0116	.00	62.54	47711.77

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
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Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.30	63.09	9.865	2.0127	.00	63.09	47809.50
343.31	63.66	9.885	2.0138	.00	63.66	47907.61
343.32	64.22	9.905	2.0149	.00	64.22	48005.76
343.33	64.80	9.925	2.0160	.00	64.80	48103.67
343.34	65.38	9.946	2.0171	.00	65.38	48201.95
343.35	65.97	9.966	2.0182	.00	65.97	48300.29
343.36	66.57	9.986	2.0193	.00	66.57	48398.40
343.37	67.18	10.006	2.0204	.00	67.18	48496.86
343.38	67.79	10.026	2.0215	.00	67.79	48595.38
343.39	68.42	10.047	2.0225	.00	68.42	48693.96
343.40	69.04	10.067	2.0236	.00	69.04	48792.30
343.41	69.67	10.087	2.0247	.00	69.67	48891.00
343.42	70.31	10.107	2.0258	.00	70.31	48989.75
343.43	70.95	10.128	2.0269	.00	70.95	49088.27
343.44	71.61	10.148	2.0280	.00	71.61	49187.16
343.45	72.28	10.168	2.0291	.00	72.28	49286.11
343.46	72.94	10.188	2.0302	.00	72.94	49384.80
343.47	73.61	10.209	2.0313	.00	73.61	49483.86
343.48	74.30	10.229	2.0324	.00	74.30	49582.98
343.49	74.98	10.249	2.0335	.00	74.98	49681.85

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.50	75.68	10.270	2.0346	.00	75.68	49781.09
343.51	76.37	10.290	2.0357	.00	76.37	49880.39
343.52	77.08	10.310	2.0368	.00	77.08	49979.45
343.53	77.78	10.331	2.0379	.00	77.78	50078.86
343.54	78.50	10.351	2.0390	.00	78.50	50178.34
343.55	79.22	10.372	2.0401	.00	79.22	50277.56
343.56	79.95	10.392	2.0412	.00	79.95	50377.16
343.57	80.69	10.412	2.0423	.00	80.69	50476.82
343.58	81.43	10.433	2.0434	.00	81.43	50576.23
343.59	82.18	10.453	2.0445	.00	82.18	50676.00
343.60	82.93	10.474	2.0456	.00	82.93	50775.83
343.61	83.68	10.494	2.0467	.00	83.68	50875.41
343.62	84.43	10.515	2.0478	.00	84.43	50975.36
343.63	85.24	10.535	2.0490	.00	85.24	51075.40
343.64	86.01	10.556	2.0501	.00	86.01	51175.46
343.65	86.79	10.576	2.0512	.00	86.79	51275.29
343.66	87.57	10.597	2.0523	.00	87.57	51375.47
343.67	88.37	10.617	2.0534	.00	88.37	51475.71
343.68	89.16	10.638	2.0545	.00	89.16	51575.71
343.69	89.96	10.658	2.0556	.00	89.96	51676.07

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.70	90.76	10.679	2.0567	.00	90.76	51776.49
343.71	91.58	10.699	2.0578	.00	91.58	51876.67
343.72	92.40	10.720	2.0589	.00	92.40	51977.21
343.73	93.21	10.741	2.0600	.00	93.21	52077.80
343.74	94.04	10.761	2.0611	.00	94.04	52178.16
343.75	94.87	10.782	2.0622	.00	94.87	52278.87
343.76	95.71	10.802	2.0633	.00	95.71	52379.64
343.77	96.54	10.823	2.0644	.00	96.54	52480.16
343.78	97.40	10.844	2.0655	.00	97.40	52581.05
343.79	98.25	10.864	2.0667	.00	98.25	52682.00
343.80	99.10	10.885	2.0678	.00	99.10	52782.70
343.81	99.96	10.906	2.0689	.00	99.96	52883.77
343.82	100.82	10.926	2.0700	.00	100.82	52984.89
343.83	101.69	10.947	2.0711	.00	101.69	53085.76
343.84	102.57	10.968	2.0722	.00	102.57	53187.00
343.85	103.45	10.989	2.0733	.00	103.45	53288.30
343.86	104.33	11.009	2.0744	.00	104.33	53389.36
343.87	105.22	11.030	2.0755	.00	105.22	53490.77
343.88	106.11	11.051	2.0766	.00	106.11	53592.23
343.89	107.01	11.072	2.0778	.00	107.01	53693.77

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.90	107.91	11.092	2.0789	.00	107.91	53795.05
343.91	108.81	11.113	2.0800	.00	108.81	53896.70
343.92	109.72	11.134	2.0811	.00	109.72	53998.41
343.93	110.64	11.155	2.0822	.00	110.64	54099.86
343.94	111.55	11.176	2.0833	.00	111.55	54201.68
343.95	112.48	11.197	2.0844	.00	112.48	54303.57
343.96	113.40	11.217	2.0855	.00	113.40	54405.20
343.97	114.34	11.238	2.0867	.00	114.34	54507.20
343.98	115.28	11.259	2.0878	.00	115.28	54609.26
343.99	116.22	11.280	2.0889	.00	116.22	54711.06
344.00	117.17	11.301	2.0900	.00	117.17	54813.24
344.01	118.11	11.322	2.0911	.00	118.11	54915.46
344.02	119.07	11.343	2.0922	.00	119.07	55017.44
344.03	120.03	11.364	2.0933	.00	120.03	55119.79
344.04	120.99	11.385	2.0943	.00	120.99	55222.19
344.05	121.95	11.405	2.0954	.00	121.95	55324.33
344.06	122.92	11.426	2.0965	.00	122.92	55426.85
344.07	123.90	11.447	2.0976	.00	123.90	55529.43
344.08	124.88	11.468	2.0987	.00	124.88	55631.75
344.09	125.86	11.489	2.0998	.00	125.86	55734.43

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.10	126.85	11.510	2.1009	.00	126.85	55837.18
344.11	127.83	11.531	2.1020	.00	127.83	55939.65
344.12	128.83	11.552	2.1031	.00	128.83	56042.51
344.13	129.83	11.573	2.1041	.00	129.83	56145.42
344.14	130.83	11.595	2.1052	.00	130.83	56248.39
344.15	131.83	11.616	2.1063	.00	131.83	56351.11
344.16	132.84	11.637	2.1074	.00	132.84	56454.18
344.17	133.86	11.658	2.1085	.00	133.86	56557.32
344.18	134.88	11.679	2.1096	.00	134.88	56660.21
344.19	135.90	11.700	2.1107	.00	135.90	56763.46
344.20	136.93	11.721	2.1118	.00	136.93	56866.77
344.21	137.96	11.742	2.1129	.00	137.96	56969.83
344.22	138.99	11.763	2.1140	.00	138.99	57073.25
344.23	140.03	11.784	2.1151	.00	140.03	57176.73
344.24	141.06	11.806	2.1161	.00	141.06	57279.94
344.25	142.10	11.827	2.1172	.00	142.10	57383.54
344.26	143.15	11.848	2.1183	.00	143.15	57487.19
344.27	144.21	11.869	2.1194	.00	144.21	57590.58
344.28	145.27	11.890	2.1205	.00	145.27	57694.35
344.29	146.32	11.912	2.1216	.00	146.32	57798.16

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.30	147.38	11.933	2.1227	.00	147.38	57901.72
344.31	148.46	11.954	2.1238	.00	148.46	58005.66
344.32	149.52	11.975	2.1249	.00	149.52	58109.64
344.33	150.60	11.996	2.1260	.00	150.60	58213.38
344.34	151.68	12.018	2.1271	.00	151.68	58317.48
344.35	152.76	12.039	2.1282	.00	152.76	58421.64
344.36	153.83	12.060	2.1293	.00	153.83	58525.53
344.37	154.93	12.082	2.1304	.00	154.93	58629.81
344.38	156.02	12.103	2.1315	.00	156.02	58734.14
344.39	157.12	12.124	2.1326	.00	157.12	58838.53
344.40	158.21	12.146	2.1337	.00	158.21	58942.65
344.41	159.32	12.167	2.1348	.00	159.32	59047.15
344.42	160.42	12.188	2.1359	.00	160.42	59151.70
344.43	161.53	12.210	2.1370	.00	161.53	59256.00
344.44	162.65	12.231	2.1381	.00	162.65	59360.68
344.45	163.76	12.252	2.1392	.00	163.76	59465.40
344.46	164.88	12.274	2.1402	.00	164.88	59569.86
344.47	166.00	12.295	2.1413	.00	166.00	59674.71
344.48	167.13	12.317	2.1424	.00	167.13	59779.61
344.49	168.25	12.338	2.1435	.00	168.25	59884.23

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.50	169.39	12.359	2.1446	.00	169.39	59989.23
344.51	170.53	12.381	2.1457	.00	170.53	60094.30
344.52	171.67	12.402	2.1468	.00	171.67	60199.11
344.53	172.80	12.424	2.1479	.00	172.80	60304.28
344.54	173.95	12.445	2.1491	.00	173.95	60409.52
344.55	175.10	12.467	2.1502	.00	175.10	60514.50
344.56	176.25	12.488	2.1513	.00	176.25	60619.84
344.57	177.42	12.510	2.1524	.00	177.42	60725.25
344.58	178.57	12.531	2.1535	.00	178.57	60830.39
344.59	179.73	12.553	2.1546	.00	179.73	60935.91
344.60	180.90	12.575	2.1557	.00	180.90	61041.49
344.61	182.06	12.596	2.1568	.00	182.06	61146.79
344.62	183.24	12.618	2.1579	.00	183.24	61252.49
344.63	184.41	12.639	2.1590	.00	184.41	61358.23
344.64	185.60	12.661	2.1601	.00	185.60	61464.04
344.65	186.77	12.682	2.1612	.00	186.77	61569.57
344.66	187.96	12.704	2.1623	.00	187.96	61675.49
344.67	189.14	12.726	2.1634	.00	189.14	61781.45
344.68	190.34	12.747	2.1645	.00	190.34	61887.17
344.69	191.53	12.769	2.1656	.00	191.53	61993.25

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.70	192.73	12.791	2.1667	.00	192.73	62099.40
344.71	193.92	12.812	2.1678	.00	193.92	62205.27
344.72	195.13	12.834	2.1689	.00	195.13	62311.52
344.73	196.33	12.856	2.1700	.00	196.33	62417.84
344.74	197.54	12.877	2.1711	.00	197.54	62523.88
344.75	198.75	12.899	2.1722	.00	198.75	62630.30
344.76	199.97	12.921	2.1733	.00	199.97	62736.79
344.77	201.30	12.943	2.1744	.00	201.30	62843.11
344.78	202.52	12.964	2.1756	.00	202.52	62949.71
344.79	203.74	12.986	2.1767	.00	203.74	63056.36
344.80	204.97	13.008	2.1778	.00	204.97	63162.75
344.81	206.20	13.030	2.1789	.00	206.20	63269.51
344.82	207.44	13.051	2.1800	.00	207.44	63376.33
344.83	208.68	13.073	2.1811	.00	208.68	63482.89
344.84	209.92	13.095	2.1822	.00	209.92	63589.82
344.85	211.16	13.117	2.1833	.00	211.16	63696.81
344.86	212.41	13.139	2.1844	.00	212.41	63803.54
344.87	213.66	13.161	2.1855	.00	213.66	63910.66
344.88	214.92	13.182	2.1867	.00	214.92	64017.82
344.89	216.17	13.204	2.1878	.00	216.17	64125.04

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 IN 2
Outflow HYG file = NONE STORED - POND 1 OUT 2

Pond Node Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 337.50 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.90	217.43	13.226	2.1889	.00	217.43	64231.99
344.91	218.68	13.248	2.1900	.00	218.68	64339.32
344.92	219.94	13.270	2.1911	.00	219.94	64446.70
344.93	221.21	13.292	2.1922	.00	221.21	64553.82
344.94	222.48	13.314	2.1933	.00	222.48	64661.33
344.95	223.75	13.336	2.1944	.00	223.75	64768.89
344.96	225.03	13.358	2.1955	.00	225.03	64876.18
344.97	226.31	13.380	2.1967	.00	226.31	64983.86
344.98	227.60	13.402	2.1978	.00	227.60	65091.60
344.99	228.87	13.424	2.1989	.00	228.87	65199.05
345.00	230.17	13.446	2.2000	.00	230.17	65306.89

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1       IN  2
Outflow HYG file = NONE STORED - POND 1       OUT 2

Pond Node   Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   337.50 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    115.43 cfs    at   11.9450 hrs
Peak Outflow      =      2.95 cfs    at   15.3350 hrs
-----------------------------------------------------
Peak Elevation    =    340.45 ft
Peak Storage =          4.581 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =       6.739
- Infiltration =        .000
- HYG Vol OUT  =       2.961
- Retained Vol =       3.778
                 ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1       IN  10
Outflow HYG file = NONE STORED - POND 1       OUT 10

Pond Node   Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   337.50 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    194.21 cfs    at   11.9450 hrs
Peak Outflow      =     13.46 cfs    at   12.7650 hrs
-----------------------------------------------------
Peak Elevation    =    341.86 ft
Peak Storage =          7.082 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =      11.528
- Infiltration =        .000
- HYG Vol OUT  =       5.736
- Retained Vol =       5.792
                  ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1       IN  25
Outflow HYG file = NONE STORED - POND 1       OUT 25

Pond Node   Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   337.50 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    236.83 cfs    at   11.9450 hrs
Peak Outflow      =     37.24 cfs    at   12.2500 hrs
-----------------------------------------------------
Peak Elevation    =    342.32 ft
Peak Storage =          7.946 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =      14.186
- Infiltration =        .000
- HYG Vol OUT  =       8.178
- Retained Vol =       6.008
                  ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1       IN  100
Outflow HYG file = NONE STORED - POND 1       OUT 100

Pond Node   Data = POND 1
Pond Volume Data = POND 1
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   337.50 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    304.82 cfs    at   11.9400 hrs
Peak Outflow      =     64.97 cfs    at   12.1750 hrs
-----------------------------------------------------
Peak Elevation    =    343.33 ft
Peak Storage =          9.931 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =      18.494
- Infiltration =        .000
- HYG Vol OUT  =      12.237
- Retained Vol =       6.257
                  ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
337.50	.00	.000	1.4000	.00	.00	.00
337.51	.00	.014	1.4010	.00	.00	67.85
337.52	.00	.028	1.4020	.00	.00	135.54
337.53	.00	.042	1.4030	.00	.00	203.49
337.54	.00	.056	1.4040	.00	.00	271.48
337.55	.00	.070	1.4050	.00	.00	339.32
337.56	.00	.084	1.4060	.00	.00	407.41
337.57	.00	.098	1.4069	.00	.00	475.55
337.58	.00	.112	1.4079	.00	.00	543.53
337.59	.00	.126	1.4089	.00	.00	611.76
337.60	.00	.141	1.4099	.00	.00	680.04
337.61	.00	.155	1.4109	.00	.00	748.17
337.62	.00	.169	1.4119	.00	.00	816.55
337.63	.00	.183	1.4129	.00	.00	884.97
337.64	.00	.197	1.4139	.00	.00	953.45
337.65	.00	.211	1.4149	.00	.00	1021.77
337.66	.00	.225	1.4159	.00	.00	1090.34
337.67	.00	.239	1.4169	.00	.00	1158.96
337.68	.00	.254	1.4179	.00	.00	1227.42
337.69	.00	.268	1.4189	.00	.00	1296.14

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
337.70	.00	.282	1.4199	.00	.00	1364.90
337.71	.00	.296	1.4209	.00	.00	1433.51
337.72	.00	.310	1.4219	.00	.00	1502.37
337.73	.00	.325	1.4229	.00	.00	1571.28
337.74	.00	.339	1.4239	.00	.00	1640.03
337.75	.00	.353	1.4249	.00	.00	1709.04
337.76	.00	.367	1.4259	.00	.00	1778.09
337.77	.00	.382	1.4269	.00	.00	1846.99
337.78	.00	.396	1.4279	.00	.00	1916.14
337.79	.00	.410	1.4289	.00	.00	1985.34
337.80	.00	.424	1.4299	.00	.00	2054.38
337.81	.00	.439	1.4309	.00	.00	2123.68
337.82	.00	.453	1.4319	.00	.00	2193.03
337.83	.00	.467	1.4329	.00	.00	2262.21
337.84	.00	.482	1.4339	.00	.00	2331.66
337.85	.00	.496	1.4349	.00	.00	2401.15
337.86	.00	.510	1.4359	.00	.00	2470.48
337.87	.00	.525	1.4369	.00	.00	2540.07
337.88	.00	.539	1.4379	.00	.00	2609.71
337.89	.00	.554	1.4389	.00	.00	2679.40

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
337.90	.00	.568	1.4399	.00	.00	2748.92
337.91	.00	.582	1.4409	.00	.00	2818.70
337.92	.00	.597	1.4419	.00	.00	2888.54
337.93	.00	.611	1.4429	.00	.00	2958.21
337.94	.00	.626	1.4440	.00	.00	3028.14
337.95	.00	.640	1.4450	.00	.00	3098.12
337.96	.00	.655	1.4460	.00	.00	3167.93
337.97	.00	.669	1.4470	.00	.00	3238.01
337.98	.00	.683	1.4480	.00	.00	3308.14
337.99	.00	.698	1.4490	.00	.00	3378.10
338.00	.00	.712	1.4500	.00	.00	3448.32
338.01	.00	.727	1.4510	.00	.00	3518.60
338.02	.00	.741	1.4520	.00	.00	3588.70
338.03	.00	.756	1.4530	.00	.00	3659.07
338.04	.00	.771	1.4539	.00	.00	3729.48
338.05	.00	.785	1.4549	.00	.00	3799.73
338.06	.00	.800	1.4559	.00	.00	3870.24
338.07	.00	.814	1.4569	.00	.00	3940.80
338.08	.00	.829	1.4579	.00	.00	4011.19
338.09	.00	.843	1.4589	.00	.00	4081.85

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

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Starting Outflow = .00 cfs
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.10	.00	.858	1.4599	.00	.00	4152.55
338.11	.00	.873	1.4608	.00	.00	4223.08
338.12	.00	.887	1.4618	.00	.00	4293.88
338.13	.00	.902	1.4628	.00	.00	4364.73
338.14	.00	.916	1.4638	.00	.00	4435.62
338.15	.00	.931	1.4648	.00	.00	4506.34
338.16	.00	.946	1.4658	.00	.00	4577.33
338.17	.00	.960	1.4668	.00	.00	4648.37
338.18	.00	.975	1.4678	.00	.00	4719.24
338.19	.00	.990	1.4687	.00	.00	4790.37
338.20	.00	1.004	1.4697	.00	.00	4861.55
338.21	.00	1.019	1.4707	.00	.00	4932.56
338.22	.00	1.034	1.4717	.00	.00	5003.84
338.23	.00	1.049	1.4727	.00	.00	5075.16
338.24	.00	1.063	1.4737	.00	.00	5146.32
338.25	.00	1.078	1.4747	.00	.00	5217.74
338.26	.00	1.093	1.4757	.00	.00	5289.21
338.27	.00	1.108	1.4767	.00	.00	5360.51
338.28	.00	1.122	1.4777	.00	.00	5432.07
338.29	.00	1.137	1.4787	.00	.00	5503.68

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
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Pond Volume Data = POND 1 W/ 1FT
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.30	.00	1.152	1.4796	.00	.00	5575.13
338.31	.00	1.167	1.4806	.00	.00	5646.83
338.32	.00	1.182	1.4816	.00	.00	5718.59
338.33	.00	1.196	1.4826	.00	.00	5790.18
338.34	.00	1.211	1.4836	.00	.00	5862.03
338.35	.00	1.226	1.4846	.00	.00	5933.93
338.36	.00	1.241	1.4856	.00	.00	6005.66
338.37	.00	1.256	1.4866	.00	.00	6077.66
338.38	.00	1.271	1.4876	.00	.00	6149.71
338.39	.00	1.285	1.4886	.00	.00	6221.80
338.40	.00	1.300	1.4896	.00	.00	6293.73
338.41	.00	1.315	1.4906	.00	.00	6365.92
338.42	.00	1.330	1.4916	.00	.00	6438.16
338.43	.00	1.345	1.4926	.00	.00	6510.22
338.44	.00	1.360	1.4936	.00	.00	6582.56
338.45	.00	1.375	1.4946	.00	.00	6654.95
338.46	.00	1.390	1.4956	.00	.00	6727.16
338.47	.00	1.405	1.4966	.00	.00	6799.64
338.48	.00	1.420	1.4976	.00	.00	6872.17
338.49	.00	1.435	1.4986	.00	.00	6944.52

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
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No Infiltration

INITIAL CONDITIONS

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Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.50	.00	1.450	1.4996	.00	.00	7017.15
338.51	.00	1.465	1.5006	.00	.00	7089.83
338.52	.00	1.480	1.5016	.00	.00	7162.33
338.53	.01	1.495	1.5026	.00	.01	7235.11
338.54	.01	1.510	1.5036	.00	.01	7307.93
338.55	.03	1.525	1.5046	.00	.03	7380.60
338.56	.05	1.540	1.5056	.00	.05	7453.53
338.57	.06	1.555	1.5066	.00	.06	7526.51
338.58	.08	1.570	1.5076	.00	.08	7599.32
338.59	.10	1.585	1.5086	.00	.10	7672.40
338.60	.12	1.600	1.5096	.00	.12	7745.54
338.61	.14	1.615	1.5106	.00	.14	7818.50
338.62	.17	1.630	1.5116	.00	.17	7891.73
338.63	.19	1.646	1.5126	.00	.19	7965.02
338.64	.22	1.661	1.5136	.00	.22	8038.35
338.65	.25	1.676	1.5146	.00	.25	8111.50
338.66	.27	1.691	1.5156	.00	.27	8184.94
338.67	.31	1.706	1.5166	.00	.31	8258.42
338.68	.33	1.721	1.5176	.00	.33	8331.72
338.69	.34	1.737	1.5186	.00	.34	8405.28

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
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Starting Outflow = .00 cfs
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Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.70	.36	1.752	1.5197	.00	.36	8478.90
338.71	.37	1.767	1.5207	.00	.37	8552.34
338.72	.39	1.782	1.5217	.00	.39	8626.05
338.73	.40	1.797	1.5227	.00	.40	8699.81
338.74	.42	1.813	1.5237	.00	.42	8773.39
338.75	.43	1.828	1.5247	.00	.43	8847.24
338.76	.44	1.843	1.5257	.00	.44	8921.15
338.77	.45	1.858	1.5267	.00	.45	8994.87
338.78	.47	1.874	1.5277	.00	.47	9068.88
338.79	.48	1.889	1.5287	.00	.48	9142.92
338.80	.49	1.904	1.5297	.00	.49	9216.80
338.81	.50	1.920	1.5307	.00	.50	9290.94
338.82	.51	1.935	1.5318	.00	.51	9365.14
338.83	.52	1.950	1.5328	.00	.52	9439.16
338.84	.53	1.965	1.5338	.00	.53	9513.45
338.85	.54	1.981	1.5348	.00	.54	9587.79
338.86	.55	1.996	1.5358	.00	.55	9661.96
338.87	.56	2.012	1.5368	.00	.56	9736.40
338.88	.57	2.027	1.5378	.00	.57	9810.89
338.89	.58	2.042	1.5388	.00	.58	9885.42

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

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INITIAL CONDITIONS

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Starting Outflow = .00 cfs
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.90	.59	2.058	1.5398	.00	.59	9959.78
338.91	.60	2.073	1.5409	.00	.60	10034.42
338.92	.61	2.089	1.5419	.00	.61	10109.10
338.93	.62	2.104	1.5429	.00	.62	10183.61
338.94	.63	2.119	1.5439	.00	.63	10258.39
338.95	.64	2.135	1.5449	.00	.64	10333.22
338.96	.64	2.150	1.5459	.00	.64	10407.87
338.97	.65	2.166	1.5470	.00	.65	10482.80
338.98	.66	2.181	1.5480	.00	.66	10557.78
338.99	.67	2.197	1.5490	.00	.67	10632.58
339.00	.68	2.212	1.5500	.00	.68	10707.66
339.01	.69	2.228	1.5511	.00	.69	10782.79
339.02	.69	2.243	1.5522	.00	.69	10857.74
339.03	.70	2.259	1.5532	.00	.70	10932.97
339.04	.71	2.274	1.5543	.00	.71	11008.25
339.05	.72	2.290	1.5554	.00	.72	11083.36
339.06	.73	2.305	1.5565	.00	.73	11158.75
339.07	.73	2.321	1.5576	.00	.73	11234.19
339.08	.74	2.337	1.5587	.00	.74	11309.45
339.09	.75	2.352	1.5597	.00	.75	11385.00

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.10	.76	2.368	1.5608	.00	.76	11460.60
339.11	.76	2.383	1.5619	.00	.76	11536.02
339.12	.77	2.399	1.5630	.00	.77	11611.72
339.13	.78	2.415	1.5641	.00	.78	11687.48
339.14	.78	2.430	1.5652	.00	.78	11763.29
339.15	.79	2.446	1.5663	.00	.79	11838.92
339.16	.80	2.462	1.5673	.00	.80	11914.83
339.17	.80	2.477	1.5684	.00	.80	11990.80
339.18	.81	2.493	1.5695	.00	.81	12066.59
339.19	.82	2.509	1.5706	.00	.82	12142.66
339.20	.82	2.524	1.5717	.00	.82	12218.79
339.21	.83	2.540	1.5728	.00	.83	12294.73
339.22	.84	2.556	1.5739	.00	.84	12370.96
339.23	.84	2.572	1.5750	.00	.84	12447.24
339.24	.85	2.587	1.5761	.00	.85	12523.35
339.25	.86	2.603	1.5771	.00	.86	12599.74
339.26	.86	2.619	1.5782	.00	.86	12676.18
339.27	.87	2.635	1.5793	.00	.87	12752.44
339.28	.88	2.650	1.5804	.00	.88	12828.99
339.29	.88	2.666	1.5815	.00	.88	12905.58

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

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INITIAL CONDITIONS

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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.30	.89	2.682	1.5826	.00	.89	12982.00
339.31	.90	2.698	1.5837	.00	.90	13058.71
339.32	.90	2.714	1.5848	.00	.90	13135.47
339.33	.91	2.730	1.5859	.00	.91	13212.04
339.34	.91	2.745	1.5870	.00	.91	13288.91
339.35	.92	2.761	1.5881	.00	.92	13365.83
339.36	.93	2.777	1.5892	.00	.93	13442.56
339.37	.93	2.793	1.5903	.00	.93	13519.58
339.38	.94	2.809	1.5914	.00	.94	13596.66
339.39	.94	2.825	1.5925	.00	.94	13673.79
339.40	.95	2.841	1.5935	.00	.95	13750.74
339.41	.96	2.857	1.5946	.00	.96	13827.97
339.42	.96	2.873	1.5957	.00	.96	13905.26
339.43	.97	2.889	1.5968	.00	.97	13982.37
339.44	.97	2.905	1.5979	.00	.97	14059.76
339.45	.98	2.921	1.5990	.00	.98	14137.21
339.46	.98	2.937	1.6001	.00	.98	14214.47
339.47	.99	2.953	1.6012	.00	.99	14292.03
339.48	.99	2.969	1.6023	.00	.99	14369.64
339.49	1.00	2.985	1.6034	.00	1.00	14447.06

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
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Pond Node Data = POND 1 W/ 1FT
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No Infiltration

INITIAL CONDITIONS

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Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.50	1.01	3.001	1.6045	.00	1.01	14524.77
339.51	1.01	3.017	1.6056	.00	1.01	14602.54
339.52	1.02	3.033	1.6067	.00	1.02	14680.13
339.53	1.02	3.049	1.6078	.00	1.02	14758.00
339.54	1.03	3.065	1.6089	.00	1.03	14835.92
339.55	1.03	3.081	1.6100	.00	1.03	14913.67
339.56	1.04	3.097	1.6111	.00	1.04	14991.70
339.57	1.04	3.113	1.6122	.00	1.04	15069.79
339.58	1.05	3.129	1.6133	.00	1.05	15147.69
339.59	1.05	3.146	1.6144	.00	1.05	15225.88
339.60	1.06	3.162	1.6155	.00	1.06	15304.13
339.61	1.06	3.178	1.6167	.00	1.06	15382.19
339.62	1.07	3.194	1.6178	.00	1.07	15460.55
339.63	1.07	3.210	1.6189	.00	1.07	15538.96
339.64	1.08	3.227	1.6200	.00	1.08	15617.42
339.65	1.08	3.243	1.6211	.00	1.08	15695.69
339.66	1.09	3.259	1.6222	.00	1.09	15774.26
339.67	1.10	3.275	1.6233	.00	1.10	15852.88
339.68	1.10	3.291	1.6244	.00	1.10	15931.32
339.69	1.11	3.308	1.6255	.00	1.11	16010.05

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.70	1.11	3.324	1.6266	.00	1.11	16088.83
339.71	1.11	3.340	1.6277	.00	1.11	16167.43
339.72	1.12	3.356	1.6288	.00	1.12	16246.32
339.73	1.12	3.373	1.6299	.00	1.12	16325.26
339.74	1.13	3.389	1.6310	.00	1.13	16404.02
339.75	1.13	3.405	1.6321	.00	1.13	16483.07
339.76	1.14	3.422	1.6333	.00	1.14	16562.17
339.77	1.14	3.438	1.6344	.00	1.14	16641.09
339.78	1.15	3.454	1.6355	.00	1.15	16720.30
339.79	1.15	3.471	1.6366	.00	1.15	16799.57
339.80	1.16	3.487	1.6377	.00	1.16	16878.64
339.81	1.16	3.503	1.6388	.00	1.16	16958.02
339.82	1.17	3.520	1.6399	.00	1.17	17037.45
339.83	1.17	3.536	1.6410	.00	1.17	17116.69
339.84	1.18	3.553	1.6421	.00	1.18	17196.22
339.85	1.18	3.569	1.6433	.00	1.18	17275.81
339.86	1.19	3.586	1.6444	.00	1.19	17355.21
339.87	1.19	3.602	1.6455	.00	1.19	17434.91
339.88	1.20	3.618	1.6466	.00	1.20	17514.66
339.89	1.20	3.635	1.6477	.00	1.20	17594.46

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.90	1.21	3.651	1.6488	.00	1.21	17674.08
339.91	1.21	3.668	1.6499	.00	1.21	17753.99
339.92	1.21	3.684	1.6511	.00	1.21	17833.96
339.93	1.22	3.701	1.6522	.00	1.22	17913.74
339.94	1.22	3.717	1.6533	.00	1.22	17993.81
339.95	1.23	3.734	1.6544	.00	1.23	18073.94
339.96	1.23	3.751	1.6555	.00	1.23	18153.88
339.97	1.24	3.767	1.6566	.00	1.24	18234.12
339.98	1.24	3.784	1.6578	.00	1.24	18314.41
339.99	1.25	3.800	1.6589	.00	1.25	18394.51
340.00	1.25	3.817	1.6600	.00	1.25	18474.91
340.01	1.25	3.833	1.6610	.00	1.25	18555.36
340.02	1.26	3.850	1.6620	.00	1.26	18635.61
340.03	1.26	3.867	1.6630	.00	1.26	18716.16
340.04	1.27	3.883	1.6639	.00	1.27	18796.75
340.05	1.27	3.900	1.6649	.00	1.27	18877.15
340.06	1.28	3.917	1.6659	.00	1.28	18957.84
340.07	1.28	3.933	1.6669	.00	1.28	19038.58
340.08	1.29	3.950	1.6679	.00	1.29	19119.12
340.09	1.29	3.967	1.6689	.00	1.29	19199.95

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.10	1.29	3.983	1.6699	.00	1.29	19280.83
340.11	1.30	4.000	1.6709	.00	1.30	19361.51
340.12	1.30	4.017	1.6718	.00	1.30	19442.49
340.13	1.31	4.034	1.6728	.00	1.31	19523.51
340.14	1.31	4.050	1.6738	.00	1.31	19604.58
340.15	1.31	4.067	1.6748	.00	1.31	19685.46
340.16	1.32	4.084	1.6758	.00	1.32	19766.63
340.17	1.32	4.101	1.6768	.00	1.32	19847.84
340.18	1.33	4.117	1.6778	.00	1.33	19928.86
340.19	1.33	4.134	1.6788	.00	1.33	20010.17
340.20	1.34	4.151	1.6798	.00	1.34	20091.53
340.21	1.34	4.168	1.6808	.00	1.34	20172.69
340.22	1.34	4.184	1.6817	.00	1.34	20254.15
340.23	1.35	4.201	1.6827	.00	1.35	20335.65
340.24	1.35	4.218	1.6837	.00	1.35	20416.96
340.25	1.36	4.235	1.6847	.00	1.36	20498.55
340.26	1.36	4.252	1.6857	.00	1.36	20580.20
340.27	1.36	4.269	1.6867	.00	1.36	20661.65
340.28	1.37	4.286	1.6877	.00	1.37	20743.40
340.29	1.37	4.302	1.6887	.00	1.37	20825.19

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.30	1.38	4.319	1.6897	.00	1.38	20906.78
340.31	1.38	4.336	1.6907	.00	1.38	20988.67
340.32	1.38	4.353	1.6917	.00	1.38	21070.61
340.33	1.39	4.370	1.6927	.00	1.39	21152.34
340.34	1.39	4.387	1.6937	.00	1.39	21234.38
340.35	1.40	4.404	1.6947	.00	1.40	21316.46
340.36	1.40	4.421	1.6957	.00	1.40	21398.34
340.37	1.40	4.438	1.6967	.00	1.40	21480.52
340.38	1.41	4.455	1.6977	.00	1.41	21562.74
340.39	1.41	4.472	1.6987	.00	1.41	21645.02
340.40	1.42	4.489	1.6996	.00	1.42	21727.09
340.41	1.42	4.506	1.7006	.00	1.42	21809.46
340.42	1.42	4.523	1.7016	.00	1.42	21891.88
340.43	1.43	4.540	1.7026	.00	1.43	21974.10
340.44	1.43	4.557	1.7036	.00	1.43	22056.61
340.45	1.44	4.574	1.7046	.00	1.44	22139.18
340.46	1.44	4.591	1.7056	.00	1.44	22221.54
340.47	1.44	4.608	1.7066	.00	1.44	22304.20
340.48	1.45	4.625	1.7076	.00	1.45	22386.91
340.49	1.45	4.642	1.7086	.00	1.45	22469.42

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.50	1.45	4.659	1.7096	.00	1.45	22552.22
340.51	1.46	4.676	1.7106	.00	1.46	22635.08
340.52	1.46	4.693	1.7116	.00	1.46	22717.73
340.53	1.47	4.711	1.7126	.00	1.47	22800.68
340.54	1.47	4.728	1.7136	.00	1.47	22883.68
340.55	1.47	4.745	1.7146	.00	1.47	22966.48
340.56	1.48	4.762	1.7156	.00	1.48	23049.57
340.57	1.48	4.779	1.7166	.00	1.48	23132.72
340.58	1.48	4.796	1.7176	.00	1.48	23215.66
340.59	1.49	4.814	1.7186	.00	1.49	23298.91
340.60	1.49	4.831	1.7197	.00	1.49	23382.20
340.61	1.50	4.848	1.7207	.00	1.50	23465.29
340.62	1.50	4.865	1.7217	.00	1.50	23548.67
340.63	1.50	4.882	1.7227	.00	1.50	23632.11
340.64	1.51	4.900	1.7237	.00	1.51	23715.60
340.65	1.51	4.917	1.7247	.00	1.51	23798.88
340.66	1.51	4.934	1.7257	.00	1.51	23882.46
340.67	1.52	4.951	1.7267	.00	1.52	23966.09
340.68	1.52	4.969	1.7277	.00	1.52	24049.52
340.69	1.52	4.986	1.7287	.00	1.52	24133.25

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.70	1.53	5.003	1.7297	.00	1.53	24217.02
340.71	1.53	5.020	1.7307	.00	1.53	24300.59
340.72	1.54	5.038	1.7317	.00	1.54	24384.47
340.73	1.54	5.055	1.7327	.00	1.54	24468.40
340.74	1.54	5.072	1.7337	.00	1.54	24552.11
340.75	1.55	5.090	1.7347	.00	1.55	24636.13
340.76	1.55	5.107	1.7357	.00	1.55	24720.20
340.77	1.55	5.124	1.7367	.00	1.55	24804.07
340.78	1.56	5.142	1.7377	.00	1.56	24888.24
340.79	1.56	5.159	1.7388	.00	1.56	24972.45
340.80	1.56	5.177	1.7398	.00	1.56	25056.46
340.81	1.57	5.194	1.7408	.00	1.57	25140.78
340.82	1.57	5.211	1.7418	.00	1.57	25225.14
340.83	1.57	5.229	1.7428	.00	1.57	25309.30
340.84	1.58	5.246	1.7438	.00	1.58	25393.76
340.85	1.58	5.264	1.7448	.00	1.58	25478.27
340.86	1.58	5.281	1.7458	.00	1.58	25562.57
340.87	1.59	5.299	1.7468	.00	1.59	25647.18
340.88	1.59	5.316	1.7478	.00	1.59	25731.84
340.89	1.60	5.334	1.7489	.00	1.60	25816.54

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.90	1.60	5.351	1.7499	.00	1.60	25901.04
340.91	1.60	5.369	1.7509	.00	1.60	25985.84
340.92	1.61	5.386	1.7519	.00	1.61	26070.70
340.93	1.61	5.404	1.7529	.00	1.61	26155.34
340.94	1.61	5.421	1.7539	.00	1.61	26240.29
340.95	1.62	5.439	1.7549	.00	1.62	26325.29
340.96	1.62	5.456	1.7559	.00	1.62	26410.08
340.97	1.62	5.474	1.7570	.00	1.62	26495.18
340.98	1.63	5.491	1.7580	.00	1.63	26580.33
340.99	1.63	5.509	1.7590	.00	1.63	26665.27
341.00	1.63	5.527	1.7600	.00	1.63	26750.51
341.01	1.64	5.544	1.7611	.00	1.64	26835.81
341.02	1.64	5.562	1.7622	.00	1.64	26920.90
341.03	1.64	5.579	1.7633	.00	1.64	27006.30
341.04	1.65	5.597	1.7643	.00	1.65	27091.75
341.05	1.65	5.615	1.7654	.00	1.65	27177.00
341.06	1.65	5.632	1.7665	.00	1.65	27262.56
341.07	1.66	5.650	1.7676	.00	1.66	27348.17
341.08	1.66	5.668	1.7687	.00	1.66	27433.58
341.09	1.66	5.685	1.7698	.00	1.66	27519.29

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.10	1.67	5.703	1.7709	.00	1.67	27605.06
341.11	1.67	5.721	1.7719	.00	1.67	27690.62
341.12	1.67	5.739	1.7730	.00	1.67	27776.50
341.13	1.68	5.756	1.7741	.00	1.68	27862.42
341.14	1.68	5.774	1.7752	.00	1.68	27948.41
341.15	1.68	5.792	1.7763	.00	1.68	28034.18
341.16	1.69	5.810	1.7774	.00	1.69	28120.26
341.17	1.69	5.827	1.7785	.00	1.69	28206.40
341.18	1.69	5.845	1.7796	.00	1.69	28292.33
341.19	1.70	5.863	1.7806	.00	1.70	28378.57
341.20	1.70	5.881	1.7817	.00	1.70	28464.87
341.21	1.70	5.899	1.7828	.00	1.70	28550.96
341.22	1.71	5.916	1.7839	.00	1.71	28637.36
341.23	1.71	5.934	1.7850	.00	1.71	28723.82
341.24	1.71	5.952	1.7861	.00	1.71	28810.06
341.25	1.72	5.970	1.7872	.00	1.72	28896.62
341.26	1.72	5.988	1.7883	.00	1.72	28983.24
341.27	1.72	6.006	1.7894	.00	1.72	29069.64
341.28	1.73	6.024	1.7905	.00	1.73	29156.36
341.29	1.73	6.042	1.7916	.00	1.73	29243.13

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.30	1.73	6.059	1.7926	.00	1.73	29329.69
341.31	1.73	6.077	1.7937	.00	1.73	29416.57
341.32	1.74	6.095	1.7948	.00	1.74	29503.50
341.33	1.74	6.113	1.7959	.00	1.74	29590.22
341.34	1.74	6.131	1.7970	.00	1.74	29677.26
341.35	1.75	6.149	1.7981	.00	1.75	29764.35
341.36	1.75	6.167	1.7992	.00	1.75	29851.23
341.37	1.75	6.185	1.8003	.00	1.75	29938.42
341.38	1.76	6.203	1.8014	.00	1.76	30025.67
341.39	1.76	6.221	1.8025	.00	1.76	30112.98
341.40	1.76	6.239	1.8036	.00	1.76	30200.07
341.41	1.77	6.257	1.8047	.00	1.77	30287.47
341.42	1.77	6.275	1.8058	.00	1.77	30374.94
341.43	1.77	6.293	1.8069	.00	1.77	30462.19
341.44	1.78	6.312	1.8080	.00	1.78	30549.76
341.45	1.78	6.330	1.8091	.00	1.78	30637.38
341.46	1.78	6.348	1.8102	.00	1.78	30724.79
341.47	1.78	6.366	1.8113	.00	1.78	30812.52
341.48	1.79	6.384	1.8124	.00	1.79	30900.29
341.49	1.79	6.402	1.8135	.00	1.79	30987.86

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.50	1.79	6.420	1.8146	.00	1.79	31075.75
341.51	1.84	6.438	1.8157	.00	1.84	31163.74
341.52	1.93	6.457	1.8168	.00	1.93	31251.55
341.53	2.05	6.475	1.8179	.00	2.05	31339.71
341.54	2.18	6.493	1.8190	.00	2.18	31427.95
341.55	2.34	6.511	1.8201	.00	2.34	31515.98
341.56	2.50	6.529	1.8212	.00	2.50	31604.35
341.57	2.69	6.548	1.8223	.00	2.69	31692.79
341.58	2.88	6.566	1.8234	.00	2.88	31781.04
341.59	3.09	6.584	1.8245	.00	3.09	31869.61
341.60	3.31	6.602	1.8256	.00	3.31	31958.25
341.61	3.55	6.620	1.8267	.00	3.55	32046.68
341.62	3.79	6.639	1.8278	.00	3.79	32135.45
341.63	4.04	6.657	1.8289	.00	4.04	32224.28
341.64	4.31	6.675	1.8300	.00	4.31	32313.18
341.65	4.57	6.694	1.8311	.00	4.57	32401.86
341.66	4.84	6.712	1.8322	.00	4.84	32490.87
341.67	5.12	6.730	1.8333	.00	5.12	32579.94
341.68	5.41	6.749	1.8344	.00	5.41	32668.80
341.69	5.71	6.767	1.8355	.00	5.71	32758.00

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.70	6.01	6.785	1.8367	.00	6.01	32847.26
341.71	6.32	6.804	1.8378	.00	6.32	32936.31
341.72	6.64	6.822	1.8389	.00	6.64	33025.70
341.73	6.97	6.841	1.8400	.00	6.97	33115.14
341.74	7.31	6.859	1.8411	.00	7.31	33204.36
341.75	7.65	6.877	1.8422	.00	7.65	33293.93
341.76	8.00	6.896	1.8433	.00	8.00	33383.56
341.77	8.35	6.914	1.8444	.00	8.35	33472.97
341.78	8.72	6.933	1.8455	.00	8.72	33562.71
341.79	9.09	6.951	1.8466	.00	9.09	33652.52
341.80	9.46	6.970	1.8477	.00	9.46	33742.11
341.81	9.85	6.988	1.8488	.00	9.85	33832.04
341.82	10.23	7.007	1.8500	.00	10.23	33922.03
341.83	10.63	7.025	1.8511	.00	10.63	34011.80
341.84	11.03	7.044	1.8522	.00	11.03	34101.91
341.85	11.44	7.062	1.8533	.00	11.44	34192.08
341.86	11.85	7.081	1.8544	.00	11.85	34282.03
341.87	12.27	7.099	1.8555	.00	12.27	34372.32
341.88	12.69	7.118	1.8566	.00	12.69	34462.66
341.89	13.13	7.136	1.8577	.00	13.13	34553.07

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.90	13.56	7.155	1.8588	.00	13.56	34643.26
341.91	14.00	7.174	1.8600	.00	14.00	34733.79
341.92	14.45	7.192	1.8611	.00	14.45	34824.37
341.93	14.90	7.211	1.8622	.00	14.90	34914.74
341.94	15.36	7.229	1.8633	.00	15.36	35005.44
341.95	15.82	7.248	1.8644	.00	15.82	35096.20
341.96	16.29	7.267	1.8655	.00	16.29	35186.75
341.97	16.76	7.285	1.8667	.00	16.76	35277.63
341.98	17.24	7.304	1.8678	.00	17.24	35368.57
341.99	17.72	7.323	1.8689	.00	17.72	35459.29
342.00	18.21	7.341	1.8700	.00	18.21	35550.35
342.01	18.71	7.360	1.8711	.00	18.71	35641.46
342.02	19.20	7.379	1.8722	.00	19.20	35732.36
342.03	19.71	7.397	1.8733	.00	19.71	35823.59
342.04	20.21	7.416	1.8743	.00	20.21	35914.88
342.05	20.72	7.435	1.8754	.00	20.72	36005.95
342.06	21.24	7.454	1.8765	.00	21.24	36097.35
342.07	21.76	7.473	1.8776	.00	21.76	36188.81
342.08	22.29	7.491	1.8787	.00	22.29	36280.05
342.09	22.82	7.510	1.8798	.00	22.82	36371.63

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.10	23.36	7.529	1.8809	.00	23.36	36463.25
342.11	23.89	7.548	1.8819	.00	23.89	36554.66
342.12	24.44	7.567	1.8830	.00	24.44	36646.41
342.13	24.99	7.585	1.8841	.00	24.99	36738.21
342.14	25.54	7.604	1.8852	.00	25.54	36830.07
342.15	26.09	7.623	1.8863	.00	26.09	36921.71
342.16	26.65	7.642	1.8874	.00	26.65	37013.68
342.17	27.22	7.661	1.8885	.00	27.22	37105.71
342.18	27.79	7.680	1.8896	.00	27.79	37197.52
342.19	28.36	7.699	1.8907	.00	28.36	37289.66
342.20	28.94	7.718	1.8918	.00	28.94	37381.86
342.21	29.52	7.736	1.8928	.00	29.52	37473.84
342.22	30.10	7.755	1.8939	.00	30.10	37566.15
342.23	30.69	7.774	1.8950	.00	30.69	37658.53
342.24	31.28	7.793	1.8961	.00	31.28	37750.67
342.25	31.88	7.812	1.8972	.00	31.88	37843.15
342.26	32.48	7.831	1.8983	.00	32.48	37935.69
342.27	33.08	7.850	1.8994	.00	33.08	38028.01
342.28	33.69	7.869	1.9005	.00	33.69	38120.66
342.29	34.30	7.888	1.9016	.00	34.30	38213.38

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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No Infiltration

INITIAL CONDITIONS

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Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.30	34.91	7.907	1.9027	.00	34.91	38305.86
342.31	35.53	7.926	1.9038	.00	35.53	38398.68
342.32	36.15	7.945	1.9049	.00	36.15	38491.55
342.33	36.77	7.964	1.9060	.00	36.77	38584.21
342.34	37.39	7.983	1.9070	.00	37.39	38677.20
342.35	38.02	8.003	1.9081	.00	38.02	38770.24
342.36	38.65	8.022	1.9092	.00	38.65	38863.06
342.37	39.28	8.041	1.9103	.00	39.28	38956.22
342.38	39.91	8.060	1.9114	.00	39.91	39049.43
342.39	40.54	8.079	1.9125	.00	40.54	39142.68
342.40	41.17	8.098	1.9136	.00	41.17	39235.71
342.41	41.80	8.117	1.9147	.00	41.80	39329.08
342.42	42.43	8.136	1.9158	.00	42.43	39422.50
342.43	43.06	8.156	1.9169	.00	43.06	39515.69
342.44	43.69	8.175	1.9180	.00	43.69	39609.21
342.45	44.32	8.194	1.9191	.00	44.32	39702.80
342.46	44.95	8.213	1.9202	.00	44.95	39796.14
342.47	45.57	8.232	1.9213	.00	45.57	39889.82
342.48	46.19	8.252	1.9224	.00	46.19	39983.55
342.49	46.79	8.271	1.9235	.00	46.79	40077.03

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
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No Infiltration

INITIAL CONDITIONS

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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.50	47.78	8.290	1.9246	.00	47.78	40171.23
342.51	47.84	8.309	1.9257	.00	47.84	40264.56
342.52	47.91	8.328	1.9268	.00	47.91	40357.67
342.53	47.99	8.348	1.9279	.00	47.99	40451.12
342.54	48.06	8.367	1.9290	.00	48.06	40544.62
342.55	48.13	8.386	1.9301	.00	48.13	40637.89
342.56	48.19	8.406	1.9312	.00	48.19	40731.49
342.57	48.26	8.425	1.9323	.00	48.26	40825.14
342.58	48.34	8.444	1.9334	.00	48.34	40918.58
342.59	48.41	8.464	1.9345	.00	48.41	41012.34
342.60	48.48	8.483	1.9356	.00	48.48	41106.17
342.61	48.54	8.502	1.9367	.00	48.54	41199.74
342.62	48.61	8.522	1.9378	.00	48.61	41293.67
342.63	48.70	8.541	1.9389	.00	48.70	41387.67
342.64	48.78	8.561	1.9400	.00	48.78	41481.71
342.65	48.83	8.580	1.9411	.00	48.83	41575.49
342.66	48.91	8.599	1.9422	.00	48.91	41669.64
342.67	48.98	8.619	1.9434	.00	48.98	41763.84
342.68	49.06	8.638	1.9445	.00	49.06	41857.80
342.69	49.12	8.658	1.9456	.00	49.12	41952.09

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
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Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.70	49.19	8.677	1.9467	.00	49.19	42046.45
342.71	49.26	8.697	1.9478	.00	49.26	42140.57
342.72	49.33	8.716	1.9489	.00	49.33	42235.02
342.73	49.40	8.736	1.9500	.00	49.40	42329.54
342.74	49.47	8.755	1.9511	.00	49.47	42423.82
342.75	49.53	8.775	1.9522	.00	49.53	42518.43
342.76	49.60	8.794	1.9533	.00	49.60	42613.12
342.77	49.68	8.814	1.9544	.00	49.68	42707.56
342.78	49.74	8.833	1.9555	.00	49.74	42802.33
342.79	49.81	8.853	1.9566	.00	49.81	42897.18
342.80	49.88	8.872	1.9577	.00	49.88	42991.78
342.81	49.95	8.892	1.9589	.00	49.95	43086.71
342.82	50.02	8.912	1.9600	.00	50.02	43181.71
342.83	50.09	8.931	1.9611	.00	50.09	43276.48
342.84	50.15	8.951	1.9622	.00	50.15	43371.58
342.85	50.23	8.970	1.9633	.00	50.23	43466.75
342.86	50.29	8.990	1.9644	.00	50.29	43561.66
342.87	50.36	9.010	1.9655	.00	50.36	43656.92
342.88	50.43	9.029	1.9666	.00	50.43	43752.24
342.89	50.50	9.049	1.9677	.00	50.50	43847.61

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
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Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.90	50.57	9.069	1.9689	.00	50.57	43942.76
342.91	50.63	9.088	1.9700	.00	50.63	44038.23
342.92	50.71	9.108	1.9711	.00	50.71	44133.77
342.93	50.76	9.128	1.9722	.00	50.76	44229.06
342.94	50.84	9.147	1.9733	.00	50.84	44324.71
342.95	50.90	9.167	1.9744	.00	50.90	44420.40
342.96	50.98	9.187	1.9755	.00	50.98	44515.87
342.97	51.03	9.207	1.9767	.00	51.03	44611.66
342.98	51.11	9.227	1.9778	.00	51.11	44707.53
342.99	51.17	9.246	1.9789	.00	51.17	44803.14
343.00	51.24	9.266	1.9800	.00	51.24	44899.11
343.01	51.37	9.286	1.9811	.00	51.37	44995.19
343.02	51.56	9.306	1.9822	.00	51.56	45091.09
343.03	51.77	9.326	1.9833	.00	51.77	45187.36
343.04	52.01	9.345	1.9843	.00	52.01	45283.71
343.05	52.26	9.365	1.9854	.00	52.26	45379.83
343.06	52.55	9.385	1.9865	.00	52.55	45476.33
343.07	52.84	9.405	1.9876	.00	52.84	45572.89
343.08	53.16	9.425	1.9887	.00	53.16	45669.24
343.09	53.48	9.445	1.9898	.00	53.48	45765.93

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

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Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
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Starting Total Qout= .00 cfs
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Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.10	53.83	9.465	1.9909	.00	53.83	45862.71
343.11	54.19	9.485	1.9920	.00	54.19	45959.25
343.12	54.55	9.504	1.9930	.00	54.55	46056.14
343.13	54.94	9.524	1.9941	.00	54.94	46153.12
343.14	55.34	9.544	1.9952	.00	55.34	46250.15
343.15	55.75	9.564	1.9963	.00	55.75	46346.96
343.16	56.17	9.584	1.9974	.00	56.17	46444.11
343.17	56.60	9.604	1.9985	.00	56.60	46541.34
343.18	57.04	9.624	1.9996	.00	57.04	46638.34
343.19	57.50	9.644	2.0007	.00	57.50	46735.69
343.20	57.96	9.664	2.0018	.00	57.96	46833.11
343.21	58.43	9.684	2.0029	.00	58.43	46930.29
343.22	58.92	9.704	2.0039	.00	58.92	47027.84
343.23	59.41	9.724	2.0050	.00	59.41	47125.44
343.24	59.90	9.744	2.0061	.00	59.90	47222.80
343.25	60.41	9.764	2.0072	.00	60.41	47320.52
343.26	60.94	9.785	2.0083	.00	60.94	47418.33
343.27	61.46	9.805	2.0094	.00	61.46	47515.88
343.28	62.00	9.825	2.0105	.00	62.00	47613.79
343.29	62.54	9.845	2.0116	.00	62.54	47711.77

LEVEL POOL ROUTING DATA

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Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.30	63.09	9.865	2.0127	.00	63.09	47809.50
343.31	63.66	9.885	2.0138	.00	63.66	47907.61
343.32	64.22	9.905	2.0149	.00	64.22	48005.76
343.33	64.80	9.925	2.0160	.00	64.80	48103.67
343.34	65.38	9.946	2.0171	.00	65.38	48201.95
343.35	65.97	9.966	2.0182	.00	65.97	48300.29
343.36	66.57	9.986	2.0193	.00	66.57	48398.40
343.37	67.18	10.006	2.0204	.00	67.18	48496.86
343.38	67.79	10.026	2.0215	.00	67.79	48595.38
343.39	68.42	10.047	2.0225	.00	68.42	48693.96
343.40	69.04	10.067	2.0236	.00	69.04	48792.30
343.41	69.67	10.087	2.0247	.00	69.67	48891.00
343.42	70.31	10.107	2.0258	.00	70.31	48989.75
343.43	70.95	10.128	2.0269	.00	70.95	49088.27
343.44	71.61	10.148	2.0280	.00	71.61	49187.16
343.45	72.28	10.168	2.0291	.00	72.28	49286.11
343.46	72.94	10.188	2.0302	.00	72.94	49384.80
343.47	73.61	10.209	2.0313	.00	73.61	49483.86
343.48	74.30	10.229	2.0324	.00	74.30	49582.98
343.49	74.98	10.249	2.0335	.00	74.98	49681.85

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
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Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.50	75.68	10.270	2.0346	.00	75.68	49781.09
343.51	76.37	10.290	2.0357	.00	76.37	49880.39
343.52	77.08	10.310	2.0368	.00	77.08	49979.45
343.53	77.78	10.331	2.0379	.00	77.78	50078.86
343.54	78.50	10.351	2.0390	.00	78.50	50178.34
343.55	79.22	10.372	2.0401	.00	79.22	50277.56
343.56	79.95	10.392	2.0412	.00	79.95	50377.16
343.57	80.69	10.412	2.0423	.00	80.69	50476.82
343.58	81.43	10.433	2.0434	.00	81.43	50576.23
343.59	82.18	10.453	2.0445	.00	82.18	50676.00
343.60	82.93	10.474	2.0456	.00	82.93	50775.83
343.61	83.68	10.494	2.0467	.00	83.68	50875.41
343.62	84.43	10.515	2.0478	.00	84.43	50975.36
343.63	85.24	10.535	2.0490	.00	85.24	51075.40
343.64	86.01	10.556	2.0501	.00	86.01	51175.46
343.65	86.79	10.576	2.0512	.00	86.79	51275.29
343.66	87.57	10.597	2.0523	.00	87.57	51375.47
343.67	88.37	10.617	2.0534	.00	88.37	51475.71
343.68	89.16	10.638	2.0545	.00	89.16	51575.71
343.69	89.96	10.658	2.0556	.00	89.96	51676.07

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.70	90.76	10.679	2.0567	.00	90.76	51776.49
343.71	91.58	10.699	2.0578	.00	91.58	51876.67
343.72	92.40	10.720	2.0589	.00	92.40	51977.21
343.73	93.21	10.741	2.0600	.00	93.21	52077.80
343.74	94.04	10.761	2.0611	.00	94.04	52178.16
343.75	94.87	10.782	2.0622	.00	94.87	52278.87
343.76	95.71	10.802	2.0633	.00	95.71	52379.64
343.77	96.54	10.823	2.0644	.00	96.54	52480.16
343.78	97.40	10.844	2.0655	.00	97.40	52581.05
343.79	98.25	10.864	2.0667	.00	98.25	52682.00
343.80	99.10	10.885	2.0678	.00	99.10	52782.70
343.81	99.96	10.906	2.0689	.00	99.96	52883.77
343.82	100.82	10.926	2.0700	.00	100.82	52984.89
343.83	101.69	10.947	2.0711	.00	101.69	53085.76
343.84	102.57	10.968	2.0722	.00	102.57	53187.00
343.85	103.45	10.989	2.0733	.00	103.45	53288.30
343.86	104.33	11.009	2.0744	.00	104.33	53389.36
343.87	105.22	11.030	2.0755	.00	105.22	53490.77
343.88	106.11	11.051	2.0766	.00	106.11	53592.23
343.89	107.01	11.072	2.0778	.00	107.01	53693.77

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.90	107.91	11.092	2.0789	.00	107.91	53795.05
343.91	108.81	11.113	2.0800	.00	108.81	53896.70
343.92	109.72	11.134	2.0811	.00	109.72	53998.41
343.93	110.64	11.155	2.0822	.00	110.64	54099.86
343.94	111.55	11.176	2.0833	.00	111.55	54201.68
343.95	112.48	11.197	2.0844	.00	112.48	54303.57
343.96	113.40	11.217	2.0855	.00	113.40	54405.20
343.97	114.34	11.238	2.0867	.00	114.34	54507.20
343.98	115.28	11.259	2.0878	.00	115.28	54609.26
343.99	116.22	11.280	2.0889	.00	116.22	54711.06
344.00	117.17	11.301	2.0900	.00	117.17	54813.24
344.01	118.11	11.322	2.0911	.00	118.11	54915.46
344.02	119.07	11.343	2.0922	.00	119.07	55017.44
344.03	120.03	11.364	2.0933	.00	120.03	55119.79
344.04	120.99	11.385	2.0943	.00	120.99	55222.19
344.05	121.95	11.405	2.0954	.00	121.95	55324.33
344.06	122.92	11.426	2.0965	.00	122.92	55426.85
344.07	123.90	11.447	2.0976	.00	123.90	55529.43
344.08	124.88	11.468	2.0987	.00	124.88	55631.75
344.09	125.86	11.489	2.0998	.00	125.86	55734.43

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.10	126.85	11.510	2.1009	.00	126.85	55837.18
344.11	127.83	11.531	2.1020	.00	127.83	55939.65
344.12	128.83	11.552	2.1031	.00	128.83	56042.51
344.13	129.83	11.573	2.1041	.00	129.83	56145.42
344.14	130.83	11.595	2.1052	.00	130.83	56248.39
344.15	131.83	11.616	2.1063	.00	131.83	56351.11
344.16	132.84	11.637	2.1074	.00	132.84	56454.18
344.17	133.86	11.658	2.1085	.00	133.86	56557.32
344.18	134.88	11.679	2.1096	.00	134.88	56660.21
344.19	135.90	11.700	2.1107	.00	135.90	56763.46
344.20	136.93	11.721	2.1118	.00	136.93	56866.77
344.21	137.96	11.742	2.1129	.00	137.96	56969.83
344.22	138.99	11.763	2.1140	.00	138.99	57073.25
344.23	140.03	11.784	2.1151	.00	140.03	57176.73
344.24	141.06	11.806	2.1161	.00	141.06	57279.94
344.25	142.10	11.827	2.1172	.00	142.10	57383.54
344.26	143.15	11.848	2.1183	.00	143.15	57487.19
344.27	144.21	11.869	2.1194	.00	144.21	57590.58
344.28	145.27	11.890	2.1205	.00	145.27	57694.35
344.29	146.32	11.912	2.1216	.00	146.32	57798.16

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.30	147.38	11.933	2.1227	.00	147.38	57901.72
344.31	148.46	11.954	2.1238	.00	148.46	58005.66
344.32	149.52	11.975	2.1249	.00	149.52	58109.64
344.33	150.60	11.996	2.1260	.00	150.60	58213.38
344.34	151.68	12.018	2.1271	.00	151.68	58317.48
344.35	152.76	12.039	2.1282	.00	152.76	58421.64
344.36	153.83	12.060	2.1293	.00	153.83	58525.53
344.37	154.93	12.082	2.1304	.00	154.93	58629.81
344.38	156.02	12.103	2.1315	.00	156.02	58734.14
344.39	157.12	12.124	2.1326	.00	157.12	58838.53
344.40	158.21	12.146	2.1337	.00	158.21	58942.65
344.41	159.32	12.167	2.1348	.00	159.32	59047.15
344.42	160.42	12.188	2.1359	.00	160.42	59151.70
344.43	161.53	12.210	2.1370	.00	161.53	59256.00
344.44	162.65	12.231	2.1381	.00	162.65	59360.68
344.45	163.76	12.252	2.1392	.00	163.76	59465.40
344.46	164.88	12.274	2.1402	.00	164.88	59569.86
344.47	166.00	12.295	2.1413	.00	166.00	59674.71
344.48	167.13	12.317	2.1424	.00	167.13	59779.61
344.49	168.25	12.338	2.1435	.00	168.25	59884.23

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.50	169.39	12.359	2.1446	.00	169.39	59989.23
344.51	170.53	12.381	2.1457	.00	170.53	60094.30
344.52	171.67	12.402	2.1468	.00	171.67	60199.11
344.53	172.80	12.424	2.1479	.00	172.80	60304.28
344.54	173.95	12.445	2.1491	.00	173.95	60409.52
344.55	175.10	12.467	2.1502	.00	175.10	60514.50
344.56	176.25	12.488	2.1513	.00	176.25	60619.84
344.57	177.42	12.510	2.1524	.00	177.42	60725.25
344.58	178.57	12.531	2.1535	.00	178.57	60830.39
344.59	179.73	12.553	2.1546	.00	179.73	60935.91
344.60	180.90	12.575	2.1557	.00	180.90	61041.49
344.61	182.06	12.596	2.1568	.00	182.06	61146.79
344.62	183.24	12.618	2.1579	.00	183.24	61252.49
344.63	184.41	12.639	2.1590	.00	184.41	61358.23
344.64	185.60	12.661	2.1601	.00	185.60	61464.04
344.65	186.77	12.682	2.1612	.00	186.77	61569.57
344.66	187.96	12.704	2.1623	.00	187.96	61675.49
344.67	189.14	12.726	2.1634	.00	189.14	61781.45
344.68	190.34	12.747	2.1645	.00	190.34	61887.17
344.69	191.53	12.769	2.1656	.00	191.53	61993.25

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.70	192.73	12.791	2.1667	.00	192.73	62099.40
344.71	193.92	12.812	2.1678	.00	193.92	62205.27
344.72	195.13	12.834	2.1689	.00	195.13	62311.52
344.73	196.33	12.856	2.1700	.00	196.33	62417.84
344.74	197.54	12.877	2.1711	.00	197.54	62523.88
344.75	198.75	12.899	2.1722	.00	198.75	62630.30
344.76	199.97	12.921	2.1733	.00	199.97	62736.79
344.77	201.30	12.943	2.1744	.00	201.30	62843.11
344.78	202.52	12.964	2.1756	.00	202.52	62949.71
344.79	203.74	12.986	2.1767	.00	203.74	63056.36
344.80	204.97	13.008	2.1778	.00	204.97	63162.75
344.81	206.20	13.030	2.1789	.00	206.20	63269.51
344.82	207.44	13.051	2.1800	.00	207.44	63376.33
344.83	208.68	13.073	2.1811	.00	208.68	63482.89
344.84	209.92	13.095	2.1822	.00	209.92	63589.82
344.85	211.16	13.117	2.1833	.00	211.16	63696.^1
344.86	212.41	13.139	2.1844	.00	212.41	63803.54
344.87	213.66	13.161	2.1855	.00	213.66	63910.66
344.88	214.92	13.182	2.1867	.00	214.92	64017.82
344.89	216.17	13.204	2.1878	.00	216.17	64125.04

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 1 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.50 ft
Starting Volume = 1.450 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0050 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.90	217.43	13.226	2.1889	.00	217.43	64231.99
344.91	218.68	13.248	2.1900	.00	218.68	64339.32
344.92	219.94	13.270	2.1911	.00	219.94	64446.70
344.93	221.21	13.292	2.1922	.00	221.21	64553.82
344.94	222.48	13.314	2.1933	.00	222.48	64661.33
344.95	223.75	13.336	2.1944	.00	223.75	64768.89
344.96	225.03	13.358	2.1955	.00	225.03	64876.18
344.97	226.31	13.380	2.1967	.00	226.31	64983.86
344.98	227.60	13.402	2.1978	.00	227.60	65091.60
344.99	228.87	13.424	2.1989	.00	228.87	65199.05
345.00	230.17	13.446	2.2000	.00	230.17	65306.89

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1 W/ 1FTIN  2
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 2

Pond Node   Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.50 ft
Starting Volume    =    1.450 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    115.43 cfs    at   11.9450 hrs
Peak Outflow      =      1.93 cfs    at   18.2800 hrs
-----------------------------------------------------
Peak Elevation    =    341.52 ft
Peak Storage =          6.455 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =       1.450
+ HYG Vol IN   =       6.739
- Infiltration =        .000
- HYG Vol OUT  =       1.939
- Retained Vol =       6.250
                  ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1 W/ 1FTIN  10
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 10

Pond Node   Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.50 ft
Starting Volume    =    1.450 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    194.21 cfs    at   11.9450 hrs
Peak Outflow      =     30.66 cfs    at   12.2500 hrs
-----------------------------------------------------
Peak Elevation    =    342.23 ft
Peak Storage =          7.773 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =       1.450
+ HYG Vol IN   =      11.528
- Infiltration =        .000
- HYG Vol OUT  =       6.541
- Retained Vol =       6.436
                  ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1 W/ 1FTIN  25
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 25

Pond Node   Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.50 ft
Starting Volume    =    1.450 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    236.83 cfs    at   11.9450 hrs
Peak Outflow      =     50.03 cfs    at   12.1800 hrs
-----------------------------------------------------
Peak Elevation    =    342.82 ft
Peak Storage =          8.914 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =       1.450
+ HYG Vol IN   =      14.186
- Infiltration =        .000
- HYG Vol OUT  =       9.164
- Retained Vol =       6.472
                   ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 1 W/ 1FTIN  100
Outflow HYG file = NONE STORED - POND 1 W/ 1FTOUT 100

Pond Node   Data = POND 1 W/ 1FT
Pond Volume Data = POND 1 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.50 ft
Starting Volume    =    1.450 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0050 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    304.82 cfs    at   11.9400 hrs
Peak Outflow      =     95.59 cfs    at   12.1250 hrs
-----------------------------------------------------
Peak Elevation    =    343.76 ft
Peak Storage =         10.799 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =       1.450
+ HYG Vol IN   =      18.494
- Infiltration =        .000
- HYG Vol OUT  =      13.428
- Retained Vol =       6.516
                  ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

Index of Starting Page Numbers for ID Names

----- H -----
Harnett County... 3.01

----- L -----
LF AREA 3... 4.01, 5.01, 6.03, 6.04,
6.05, 6.06
LF AREA 4&5... 4.03, 5.02, 6.07,
6.08, 6.09, 6.10
LF AREA3... 4.05, 5.03
LF AREA45... 4.07, 5.04

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9.41, 9.42
POND 1 W/ 1FT... 7.02, 9.43, 9.81,
9.82, 9.83, 9.84
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6.13, 6.14
PRE-DEV... 4.11, 5.06, 6.15, 6.16,
6.17, 6.18

----- R -----
Riser/Barrel... 8.01, 8.05
Riser/Barrelwlft... 8.25, 8.28

----- S -----
SP-1DA... 4.13, 5.07

----- W -----
Watershed... 1.01, 2.01, 2.03, 2.05,
2.07

Minimum Drain Time

==========
JOB TITLE
==========

Project Date: 9/28/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

TIME vs. ELEVATION (ft)

Output Time increment = .5000 hrs
Time on left represents time for first value in each row.

Time hrs					
.0000	339.20	339.15	339.10	339.06	339.01
2.5000	338.97	338.93	338.89	338.85	338.81
5.0000	338.77	338.74	338.70	338.67	338.64
7.5000	338.62	338.59	338.57	338.54	338.52
10.0000	338.50	338.48	338.46	338.44	338.42
12.5000	338.40	338.38	338.36	338.34	338.32
15.0000	338.31	338.29	338.27	338.25	338.24
17.5000	338.22	338.21	338.19	338.17	338.16
20.0000	338.14	338.13	338.11	338.10	338.08
22.5000	338.06	338.05	338.03	338.02	338.00
25.0000	337.99	337.97	337.95	337.94	337.92
27.5000	337.91	337.89	337.88	337.86	337.85
30.0000	337.84	337.83	337.83	337.82	337.81
32.5000	337.80	337.80	337.79	337.78	337.77
35.0000	337.77	337.76	337.75	337.75	337.74
37.5000	337.74	337.73	337.72	337.72	337.71
40.0000	337.71	337.70	337.70	337.69	337.69
42.5000	337.68	337.68	337.68	337.67	337.67
45.0000	337.66	337.66	337.65	337.65	337.65
47.5000	337.64	337.64	337.64	337.63	337.63
50.0000	337.63	337.63	337.63	337.63	337.63
52.5000	337.63	337.63	337.63	337.63	337.63
55.0000	337.63	337.63	337.63	337.63	337.63
57.5000	337.63	337.63	337.63	337.63	337.63
60.0000	337.63	337.63	337.63	337.63	337.63
62.5000	337.63	337.63	337.63	337.63	337.63
65.0000	337.63	337.63	337.63	337.63	337.63
67.5000	337.63	337.63	337.63	337.63	337.63
70.0000	337.63	337.63	337.63	337.63	337.63

First Flush Volume

==========
JOB TITLE
==========

Project Date: 9/28/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

BMP FIRST FLUSH CALCULATIONS

First Flush Depth = 1.0000 in
Drainage Area = 33.390 acres

Volume = Flush Depth * Drainage Area

First Flush volume = 2.783 ac-ft

Sediment Pond 2

Minimum Drain Time

==========
JOB TITLE
==========

Project Date: 9/28/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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********** TIME VS.ELEV **********

MIN DRAIN TIMOUT Time-Elev 1.01

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

TIME vs. ELEVATION (ft)

Output Time increment = .5000 hrs
Time on left represents time for first value in each row.

Time hrs					
.0000	339.20	339.15	339.10	339.06	339.01
2.5000	338.97	338.93	338.89	338.85	338.81
5.0000	338.77	338.74	338.70	338.67	338.64
7.5000	338.62	338.59	338.57	338.54	338.52
10.0000	338.50	338.48	338.46	338.44	338.42
12.5000	338.40	338.38	338.36	338.34	338.32
15.0000	338.31	338.29	338.27	338.25	338.24
17.5000	338.22	338.21	338.19	338.17	338.16
20.0000	338.14	338.13	338.11	338.10	338.08
22.5000	338.06	338.05	338.03	338.02	338.00
25.0000	337.99	337.97	337.95	337.94	337.92
27.5000	337.91	337.89	337.88	337.86	337.85
30.0000	337.84	337.83	337.83	337.82	337.81
32.5000	337.80	337.80	337.79	337.78	337.77
35.0000	337.77	337.76	337.75	337.75	337.74
37.5000	337.74	337.73	337.72	337.72	337.71
40.0000	337.71	337.70	337.70	337.69	337.69
42.5000	337.68	337.68	337.68	337.67	337.67
45.0000	337.66	337.66	337.65	337.65	337.65
47.5000	337.64	337.64	337.64	337.63	337.63
50.0000	337.63	337.63	337.63	337.63	337.63
52.5000	337.63	337.63	337.63	337.63	337.63
55.0000	337.63	337.63	337.63	337.63	337.63
57.5000	337.63	337.63	337.63	337.63	337.63
60.0000	337.63	337.63	337.63	337.63	337.63
62.5000	337.63	337.63	337.63	337.63	337.63
65.0000	337.63	337.63	337.63	337.63	337.63
67.5000	337.63	337.63	337.63	337.63	337.63
70.0000	337.63	337.63	337.63	337.63	337.63

Index of Starting Page Numbers for ID Names

----- M -----
MIN DRAIN TIMOUT... 1.01

First Flush Volume

==========
JOB TITLE
==========

Project Date: 9/28/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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********** BMP (BEST MGT. PRACTICES) **********

FIRST FLUSH..... 1st Flush 1.01

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 1-EC.PPW

BMP FIRST FLUSH CALCULATIONS

First Flush Depth = 1.0000 in
Drainage Area = 33.390 acres

Volume = Flush Depth * Drainage Area

First Flush volume = 2.783 ac-ft

Index of Starting Page Numbers for ID Names

----- F -----
FIRST FLUSH... 1.01

Sediment Pond 2

==========
JOB TITLE
==========

Project Date: 10/4/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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Executive Summary (Links) 2.01

Watershed..... 10
Executive Summary (Links) 2.03

Watershed..... 25
Executive Summary (Links) 2.05

Watershed..... 100
Executive Summary (Links) 2.07

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Harnett County.. Design Storms 3.01

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LF AREA 1..... Tc Calcs 4.01

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LF AREA1..... Tc Calcs 4.05

LF AREA2..... Tc Calcs 4.07

PRE DEV..... Tc Calcs 4.09

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********** CN CALCULATIONS **********

LF AREA 1..... Runoff CN-Area 5.01

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LF AREA1..... Runoff CN-Area 5.03

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PRE DEV..... Runoff CN-Area 5.05

SP-2 DA..... Runoff CN-Area 5.06

SP-2DA..... Runoff CN-Area 5.07

********** RUNOFF HYDROGRAPHS **********

Unit Hyd. Equations 6.01

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Unit Hyd. Summary 6.03

LF AREA 1..... 10
Unit Hyd. Summary 6.04

LF AREA 1..... 25
Unit Hyd. Summary 6.05

LF AREA 1..... 100
Unit Hyd. Summary 6.06

LF AREA 2..... 2
Unit Hyd. Summary 6.07

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Unit Hyd. Summary 6.08

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LF AREA 2..... 25
Unit Hyd. Summary 6.09

LF AREA 2..... 100
Unit Hyd. Summary 6.10

PRE DEV..... 2
Unit Hyd. Summary 6.11

PRE DEV..... 10
Unit Hyd. Summary 6.12

PRE DEV..... 25
Unit Hyd. Summary 6.13

PRE DEV..... 100
Unit Hyd. Summary 6.14

SP-2 DA..... 2
Unit Hyd. Summary 6.15

SP-2 DA..... 10
Unit Hyd. Summary 6.16

SP-2 DA..... 25
Unit Hyd. Summary 6.17

SP-2 DA..... 100
Unit Hyd. Summary 6.18

********** POND VOLUMES **********

POND 2..... Vol: Elev-Area 7.01

POND 2 W/ 1FT... Vol: Elev-Area 7.02

********** OUTLET STRUCTURES **********

Riser/Barrel.... Outlet Input Data 8.01

Table of Contents (continued)

Composite Rating Curve 8.05

Riser/Barrelw1ft Outlet Input Data 8.24
Composite Rating Curve 8.27

********** POND ROUTING **********

POND 2..... Pond E-V-Q Table 9.01

POND 2 OUT 2
Pond Routing Summary 9.37

POND 2 OUT 10
Pond Routing Summary 9.38

POND 2 OUT 25
Pond Routing Summary 9.39

POND 2 OUT 100
Pond Routing Summary 9.40

POND 2 W/ 1FT... Pond E-V-Q Table 9.41

POND 2 W/ 1FTOUT 2
Pond Routing Summary 9.77

POND 2 W/ 1FTOUT 10
Pond Routing Summary 9.78

POND 2 W/ 1FTOUT 25
Pond Routing Summary 9.79

POND 2 W/ 1FTOUT 100
Pond Routing Summary 9.80

MASTER DESIGN STORM SUMMARY

Network Storm Collection: Harnett County

Return Event	Total Depth in	Rainfall Type	RNF ID
2	3.8000	Synthetic Curve	TypeII 24hr
10	5.6000	Synthetic Curve	TypeII 24hr
25	6.6000	Synthetic Curve	TypeII 24hr
100	8.2000	Synthetic Curve	TypeII 24hr

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
LF AREA 1	AREA	2	1.581		11.9200	29.60		
LF AREA 1	AREA	10	2.694		11.9200	49.46		
LF AREA 1	AREA	25	3.329		11.9200	60.50		
LF AREA 1	AREA	100	4.361		11.9200	78.06		
LF AREA 2	AREA	2	1.865		11.9200	34.92		
LF AREA 2	AREA	10	3.177		11.9200	58.34		
LF AREA 2	AREA	25	3.927		11.9200	71.36		
LF AREA 2	AREA	100	5.144		11.9200	92.08		
LF AREA1	AREA	2	1.581		11.9200	29.60		
LF AREA1	AREA	10	2.694		11.9200	49.46		
LF AREA1	AREA	25	3.329		11.9200	60.50		
LF AREA1	AREA	100	4.361		11.9200	78.06		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
LF AREA2	AREA	2	1.865		11.9200	34.92		
LF AREA2	AREA	10	3.177		11.9200	58.34		
LF AREA2	AREA	25	3.927		11.9200	71.36		
LF AREA2	AREA	100	5.144		11.9200	92.08		
*OUT 2	JCT	2	2.561		13.6800	2.80		
*OUT 2	JCT	10	3.859		13.7900	3.90		
*OUT 2	JCT	25	4.795		12.9000	8.37		
*OUT 2	JCT	100	6.986		12.1000	48.69		
*OUT 2 W/ 1FT	JCT	2	1.804		15.0500	1.72		
*OUT 2 W/ 1FT	JCT	10	3.039		13.1700	5.80		
*OUT 2 W/ 1FT	JCT	25	4.453		12.2800	18.12		
*OUT 2 W/ 1FT	JCT	100	6.808		12.0600	81.80		
POND 2 IN	POND	2	4.008		11.9200	73.80		
POND 2 IN	POND	10	6.715		11.9200	121.54		
POND 2 IN	POND	25	8.259		11.9200	148.06		
POND 2 IN	POND	100	10.759		11.9200	190.30		
POND 2 OUT	POND	2	2.561		13.6800	2.80	340.77	2.472
POND 2 OUT	POND	10	3.859		13.7900	3.90	342.53	4.326
POND 2 OUT	POND	25	4.796		12.9000	8.37	343.21	5.101
POND 2 OUT	POND	100	6.986		12.1000	48.69	343.78	5.784
POND 2 W/ 1FTIN	POND	2	4.008		11.9200	73.80		
POND 2 W/ 1FTIN	POND	10	6.715		11.9200	121.54		
POND 2 W/ 1FTIN	POND	25	8.259		11.9200	148.06		
POND 2 W/ 1FTIN	POND	100	10.759		11.9200	190.30		
POND 2 W/ 1FTOUT	POND	2	1.804		15.0500	1.72	341.77	3.500
POND 2 W/ 1FTOUT	POND	10	3.039		13.1700	5.80	343.18	5.077
POND 2 W/ 1FTOUT	POND	25	4.454		12.2800	18.12	343.49	5.441
POND 2 W/ 1FTOUT	POND	100	6.808		12.0600	81.80	344.10	6.183

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
PRE DEV.	AREA	2	.155		11.9300	2.77		
PRE DEV.	AREA	10	.327		11.9300	6.12		
PRE DEV.	AREA	25	.433		11.9300	8.15		
PRE DEV.	AREA	100	.614		11.9200	11.56		
*PRE OUT	JCT	2	.155		11.9300	2.77		
*PRE OUT	JCT	10	.327		11.9300	6.12		
*PRE OUT	JCT	25	.433		11.9300	8.15		
*PRE OUT	JCT	100	.614		11.9200	11.56		
SP-2 DA	AREA	2	.562		11.9200	9.27		
SP-2 DA	AREA	10	.845		11.9200	13.73		
SP-2 DA	AREA	25	1.002		11.9200	16.21		
SP-2 DA	AREA	100	1.254		11.9200	20.16		
SP-2DA	AREA	2	.562		11.9200	9.27		
SP-2DA	AREA	10	.845		11.9200	13.73		
SP-2DA	AREA	25	1.002		11.9200	16.21		
SP-2DA	AREA	100	1.254		11.9200	20.16		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 2

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 2 yr
Total Rainfall Depth= 3.8000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	1.865		11.9200	34.92	LF AREA 2
		DL	1.865		11.9200	34.92	
		DN	4.008		11.9200	73.80	POND 2 W/ 1FTIN
ADDLINK 20	ADD	UN	.155		11.9300	2.77	PRE DEV.
		DL	.155		11.9300	2.77	
		DN	.155		11.9300	2.77	PRE OUT
ADDLINK 30	ADD	UN	1.581		11.9200	29.60	LF AREA 1
		DL	1.581		11.9200	29.60	
		DN	4.008		11.9200	73.80	POND 2 W/ 1FTIN
ADDLINK 40	ADD	UN	.562		11.9200	9.27	SP-2 DA
		DL	.562		11.9200	9.27	
		DN	4.008		11.9200	73.80	POND 2 W/ 1FTIN
ADDLINK 50	ADD	UN	1.581		11.9200	29.60	LF AREA1
		DL	1.581		11.9200	29.60	
		DN	4.008		11.9200	73.80	POND 2 IN
ADDLINK 60	ADD	UN	1.865		11.9200	34.92	LF AREA2
		DL	1.865		11.9200	34.92	
		DN	4.008		11.9200	73.80	POND 2 IN
ADDLINK 70	ADD	UN	.562		11.9200	9.27	SP-2DA
		DL	.562		11.9200	9.27	
		DN	4.008		11.9200	73.80	POND 2 IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	4.008		11.9200	73.80	POND 2 W/ 1FTIN	
ROUTE 10			1.804		15.0500	1.72	POND 2 W/ 1FTOUT	
		DL	1.804		15.0500	1.72		
		DN	1.804		15.0500	1.72	OUT 2 W/ 1FT	
ROUTE 20	PONDrt	UN	4.008		11.9200	73.80	POND 2	IN
ROUTE 20			2.561		13.6800	2.80	POND 2	OUT
		DL	2.561		13.6800	2.80		
		DN	2.561		13.6800	2.80	OUT 2	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 10

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 10 yr
Total Rainfall Depth= 5.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	3.177		11.9200	58.34	LF AREA 2
		DL	3.177		11.9200	58.34	
		DN	6.715		11.9200	121.54	POND 2 W/ 1FTIN
ADDLINK 20	ADD	UN	.327		11.9300	6.12	PRE DEV.
		DL	.327		11.9300	6.12	
		DN	.327		11.9300	6.12	PRE OUT
ADDLINK 30	ADD	UN	2.694		11.9200	49.46	LF AREA 1
		DL	2.694		11.9200	49.46	
		DN	6.715		11.9200	121.54	POND 2 W/ 1FTIN
ADDLINK 40	ADD	UN	.845		11.9200	13.73	SP-2 DA
		DL	.845		11.9200	13.73	
		DN	6.715		11.9200	121.54	POND 2 W/ 1FTIN
ADDLINK 50	ADD	UN	2.694		11.9200	49.46	LF AREA1
		DL	2.694		11.9200	49.46	
		DN	6.715		11.9200	121.54	POND 2 IN
ADDLINK 60	ADD	UN	3.177		11.9200	58.34	LF AREA2
		DL	3.177		11.9200	58.34	
		DN	6.715		11.9200	121.54	POND 2 IN
ADDLINK 70	ADD	UN	.845		11.9200	13.73	SP-2DA
		DL	.845		11.9200	13.73	
		DN	6.715		11.9200	121.54	POND 2 IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	6.715		11.9200	121.54	POND 2 W/ 1FTIN	
ROUTE 10			3.039		13.1700	5.80	POND 2 W/ 1FTOUT	
		DL	3.039		13.1700	5.80		
		DN	3.039		13.1700	5.80	OUT 2 W/ 1FT	
ROUTE 20	PONDrt	UN	6.715		11.9200	121.54	POND 2	IN
ROUTE 20			3.859		13.7900	3.90	POND 2	OUT
		DL	3.859		13.7900	3.90		
		DN	3.859		13.7900	3.90	OUT 2	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 25

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 25 yr
Total Rainfall Depth= 6.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	3.927		11.9200	71.36	LF AREA 2
		DL	3.927		11.9200	71.36	
		DN	8.259		11.9200	148.06	POND 2 W/ 1FTIN
ADDLINK 20	ADD	UN	.433		11.9300	8.15	PRE DEV.
		DL	.433		11.9300	8.15	
		DN	.433		11.9300	8.15	PRE OUT
ADDLINK 30	ADD	UN	3.329		11.9200	60.50	LF AREA 1
		DL	3.329		11.9200	60.50	
		DN	8.259		11.9200	148.06	POND 2 W/ 1FTIN
ADDLINK 40	ADD	UN	1.002		11.9200	16.21	SP-2 DA
		DL	1.002		11.9200	16.21	
		DN	8.259		11.9200	148.06	POND 2 W/ 1FTIN
ADDLINK 50	ADD	UN	3.329		11.9200	60.50	LF AREA1
		DL	3.329		11.9200	60.50	
		DN	8.259		11.9200	148.06	POND 2 IN
ADDLINK 60	ADD	UN	3.927		11.9200	71.36	LF AREA2
		DL	3.927		11.9200	71.36	
		DN	8.259		11.9200	148.06	POND 2 IN
ADDLINK 70	ADD	UN	1.002		11.9200	16.21	SP-2DA
		DL	1.002		11.9200	16.21	
		DN	8.259		11.9200	148.06	POND 2 IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	8.259		11.9200	148.06	POND 2 W/ 1FTIN	
ROUTE 10			4.454		12.2800	18.12	POND 2 W/ 1FTOUT	
		DL	4.453		12.2800	18.12		
		DN	4.453		12.2800	18.12	OUT 2 W/ 1FT	
ROUTE 20	PONDrt	UN	8.259		11.9200	148.06	POND 2	IN
ROUTE 20			4.796		12.9000	8.37	POND 2	OUT
		DL	4.795		12.9000	8.37		
		DN	4.795		12.9000	8.37	OUT 2	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 100 yr
Total Rainfall Depth= 8.2000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	5.144		11.9200	92.08	LF AREA 2
		DL	5.144		11.9200	92.08	
		DN	10.759		11.9200	190.30	POND 2 W/ 1FTIN
ADDLINK 20	ADD	UN	.614		11.9200	11.56	PRE DEV.
		DL	.614		11.9200	11.56	
		DN	.614		11.9200	11.56	PRE OUT
ADDLINK 30	ADD	UN	4.361		11.9200	78.06	LF AREA 1
		DL	4.361		11.9200	78.06	
		DN	10.759		11.9200	190.30	POND 2 W/ 1FTIN
ADDLINK 40	ADD	UN	1.254		11.9200	20.16	SP-2 DA
		DL	1.254		11.9200	20.16	
		DN	10.759		11.9200	190.30	POND 2 W/ 1FTIN
ADDLINK 50	ADD	UN	4.361		11.9200	78.06	LF AREA1
		DL	4.361		11.9200	78.06	
		DN	10.759		11.9200	190.30	POND 2 IN
ADDLINK 60	ADD	UN	5.144		11.9200	92.08	LF AREA2
		DL	5.144		11.9200	92.08	
		DN	10.759		11.9200	190.30	POND 2 IN
ADDLINK 70	ADD	UN	1.254		11.9200	20.16	SP-2DA
		DL	1.254		11.9200	20.16	
		DN	10.759		11.9200	190.30	POND 2 IN

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ROUTE 10	PONDrt	UN	10.759		11.9200	190.30	POND 2 W/ 1FTIN	
ROUTE 10			6.808		12.0600	81.80	POND 2 W/ 1FTOUT	
		DL	6.808		12.0600	81.80		
		DN	6.808		12.0600	81.80	OUT 2 W/ 1FT	
ROUTE 20	PONDrt	UN	10.759		11.9200	190.30	POND 2	IN
ROUTE 20			6.986		12.1000	48.69	POND 2	OUT
		DL	6.986		12.1000	48.69		
		DN	6.986		12.1000	48.69	OUT 2	

File.... K:\02\02-330\02334.01 LF Expansion\H&H\
Title... Project Date: 10/4/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

DESIGN STORMS SUMMARY

Design Storm File,ID = Harnett County

Storm Tag Name = 2

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 2 yr
Total Rainfall Depth= 3.8000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 10

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 10 yr
Total Rainfall Depth= 5.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 25

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 25 yr
Total Rainfall Depth= 6.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 100 yr
Total Rainfall Depth= 8.2000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	86	8.020			86.00
COMPOSITE AREA & WEIGHTED CN --->		8.020			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	86	9.460			86.00
COMPOSITE AREA & WEIGHTED CN --->		9.460			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	86	8.020			86.00
COMPOSITE AREA & WEIGHTED CN --->		8.020			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	9.460			86.00
COMPOSITE AREA & WEIGHTED CN --->		9.460			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	69	1.630			69.00
COMPOSITE AREA & WEIGHTED CN --->		1.630			69.00 (69)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	98	1.890			98.00

COMPOSITE AREA & WEIGHTED CN ---> 1.890 98.00 (98)
::

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	98	1.890			98.00
COMPOSITE AREA & WEIGHTED CN --->		1.890			98.00 (98)

SCS UNIT HYDROGRAPH METHOD
(Computational Notes)

DEFINITION OF TERMS: ----------

```
At    = Total area (acres):  At = Ai+Ap
Ai    = Impervious area (acres)
Ap    = Pervious area (acres)
CNi   = Runoff curve number for impervious area
CNp   = Runoff curve number for pervious area
fLoss = f loss constant infiltration (depth/time)
gKs   = Saturated Hydraulic Conductivity (depth/time)
Md    = Volumetric Moisture Deficit
Psi   = Capillary Suction (length)
hK    = Horton Infiltration Decay Rate (time^-1)
fo    = Initial Infiltration Rate (depth/time)
fc    = Ultimate(capacity)Infiltration Rate (depth/time)
Ia    = Initial Abstraction (length)
dt    = Computational increment (duration of unit excess rainfall)
        Default dt is smallest value of 0.1333Tc, rtm, and th
        (Smallest dt is then adjusted to match up with Tp)
UDdt  = User specified override computational main time increment
        (only used if UDdt is => .1333Tc)
D(t)  = Point on distribution curve (fraction of P) for time step t

K     = 2 / (1 + (Tr/Tp)):  default K = 0.75: (for Tr/Tp = 1.67)
Ks    = Hydrograph shape factor
      = Unit Conversions * K:
      = ((1hr/3600sec) * (1ft/12in) * ((5280ft)**2/sq.mi)) * K
        Default Ks = 645.333 * 0.75 = 484

Lag   = Lag time from center of excess runoff (dt) to Tp:  Lag = 0.6Tc
P     = Total precipitation depth, inches
Pa(t) = Accumulated rainfall at time step t
Pi(t) = Incremental rainfall at time step t
qp    = Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi.
      = (Ks * A * Q) / Tp   (where Q = 1in. runoff, A=sq.mi.)
Qu(t) = Unit hydrograph ordinate (cfs) at time step t
Q(t)  = Final hydrograph ordinate (cfs) at time step t
Rai(t)= Accumulated runoff (inches) at time step t for impervious area
Rap(t)= Accumulated runoff (inches) at time step t for pervious area
Rii(t)= Incremental runoff (inches) at time step t for impervious area
Rip(t)= Incremental runoff (inches) at time step t for pervious area
R(t)  = Incremental weighted total runoff (inches)
Rtm   = Time increment for rainfall table
Si    = S for impervious area:  Si = (1000/CNi) - 10
Sp    = S for pervious area:    Sp = (1000/CNp) - 10
t     = Time step (row) number
Tc    = Time of concentration
Tb    = Time (hrs) of entire unit hydrograph:  Tb = Tp + Tr
Tp    = Time (hrs) to peak of a unit hydrograph:  Tp = (dt/2) + Lag
Tr    = Time (hrs) of receding limb of unit hydrograph:  Tr = ratio of Tp
```

SCS UNIT HYDROGRAPH METHOD
(Computational Notes)

```
PRECIPITATION: ------------------------------------------------------------
Column (1):  Time for time step t
Column (2):  D(t)  = Point on distribution curve for time step t
Column (3):  Pi(t) = Pa(t) - Pa(t-1): Col.(4) - Preceding Col.(4)
Column (4):  Pa(t) = D(t) x P:        Col.(2) x P


PERVIOUS AREA RUNOFF (using SCS Runoff CN Method) -------------------------
Column (5):  Rap(t) = Accumulated pervious runoff for time step t
             If (Pa(t) is <= 0.2Sp) then use: Rap(t) = 0.0
             If (Pa(t) is >  0.2Sp) then use:

             Rap(t) = (Col.(4)-0.2Sp)**2 / (Col.(4)+0.8Sp)

Column (6):  Rip(t) = Incremental pervious runoff for time step t
             Rip(t) =         Rap(t)          -         Rap(t-1)
             Rip(t) = Col.(5) for current row - Col.(5) for preceding row.

IMPERVIOUS AREA RUNOFF ----------------------------------------------------
Column (7 & 8)... Did not specify to use impervious areas.


INCREMENTAL WEIGHTED RUNOFF: ----------------------------------------------
Column (9):  R(t)  = (Ap/At) x Rip(t)   +  (Ai/At) x Rii(t)
             R(t)  = (Ap/At) x Col.(6)  +  (Ai/At) x Col.(8)


SCS UNIT HYDROGRAPH METHOD: -----------------------------------------------
Column (10): Q(t)   is computed with the SCS unit hydrograph method
                    using R() and Qu().
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.8000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 2
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    29.62 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    29.60 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 1
               CN   =   86
               Area =       8.020 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3659 in
                        1.581 ac-ft

HYG Volume...           1.581 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 10
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    49.54 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    49.46 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 1
              CN   =   86
              Area =       8.020 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       2.694 ac-ft

HYG Volume...          2.694 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 25
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    60.61 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    60.50 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 1
               CN   =   86
               Area =       8.020 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        3.330 ac-ft

HYG Volume...           3.329 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 100
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    78.23 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    78.06 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 1
              CN   =   86
              Area =       8.020 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       4.361 ac-ft

HYG Volume...          4.361 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.8000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 2
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

==========================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    34.94 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    34.92 cfs
==========================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 2
               CN   =   86
               Area =       9.460 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3659 in
                        1.865 ac-ft

HYG Volume...           1.865 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 10
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    58.43 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    58.34 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 2
              CN   =   86
              Area =       9.460 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       3.177 ac-ft

HYG Volume...          3.177 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 25
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    71.49 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    71.36 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 2
               CN   =   86
               Area =       9.460 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        3.927 ac-ft

HYG Volume...          3.927 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 100
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    92.28 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    92.08 cfs
=============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 2
              CN   =   86
              Area =       9.460 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       5.144 ac-ft

HYG Volume...          5.144 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.8000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE DEV. 2
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.630 acres  Runoff CN= 69

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9286 hrs
Computed Peak Flow             =     2.77 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9302 hrs
Peak Flow, Interpolated Output =     2.77 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:PRE DEV.
              CN   =   69
              Area =       1.630 acres
              S    =   4.4928 in
              0.2S =    .8986 in

               Cumulative Runoff
              -------------------
                      1.1385 in
                        .155 ac-ft

HYG Volume...            155 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: PRE DEV.)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    22.17 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE DEV. 10
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.630 acres  Runoff CN= 69

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9286 hrs
Computed Peak Flow             =     6.13 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9302 hrs
Peak Flow, Interpolated Output =     6.12 cfs
============================================

                 DRAINAGE AREA
              -------------------
              ID:PRE DEV.
              CN   =   69
              Area =       1.630 acres
              S    =   4.4928 in
              0.2S =    .8986 in

               Cumulative Runoff
              -------------------
                      2.4041 in
                        .327 ac-ft

HYG Volume...           .327 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: PRE DEV.)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    22.17 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE DEV. 25
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.630 acres  Runoff CN= 69

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9286 hrs
Computed Peak Flow             =     8.17 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9302 hrs
Peak Flow, Interpolated Output =     8.15 cfs
=============================================

                 DRAINAGE AREA
               -------------------
               ID:PRE DEV.
               CN   =   69
               Area =       1.630 acres
               S    =   4.4928 in
               0.2S =    .8986 in

                Cumulative Runoff
               -------------------
                       3.1887 in
                         .433 ac-ft

HYG Volume...            .433 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: PRE DEV.)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    22.17 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - PRE DEV. 100
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.630 acres  Runoff CN= 69

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9286 hrs
Computed Peak Flow             =    11.57 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    11.56 cfs
=============================================

                DRAINAGE AREA
              -------------------
              ID:PRE DEV.
              CN   =   69
              Area =       1.630 acres
              S    =   4.4928 in
              0.2S =    .8986 in

               Cumulative Runoff
              -------------------
                      4.5201 in
                        .614 ac-ft

HYG Volume...           .614 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: PRE DEV.)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    22.17 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.8000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - SP-2 DA 2
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.890 acres  Runoff CN= 98

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =     9.30 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =     9.27 cfs
=============================================

                 DRAINAGE AREA
               --------------------
               ID:SP-2 DA
               CN   =   98
               Area =       1.890 acres
               S    =    .2041 in
               0.2S =    .0408 in

                Cumulative Runoff
               --------------------
                       3.5656 in
                         .562 ac-ft

HYG Volume...            .562 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: SP-2 DA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.71 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - SP-2 DA 10
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.890 acres  Runoff CN= 98

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    13.77 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    13.73 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:SP-2 DA
              CN   =   98
              Area =       1.890 acres
              S    =    .2041 in
              0.2S =    .0408 in

               Cumulative Runoff
              -------------------
                      5.3623 in
                        .845 ac-ft

HYG Volume...           .845 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: SP-2 DA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.71 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - SP-2 DA 25
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.890 acres  Runoff CN= 98

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    16.25 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    16.21 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:SP-2 DA
               CN   =   98
               Area =       1.890 acres
               S    =    .2041 in
               0.2S =    .0408 in

                Cumulative Runoff
               -------------------
                       6.3613 in
                        1.002 ac-ft

HYG Volume...          1.002 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: SP-2 DA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.71 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - SP-2 DA 100
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.890 acres  Runoff CN= 98

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    20.22 cfs

Time Increment for HYG File    =    .0100 hrs
Peak Time, Interpolated Output =  11.9202 hrs
Peak Flow, Interpolated Output =    20.16 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:SP-2 DA
             CN   =   98
             Area =       1.890 acres
             S    =    .2041 in
             0.2S =    .0408 in

              Cumulative Runoff
             -------------------
                     7.9601 in
                      1.254 ac-ft

HYG Volume...          1.254 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: SP-2 DA)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.71 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
338.00	-----	.8000	.0000	.000	.000
342.00	-----	1.0800	2.8095	3.746	3.746
344.00	-----	1.2300	3.4626	2.308	6.054
345.00	-----	1.3100	3.8094	1.270	7.324

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Volume = (1/3) * (EL2-EL1) * (Area1 + Area2 + sq.rt.(Area1*Area2))

where: EL1, EL2 = Lower and upper elevations of the increment
Area1,Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
338.00	-----	.8000	.0000	.000	.000
342.00	-----	1.0800	2.8095	3.746	3.746
344.00	-----	1.2300	3.4626	2.308	6.054
345.00	-----	1.3100	3.8094	1.270	7.324

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Volume = (1/3) * (EL2-EL1) * (Area1 + Area2 + sq.rt.(Area1*Area2))

where: EL1, EL2 = Lower and upper elevations of the increment
Area1,Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

REQUESTED POND WS ELEVATIONS:

Min. Elev.= 338.00 ft
Increment = .01 ft
Max. Elev.= 345.00 ft

****
OUTLET CONNECTIVITY
****

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.		Outfall	E1, ft	E2, ft
Weir-Rectangular	WR	--->	TW	343.500	345.000
Stand Pipe	SP	--->	CV	343.000	345.000
Orifice-Circular	O2	--->	CV	339.000	345.000
Orifice-Circular	O1	--->	CV	338.040	345.000
Culvert-Circular	CV	--->	TW	338.040	345.000

TW SETUP, DS Channel

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

OUTLET STRUCTURE INPUT DATA

```
Structure ID       = WR
Structure Type     = Weir-Rectangular
-----------------------------------
# of Openings      =         1
Crest Elev.        =    343.50 ft
Weir Length        =     30.00 ft
Weir Coeff.        =  3.000000

Weir TW effects      (Use adjustment equation)
```

```
Structure ID       = SP
Structure Type     = Stand Pipe
-----------------------------------
# of Openings      =         1
Invert Elev.       =    343.00 ft
Diameter           =    5.0000 ft
Orifice Area       =   19.6350 sq.ft
Orifice Coeff.     =      .600
Weir Length        =     15.71 ft
Weir Coeff.        =     3.000
K, Submerged       =      .000
K, Reverse         =     1.000
Kb,Barrel          =   .000000  (per ft of full flow)
Barrel Length      =       .00 ft
Mannings n         =     .0000
```

```
Structure ID       = O2
Structure Type     = Orifice-Circular
-----------------------------------
# of Openings      =        10
Invert Elev.       =    339.00 ft
Diameter           =     .1667 ft
Orifice Coeff.     =      .600
```

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

OUTLET STRUCTURE INPUT DATA

```
Structure ID      = O1
Structure Type    = Orifice-Circular
-----------------------------------
# of Openings     =        10
Invert Elev.      =    338.04 ft
Diameter          =     .1670 ft
Orifice Coeff.    =      .600


Structure ID      = CV
Structure Type    = Culvert-Circular
-----------------------------------
No. Barrels       =         1
Barrel Diameter   =    2.5000 ft
Upstream Invert   =    338.04 ft
Dnstream Invert   =    337.62 ft
Horiz. Length     =     83.38 ft
Barrel Length     =     83.38 ft
Barrel Slope      =    .00504 ft/ft

OUTLET CONTROL DATA...
Mannings n        =     .0220
Ke                =     .9000  (forward entrance loss)
Kb                =   .026397  (per ft of full flow)
Kr                =     .9000  (reverse entrance loss)
HW Convergence    =      .001  +/- ft

INLET CONTROL DATA...
Equation form     =         1
Inlet Control K   =     .0340
Inlet Control M   =    1.5000
Inlet Control c   =    .05530
Inlet Control Y   =     .5400
T1 ratio (HW/D)   =     1.260
T2 ratio (HW/D)   =     1.422
Slope Factor      =     -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use   submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...
At T1 Elev =    341.19 ft  --->  Flow =     27.16 cfs
At T2 Elev =    341.60 ft  --->  Flow =     31.05 cfs
```

OUTLET STRUCTURE INPUT DATA

```
Structure ID      = TW
Structure Type    = TW SETUP, DS Channel
-----------------------------------
FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...
Maximum Iterations=    30
Min. TW tolerance =    .01 ft
Max. TW tolerance =    .01 ft
Min. HW tolerance =    .01 ft
Max. HW tolerance =    .01 ft
Min.  Q tolerance =    .10 cfs
Max.  Q tolerance =    .10 cfs
```

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
338.00	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
338.01	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
338.02	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
338.03	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
338.04	.00	Free Outfall		(no Q: WR,SP,O2,O1,CV)
338.05	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.06	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.07	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.08	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.09	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.10	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.11	.00	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.12	.02	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.13	.02	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.14	.02	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.15	.02	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.16	.05	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.17	.12	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.18	.12	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.19	.15	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.20	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.21	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.22	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.23	.17	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.24	.24	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.25	.24	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.26	.25	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.27	.25	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.28	.25	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.29	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.30	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.31	.26	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.32	.39	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.33	.39	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.34	.39	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.35	.39	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.36	.42	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.37	.43	Free Outfall		O1,CV (no Q: WR,SP,O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: Converge TW Elev ft	Converge Error +/-ft	Contributing Structures
338.38	.53	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.39	.53	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.40	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.41	.55	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.42	.57	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.43	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.44	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.45	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.46	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.47	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.48	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.49	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.50	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.51	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.52	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.53	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.54	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.55	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.56	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.57	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.58	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.59	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.60	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.61	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.62	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.63	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.64	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.65	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.66	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.67	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.68	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.69	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.70	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.71	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.72	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.73	.58	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.74	.59	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.75	.59	Free Outfall		O1,CV (no Q: WR,SP,O2)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
338.76	.60	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.77	.60	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.78	.61	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.79	.61	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.80	.62	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.81	.63	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.82	.63	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.83	.64	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.84	.65	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.85	.65	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.86	.66	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.87	.67	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.88	.68	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.89	.68	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.90	.69	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.91	.69	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.92	.70	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.93	.71	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.94	.71	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.95	.72	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.96	.73	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.97	.73	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.98	.74	Free Outfall		O1,CV (no Q: WR,SP,O2)
338.99	.75	Free Outfall		O1,CV (no Q: WR,SP,O2)
339.00	.75	Free Outfall		O1,CV (no Q: WR,SP,O2)
339.01	.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.02	.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.03	.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.04	.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.05	.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.06	.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.07	.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.08	.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.09	.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.10	.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.11	.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.12	.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.13	.99	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.14	1.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.15	1.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.16	1.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.17	1.11	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.18	1.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.19	1.15	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.20	1.17	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.21	1.19	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.22	1.21	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.23	1.23	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.24	1.25	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.25	1.26	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.26	1.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.27	1.30	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.28	1.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.29	1.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.30	1.34	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.31	1.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.32	1.37	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.33	1.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.34	1.40	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.35	1.42	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.36	1.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.37	1.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.38	1.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.39	1.48	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.40	1.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.41	1.51	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.42	1.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.43	1.53	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.44	1.55	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.45	1.56	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.46	1.57	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.47	1.59	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.48	1.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.49	1.61	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.50	1.62	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.51	1.64	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.52	1.65	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.53	1.66	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.54	1.67	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.55	1.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.56	1.70	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.57	1.71	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.58	1.72	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.59	1.73	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.60	1.74	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.61	1.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.62	1.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.63	1.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.64	1.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.65	1.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.66	1.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.67	1.82	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.68	1.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.69	1.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.70	1.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.71	1.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.72	1.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.73	1.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.74	1.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.75	1.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.76	1.92	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.77	1.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.78	1.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.79	1.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.80	1.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.81	1.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.82	1.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.83	1.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.84	2.00	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.85	2.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.86	2.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.87	2.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.88	2.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.89	2.05	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.90	2.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.91	2.07	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.92	2.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.93	2.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.94	2.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.95	2.11	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.96	2.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.97	2.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.98	2.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
339.99	2.15	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.00	2.16	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.01	2.17	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.02	2.18	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.03	2.19	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.04	2.20	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.05	2.21	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.06	2.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.07	2.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.08	2.23	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.09	2.24	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.10	2.25	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.11	2.26	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.12	2.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.13	2.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.14	2.29	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.15	2.30	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.16	2.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.17	2.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.18	2.32	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.19	2.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.20	2.34	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.21	2.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.22	2.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.23	2.37	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.24	2.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.25	2.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.26	2.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.27	2.40	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q			Notes	
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.28	2.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.29	2.42	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.30	2.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.31	2.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.32	2.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.33	2.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.34	2.46	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.35	2.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.36	2.48	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.37	2.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.38	2.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.39	2.50	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.40	2.51	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.41	2.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.42	2.53	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.43	2.54	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.44	2.54	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.45	2.55	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.46	2.56	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.47	2.57	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.48	2.58	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.49	2.58	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.50	2.59	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.51	2.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.52	2.61	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.53	2.62	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.54	2.62	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.55	2.63	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.56	2.64	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.57	2.65	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.58	2.65	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.59	2.66	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.60	2.67	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.61	2.68	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.62	2.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.63	2.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.64	2.70	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.65	2.71	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.66	2.72	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.67	2.72	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.68	2.73	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.69	2.74	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.70	2.75	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.71	2.75	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.72	2.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.73	2.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.74	2.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.75	2.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.76	2.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.77	2.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.78	2.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.79	2.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.80	2.82	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.81	2.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.82	2.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.83	2.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.84	2.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.85	2.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.86	2.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.87	2.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.88	2.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.89	2.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.90	2.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.91	2.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.92	2.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.93	2.92	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.94	2.92	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.95	2.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.96	2.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.97	2.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.98	2.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
340.99	2.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.00	2.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.01	2.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.02	2.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.03	2.99	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: TW Elev ft	Converge Error +/-ft	Contributing Structures
341.04	2.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.05	3.00	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.06	3.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.07	3.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.08	3.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.09	3.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.10	3.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.11	3.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.12	3.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.13	3.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.14	3.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.15	3.07	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.16	3.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.17	3.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.18	3.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.19	3.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.20	3.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.21	3.11	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.22	3.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.23	3.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.24	3.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.25	3.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.26	3.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.27	3.15	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.28	3.16	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.29	3.16	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.30	3.17	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.31	3.18	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.32	3.18	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.33	3.19	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.34	3.20	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.35	3.20	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.36	3.21	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.37	3.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.38	3.22	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.39	3.23	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.40	3.24	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.41	3.24	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.42	3.25	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.43	3.26	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.44	3.26	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.45	3.27	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.46	3.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.47	3.28	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.48	3.29	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.49	3.29	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.50	3.30	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.51	3.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.52	3.31	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.53	3.32	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.54	3.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.55	3.33	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.56	3.34	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.57	3.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.58	3.35	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.59	3.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.60	3.36	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.61	3.37	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.62	3.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.63	3.38	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.64	3.39	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.65	3.40	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.66	3.40	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.67	3.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.68	3.41	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.69	3.42	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.70	3.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.71	3.43	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.72	3.44	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.73	3.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.74	3.45	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.75	3.46	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.76	3.46	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.77	3.47	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.78	3.48	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.79	3.48	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.80	3.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.81	3.49	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.82	3.50	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.83	3.51	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.84	3.51	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.85	3.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.86	3.52	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.87	3.53	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.88	3.54	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.89	3.54	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.90	3.55	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.91	3.55	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.92	3.56	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.93	3.57	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.94	3.57	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.95	3.58	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.96	3.58	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.97	3.59	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.98	3.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
341.99	3.60	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.00	3.61	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.01	3.61	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.02	3.62	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.03	3.62	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.04	3.63	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.05	3.64	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.06	3.64	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.07	3.65	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.08	3.65	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.09	3.66	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.10	3.67	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.11	3.67	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.12	3.68	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.13	3.68	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.14	3.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.15	3.69	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.16	3.70	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.17	3.71	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q, Q cfs	Notes: TW Elev ft	Converge Error +/-ft	Contributing Structures
342.18	3.71	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.19	3.72	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.20	3.72	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.21	3.73	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.22	3.73	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.23	3.74	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.24	3.75	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.25	3.75	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.26	3.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.27	3.76	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.28	3.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.29	3.77	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.30	3.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.31	3.78	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.32	3.79	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.33	3.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.34	3.80	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.35	3.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.36	3.81	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.37	3.82	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.38	3.82	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.39	3.83	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.40	3.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.41	3.84	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.42	3.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.43	3.85	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.44	3.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.45	3.86	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.46	3.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.47	3.87	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.48	3.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.49	3.88	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.50	3.89	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.51	3.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.52	3.90	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.53	3.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.54	3.91	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.55	3.92	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.56	3.92	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.57	3.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.58	3.93	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.59	3.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.60	3.94	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.61	3.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.62	3.95	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.63	3.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.64	3.96	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.65	3.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.66	3.97	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.67	3.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.68	3.98	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.69	3.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.70	3.99	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.71	4.00	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.72	4.00	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.73	4.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.74	4.01	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.75	4.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.76	4.02	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.77	4.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.78	4.03	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.79	4.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.80	4.04	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.81	4.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.82	4.05	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.83	4.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.84	4.06	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.85	4.07	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.86	4.07	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.87	4.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.88	4.08	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.89	4.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.90	4.09	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.91	4.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.92	4.10	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.93	4.11	Free Outfall		O2,O1,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
Elev. ft	Q cfs	TW Elev ft	Converge Error +/-ft	Contributing Structures
342.94	4.11	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.95	4.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.96	4.12	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.97	4.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.98	4.13	Free Outfall		O2,O1,CV (no Q: WR,SP)
342.99	4.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
343.00	4.14	Free Outfall		O2,O1,CV (no Q: WR,SP)
343.01	4.19	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.02	4.28	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.03	4.39	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.04	4.51	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.05	4.66	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.06	4.82	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.07	4.99	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.08	5.18	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.09	5.38	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.10	5.58	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.11	5.80	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.12	6.03	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.13	6.28	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.14	6.53	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.15	6.79	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.16	7.05	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.17	7.33	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.18	7.61	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.19	7.90	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.20	8.20	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.21	8.51	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.22	8.83	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.23	9.15	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.24	9.48	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.25	9.81	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.26	10.16	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.27	10.51	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.28	10.86	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.29	11.23	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.30	11.60	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.31	11.97	Free Outfall		SP,O2,O1,CV (no Q: WR)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: Converge TW Elev ft	Converge Error +/-ft	Contributing Structures
343.32	12.35	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.33	12.74	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.34	13.13	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.35	13.53	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.36	13.94	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.37	14.35	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.38	14.76	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.39	15.19	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.40	15.61	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.41	16.04	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.42	16.48	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.43	16.92	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.44	17.37	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.45	17.82	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.46	18.28	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.47	18.74	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.48	19.21	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.49	19.67	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.50	20.15	Free Outfall		SP,O2,O1,CV (no Q: WR)
343.51	20.72	Free Outfall		WR,SP,O2,O1,CV
343.52	21.37	Free Outfall		WR,SP,O2,O1,CV
343.53	22.07	Free Outfall		WR,SP,O2,O1,CV
343.54	22.81	Free Outfall		WR,SP,O2,O1,CV
343.55	23.59	Free Outfall		WR,SP,O2,O1,CV
343.56	24.40	Free Outfall		WR,SP,O2,O1,CV
343.57	25.25	Free Outfall		WR,SP,O2,O1,CV
343.58	26.12	Free Outfall		WR,SP,O2,O1,CV
343.59	27.02	Free Outfall		WR,SP,O2,O1,CV
343.60	27.94	Free Outfall		WR,SP,O2,O1,CV
343.61	28.87	Free Outfall		WR,SP,O2,O1,CV
343.62	29.83	Free Outfall		WR,SP,O2,O1,CV
343.63	30.82	Free Outfall		WR,SP,O2,O1,CV
343.64	31.82	Free Outfall		WR,SP,O2,O1,CV
343.65	32.84	Free Outfall		WR,SP,O2,O1,CV
343.66	33.88	Free Outfall		WR,SP,O2,O1,CV
343.67	34.94	Free Outfall		WR,SP,O2,O1,CV
343.68	36.01	Free Outfall		WR,SP,O2,O1,CV
343.69	37.11	Free Outfall		WR,SP,O2,O1,CV

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.70	38.22	Free Outfall		WR,SP,O2,O1,CV
343.71	39.35	Free Outfall		WR,SP,O2,O1,CV
343.72	40.49	Free Outfall		WR,SP,O2,O1,CV
343.73	41.65	Free Outfall		WR,SP,O2,O1,CV
343.74	42.82	Free Outfall		WR,SP,O2,O1,CV
343.75	44.01	Free Outfall		WR,SP,O2,O1,CV
343.76	46.32	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.77	47.56	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.78	48.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.79	50.10	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.80	50.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.81	52.24	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.82	53.09	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.83	54.42	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.84	56.26	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.85	57.10	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.86	57.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.87	58.81	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.88	59.68	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.89	60.56	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.90	61.44	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.91	62.35	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.92	63.26	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.93	64.18	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.94	65.12	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.95	66.05	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.96	67.00	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.97	67.96	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.98	68.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
343.99	69.91	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.00	70.91	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.01	71.91	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.02	72.91	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.03	73.93	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.04	74.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.05	76.00	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.06	77.04	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.07	78.09	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.08	79.15	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.09	80.23	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.10	81.31	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.11	82.39	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.12	83.49	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.13	84.60	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.14	85.71	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.15	86.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.16	87.95	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.17	89.09	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.18	90.23	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.19	91.39	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.20	92.55	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.21	93.72	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.22	94.90	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.23	96.08	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.24	97.27	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.25	98.48	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.26	99.69	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.27	100.90	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.28	102.12	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.29	103.35	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.30	104.59	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.31	105.83	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.32	107.09	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.33	108.34	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.34	109.61	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.35	110.89	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.36	112.16	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.37	113.45	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.38	114.74	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.39	116.04	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.40	117.35	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.41	118.66	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.42	119.98	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.43	121.31	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.44	122.65	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.45	124.00	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.46	125.34	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.47	126.70	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.48	128.06	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.49	129.43	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.50	130.81	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.51	132.18	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.52	133.57	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.53	134.97	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.54	136.37	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.55	137.77	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.56	139.19	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.57	140.61	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.58	142.03	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.59	143.47	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.60	144.91	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.61	146.35	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.62	147.80	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.63	149.26	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.64	150.71	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.65	152.18	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.66	153.65	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.67	155.14	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.68	156.63	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.69	158.12	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.70	159.62	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.71	161.13	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.72	162.64	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.73	164.15	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.74	165.67	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.75	167.20	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.76	168.74	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.77	170.27	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.78	171.82	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.79	173.38	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.80	174.93	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.81	176.50	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.82	178.06	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.83	179.63	Free Outfall		WR,SP,CV (no Q: O2,O1)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q, Q cfs	Converge, TW Elev ft	Converge, Error +/-ft	Notes, Contributing Structures
344.84	181.22	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.85	182.80	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.86	184.39	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.87	185.99	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.88	187.59	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.89	189.20	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.90	190.80	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.91	192.42	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.92	194.05	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.93	195.67	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.94	197.31	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.95	198.94	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.96	200.58	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.97	202.22	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.98	203.88	Free Outfall		WR,SP,CV (no Q: O2,O1)
344.99	205.54	Free Outfall		WR,SP,CV (no Q: O2,O1)
345.00	207.20	Free Outfall		WR,SP,CV (no Q: O2,O1)

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REQUESTED POND WS ELEVATIONS:

Min. Elev.= 338.00 ft
Increment = .01 ft
Max. Elev.= 345.00 ft

****
OUTLET CONNECTIVITY
****

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.		Outfall	E1, ft	E2, ft
Weir-Rectangular	WR	--->	TW	343.500	345.000
Stand Pipe	SP	--->	CV	343.000	345.000
Orifice-Circular	O2	--->	CV	339.000	345.000
Culvert-Circular	CV	--->	TW	338.040	345.000
TW SETUP, DS Channel					

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OUTLET STRUCTURE INPUT DATA

```
Structure ID      = WR
Structure Type    = Weir-Rectangular
-----------------------------------
# of Openings     =         1
Crest Elev.       =    343.50 ft
Weir Length       =     30.00 ft
Weir Coeff.       =  3.000000

Weir TW effects     (Use adjustment equation)
```

```
Structure ID      = SP
Structure Type    = Stand Pipe
-----------------------------------
# of Openings     =         1
Invert Elev.      =    343.00 ft
Diameter          =    5.0000 ft
Orifice Area      =   19.6350 sq.ft
Orifice Coeff.    =      .600
Weir Length       =     15.71 ft
Weir Coeff.       =     3.000
K, Submerged      =      .000
K, Reverse        =     1.000
Kb,Barrel         =   .000000  (per ft of full flow)
Barrel Length     =       .00 ft
Mannings n        =     .0000
```

```
Structure ID      = O2
Structure Type    = Orifice-Circular
-----------------------------------
# of Openings     =        10
Invert Elev.      =    339.00 ft
Diameter          =     .1667 ft
Orifice Coeff.    =      .600
```

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OUTLET STRUCTURE INPUT DATA

```
Structure ID      = CV
Structure Type    = Culvert-Circular
-----------------------------------
No. Barrels       =         1
Barrel Diameter   =    2.5000 ft
Upstream Invert   =    338.04 ft
Dnstream Invert   =    337.62 ft
Horiz. Length     =     83.38 ft
Barrel Length     =     83.38 ft
Barrel Slope      =    .00504 ft/ft

OUTLET CONTROL DATA...
Mannings n        =     .0220
Ke                =     .9000  (forward entrance loss)
Kb                =   .026397  (per ft of full flow)
Kr                =     .9000  (reverse entrance loss)
HW Convergence    =      .001  +/- ft

INLET CONTROL DATA...
Equation form     =         1
Inlet Control K   =     .0340
Inlet Control M   =    1.5000
Inlet Control c   =    .05530
Inlet Control Y   =     .5400
T1 ratio (HW/D)   =     1.260
T2 ratio (HW/D)   =     1.422
Slope Factor      =     -.500

Use unsubmerged inlet control Form 1 equ. below T1 elev.
Use   submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...
At T1 Elev =    341.19 ft  --->  Flow =     27.16 cfs
At T2 Elev =    341.60 ft  --->  Flow =     31.05 cfs


Structure ID      = TW
Structure Type    = TW SETUP, DS Channel
-----------------------------------
FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...
Maximum Iterations=    30
Min. TW tolerance =    .01 ft
Max. TW tolerance =    .01 ft
Min. HW tolerance =    .01 ft
Max. HW tolerance =    .01 ft
Min.  Q tolerance =    .10 cfs
Max.  Q tolerance =    .10 cfs
```

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q, Q cfs	Notes: Converge, TW Elev ft	Converge, Error +/-ft	Contributing Structures
338.00	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.01	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.02	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.03	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.04	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.05	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.06	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.07	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.08	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.09	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.10	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.11	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.12	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.13	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.14	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.15	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.16	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.17	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.18	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.19	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.20	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.21	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.22	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.23	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.24	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.25	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.26	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.27	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.28	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.29	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.30	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.31	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.32	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.33	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.34	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.35	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.36	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.37	.00	Free Outfall		(no Q: WR,SP,O2,CV)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
338.38	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.39	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.40	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.41	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.42	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.43	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.44	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.45	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.46	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.47	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.48	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.49	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.50	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.51	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.52	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.53	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.54	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.55	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.56	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.57	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.58	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.59	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.60	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.61	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.62	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.63	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.64	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.65	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.66	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.67	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.68	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.69	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.70	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.71	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.72	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.73	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.74	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.75	.00	Free Outfall		(no Q: WR,SP,O2,CV)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
338.76	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.77	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.78	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.79	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.80	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.81	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.82	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.83	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.84	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.85	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.86	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.87	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.88	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.89	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.90	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.91	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.92	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.93	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.94	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.95	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.96	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.97	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.98	.00	Free Outfall		(no Q: WR,SP,O2,CV)
338.99	.00	Free Outfall		(no Q: WR,SP,O2,CV)
339.00	.00	Free Outfall		(no Q: WR,SP,O2,CV)
339.01	.00	Free Outfall		O2,CV (no Q: WR,SP)
339.02	.00	Free Outfall		O2,CV (no Q: WR,SP)
339.03	.01	Free Outfall		O2,CV (no Q: WR,SP)
339.04	.01	Free Outfall		O2,CV (no Q: WR,SP)
339.05	.03	Free Outfall		O2,CV (no Q: WR,SP)
339.06	.05	Free Outfall		O2,CV (no Q: WR,SP)
339.07	.06	Free Outfall		O2,CV (no Q: WR,SP)
339.08	.08	Free Outfall		O2,CV (no Q: WR,SP)
339.09	.10	Free Outfall		O2,CV (no Q: WR,SP)
339.10	.12	Free Outfall		O2,CV (no Q: WR,SP)
339.11	.14	Free Outfall		O2,CV (no Q: WR,SP)
339.12	.17	Free Outfall		O2,CV (no Q: WR,SP)
339.13	.19	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.14	.22	Free Outfall		O2,CV (no Q: WR,SP)
339.15	.25	Free Outfall		O2,CV (no Q: WR,SP)
339.16	.27	Free Outfall		O2,CV (no Q: WR,SP)
339.17	.31	Free Outfall		O2,CV (no Q: WR,SP)
339.18	.33	Free Outfall		O2,CV (no Q: WR,SP)
339.19	.34	Free Outfall		O2,CV (no Q: WR,SP)
339.20	.36	Free Outfall		O2,CV (no Q: WR,SP)
339.21	.37	Free Outfall		O2,CV (no Q: WR,SP)
339.22	.39	Free Outfall		O2,CV (no Q: WR,SP)
339.23	.40	Free Outfall		O2,CV (no Q: WR,SP)
339.24	.42	Free Outfall		O2,CV (no Q: WR,SP)
339.25	.43	Free Outfall		O2,CV (no Q: WR,SP)
339.26	.44	Free Outfall		O2,CV (no Q: WR,SP)
339.27	.45	Free Outfall		O2,CV (no Q: WR,SP)
339.28	.47	Free Outfall		O2,CV (no Q: WR,SP)
339.29	.48	Free Outfall		O2,CV (no Q: WR,SP)
339.30	.49	Free Outfall		O2,CV (no Q: WR,SP)
339.31	.50	Free Outfall		O2,CV (no Q: WR,SP)
339.32	.51	Free Outfall		O2,CV (no Q: WR,SP)
339.33	.52	Free Outfall		O2,CV (no Q: WR,SP)
339.34	.53	Free Outfall		O2,CV (no Q: WR,SP)
339.35	.54	Free Outfall		O2,CV (no Q: WR,SP)
339.36	.55	Free Outfall		O2,CV (no Q: WR,SP)
339.37	.56	Free Outfall		O2,CV (no Q: WR,SP)
339.38	.57	Free Outfall		O2,CV (no Q: WR,SP)
339.39	.58	Free Outfall		O2,CV (no Q: WR,SP)
339.40	.59	Free Outfall		O2,CV (no Q: WR,SP)
339.41	.60	Free Outfall		O2,CV (no Q: WR,SP)
339.42	.61	Free Outfall		O2,CV (no Q: WR,SP)
339.43	.62	Free Outfall		O2,CV (no Q: WR,SP)
339.44	.63	Free Outfall		O2,CV (no Q: WR,SP)
339.45	.64	Free Outfall		O2,CV (no Q: WR,SP)
339.46	.64	Free Outfall		O2,CV (no Q: WR,SP)
339.47	.65	Free Outfall		O2,CV (no Q: WR,SP)
339.48	.66	Free Outfall		O2,CV (no Q: WR,SP)
339.49	.67	Free Outfall		O2,CV (no Q: WR,SP)
339.50	.68	Free Outfall		O2,CV (no Q: WR,SP)
339.51	.69	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.52	.69	Free Outfall		O2,CV (no Q: WR,SP)
339.53	.70	Free Outfall		O2,CV (no Q: WR,SP)
339.54	.71	Free Outfall		O2,CV (no Q: WR,SP)
339.55	.72	Free Outfall		O2,CV (no Q: WR,SP)
339.56	.73	Free Outfall		O2,CV (no Q: WR,SP)
339.57	.73	Free Outfall		O2,CV (no Q: WR,SP)
339.58	.74	Free Outfall		O2,CV (no Q: WR,SP)
339.59	.75	Free Outfall		O2,CV (no Q: WR,SP)
339.60	.76	Free Outfall		O2,CV (no Q: WR,SP)
339.61	.76	Free Outfall		O2,CV (no Q: WR,SP)
339.62	.77	Free Outfall		O2,CV (no Q: WR,SP)
339.63	.78	Free Outfall		O2,CV (no Q: WR,SP)
339.64	.78	Free Outfall		O2,CV (no Q: WR,SP)
339.65	.79	Free Outfall		O2,CV (no Q: WR,SP)
339.66	.80	Free Outfall		O2,CV (no Q: WR,SP)
339.67	.80	Free Outfall		O2,CV (no Q: WR,SP)
339.68	.81	Free Outfall		O2,CV (no Q: WR,SP)
339.69	.82	Free Outfall		O2,CV (no Q: WR,SP)
339.70	.82	Free Outfall		O2,CV (no Q: WR,SP)
339.71	.83	Free Outfall		O2,CV (no Q: WR,SP)
339.72	.84	Free Outfall		O2,CV (no Q: WR,SP)
339.73	.84	Free Outfall		O2,CV (no Q: WR,SP)
339.74	.85	Free Outfall		O2,CV (no Q: WR,SP)
339.75	.86	Free Outfall		O2,CV (no Q: WR,SP)
339.76	.86	Free Outfall		O2,CV (no Q: WR,SP)
339.77	.87	Free Outfall		O2,CV (no Q: WR,SP)
339.78	.88	Free Outfall		O2,CV (no Q: WR,SP)
339.79	.88	Free Outfall		O2,CV (no Q: WR,SP)
339.80	.89	Free Outfall		O2,CV (no Q: WR,SP)
339.81	.90	Free Outfall		O2,CV (no Q: WR,SP)
339.82	.90	Free Outfall		O2,CV (no Q: WR,SP)
339.83	.91	Free Outfall		O2,CV (no Q: WR,SP)
339.84	.91	Free Outfall		O2,CV (no Q: WR,SP)
339.85	.92	Free Outfall		O2,CV (no Q: WR,SP)
339.86	.93	Free Outfall		O2,CV (no Q: WR,SP)
339.87	.93	Free Outfall		O2,CV (no Q: WR,SP)
339.88	.94	Free Outfall		O2,CV (no Q: WR,SP)
339.89	.94	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
339.90	.95	Free Outfall		O2,CV (no Q: WR,SP)
339.91	.96	Free Outfall		O2,CV (no Q: WR,SP)
339.92	.96	Free Outfall		O2,CV (no Q: WR,SP)
339.93	.97	Free Outfall		O2,CV (no Q: WR,SP)
339.94	.97	Free Outfall		O2,CV (no Q: WR,SP)
339.95	.98	Free Outfall		O2,CV (no Q: WR,SP)
339.96	.98	Free Outfall		O2,CV (no Q: WR,SP)
339.97	.99	Free Outfall		O2,CV (no Q: WR,SP)
339.98	.99	Free Outfall		O2,CV (no Q: WR,SP)
339.99	1.00	Free Outfall		O2,CV (no Q: WR,SP)
340.00	1.01	Free Outfall		O2,CV (no Q: WR,SP)
340.01	1.01	Free Outfall		O2,CV (no Q: WR,SP)
340.02	1.02	Free Outfall		O2,CV (no Q: WR,SP)
340.03	1.02	Free Outfall		O2,CV (no Q: WR,SP)
340.04	1.03	Free Outfall		O2,CV (no Q: WR,SP)
340.05	1.03	Free Outfall		O2,CV (no Q: WR,SP)
340.06	1.04	Free Outfall		O2,CV (no Q: WR,SP)
340.07	1.04	Free Outfall		O2,CV (no Q: WR,SP)
340.08	1.05	Free Outfall		O2,CV (no Q: WR,SP)
340.09	1.05	Free Outfall		O2,CV (no Q: WR,SP)
340.10	1.06	Free Outfall		O2,CV (no Q: WR,SP)
340.11	1.06	Free Outfall		O2,CV (no Q: WR,SP)
340.12	1.07	Free Outfall		O2,CV (no Q: WR,SP)
340.13	1.07	Free Outfall		O2,CV (no Q: WR,SP)
340.14	1.08	Free Outfall		O2,CV (no Q: WR,SP)
340.15	1.08	Free Outfall		O2,CV (no Q: WR,SP)
340.16	1.09	Free Outfall		O2,CV (no Q: WR,SP)
340.17	1.10	Free Outfall		O2,CV (no Q: WR,SP)
340.18	1.10	Free Outfall		O2,CV (no Q: WR,SP)
340.19	1.11	Free Outfall		O2,CV (no Q: WR,SP)
340.20	1.11	Free Outfall		O2,CV (no Q: WR,SP)
340.21	1.11	Free Outfall		O2,CV (no Q: WR,SP)
340.22	1.12	Free Outfall		O2,CV (no Q: WR,SP)
340.23	1.12	Free Outfall		O2,CV (no Q: WR,SP)
340.24	1.13	Free Outfall		O2,CV (no Q: WR,SP)
340.25	1.13	Free Outfall		O2,CV (no Q: WR,SP)
340.26	1.14	Free Outfall		O2,CV (no Q: WR,SP)
340.27	1.14	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.28	1.15	Free Outfall		O2,CV (no Q: WR,SP)
340.29	1.15	Free Outfall		O2,CV (no Q: WR,SP)
340.30	1.16	Free Outfall		O2,CV (no Q: WR,SP)
340.31	1.16	Free Outfall		O2,CV (no Q: WR,SP)
340.32	1.17	Free Outfall		O2,CV (no Q: WR,SP)
340.33	1.17	Free Outfall		O2,CV (no Q: WR,SP)
340.34	1.18	Free Outfall		O2,CV (no Q: WR,SP)
340.35	1.18	Free Outfall		O2,CV (no Q: WR,SP)
340.36	1.19	Free Outfall		O2,CV (no Q: WR,SP)
340.37	1.19	Free Outfall		O2,CV (no Q: WR,SP)
340.38	1.20	Free Outfall		O2,CV (no Q: WR,SP)
340.39	1.20	Free Outfall		O2,CV (no Q: WR,SP)
340.40	1.21	Free Outfall		O2,CV (no Q: WR,SP)
340.41	1.21	Free Outfall		O2,CV (no Q: WR,SP)
340.42	1.21	Free Outfall		O2,CV (no Q: WR,SP)
340.43	1.22	Free Outfall		O2,CV (no Q: WR,SP)
340.44	1.22	Free Outfall		O2,CV (no Q: WR,SP)
340.45	1.23	Free Outfall		O2,CV (no Q: WR,SP)
340.46	1.23	Free Outfall		O2,CV (no Q: WR,SP)
340.47	1.24	Free Outfall		O2,CV (no Q: WR,SP)
340.48	1.24	Free Outfall		O2,CV (no Q: WR,SP)
340.49	1.25	Free Outfall		O2,CV (no Q: WR,SP)
340.50	1.25	Free Outfall		O2,CV (no Q: WR,SP)
340.51	1.25	Free Outfall		O2,CV (no Q: WR,SP)
340.52	1.26	Free Outfall		O2,CV (no Q: WR,SP)
340.53	1.26	Free Outfall		O2,CV (no Q: WR,SP)
340.54	1.27	Free Outfall		O2,CV (no Q: WR,SP)
340.55	1.27	Free Outfall		O2,CV (no Q: WR,SP)
340.56	1.28	Free Outfall		O2,CV (no Q: WR,SP)
340.57	1.28	Free Outfall		O2,CV (no Q: WR,SP)
340.58	1.29	Free Outfall		O2,CV (no Q: WR,SP)
340.59	1.29	Free Outfall		O2,CV (no Q: WR,SP)
340.60	1.29	Free Outfall		O2,CV (no Q: WR,SP)
340.61	1.30	Free Outfall		O2,CV (no Q: WR,SP)
340.62	1.30	Free Outfall		O2,CV (no Q: WR,SP)
340.63	1.31	Free Outfall		O2,CV (no Q: WR,SP)
340.64	1.31	Free Outfall		O2,CV (no Q: WR,SP)
340.65	1.31	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
340.66	1.32	Free Outfall		O2,CV (no Q: WR,SP)
340.67	1.32	Free Outfall		O2,CV (no Q: WR,SP)
340.68	1.33	Free Outfall		O2,CV (no Q: WR,SP)
340.69	1.33	Free Outfall		O2,CV (no Q: WR,SP)
340.70	1.34	Free Outfall		O2,CV (no Q: WR,SP)
340.71	1.34	Free Outfall		O2,CV (no Q: WR,SP)
340.72	1.34	Free Outfall		O2,CV (no Q: WR,SP)
340.73	1.35	Free Outfall		O2,CV (no Q: WR,SP)
340.74	1.35	Free Outfall		O2,CV (no Q: WR,SP)
340.75	1.36	Free Outfall		O2,CV (no Q: WR,SP)
340.76	1.36	Free Outfall		O2,CV (no Q: WR,SP)
340.77	1.36	Free Outfall		O2,CV (no Q: WR,SP)
340.78	1.37	Free Outfall		O2,CV (no Q: WR,SP)
340.79	1.37	Free Outfall		O2,CV (no Q: WR,SP)
340.80	1.38	Free Outfall		O2,CV (no Q: WR,SP)
340.81	1.38	Free Outfall		O2,CV (no Q: WR,SP)
340.82	1.38	Free Outfall		O2,CV (no Q: WR,SP)
340.83	1.39	Free Outfall		O2,CV (no Q: WR,SP)
340.84	1.39	Free Outfall		O2,CV (no Q: WR,SP)
340.85	1.40	Free Outfall		O2,CV (no Q: WR,SP)
340.86	1.40	Free Outfall		O2,CV (no Q: WR,SP)
340.87	1.40	Free Outfall		O2,CV (no Q: WR,SP)
340.88	1.41	Free Outfall		O2,CV (no Q: WR,SP)
340.89	1.41	Free Outfall		O2,CV (no Q: WR,SP)
340.90	1.42	Free Outfall		O2,CV (no Q: WR,SP)
340.91	1.42	Free Outfall		O2,CV (no Q: WR,SP)
340.92	1.42	Free Outfall		O2,CV (no Q: WR,SP)
340.93	1.43	Free Outfall		O2,CV (no Q: WR,SP)
340.94	1.43	Free Outfall		O2,CV (no Q: WR,SP)
340.95	1.44	Free Outfall		O2,CV (no Q: WR,SP)
340.96	1.44	Free Outfall		O2,CV (no Q: WR,SP)
340.97	1.44	Free Outfall		O2,CV (no Q: WR,SP)
340.98	1.45	Free Outfall		O2,CV (no Q: WR,SP)
340.99	1.45	Free Outfall		O2,CV (no Q: WR,SP)
341.00	1.45	Free Outfall		O2,CV (no Q: WR,SP)
341.01	1.46	Free Outfall		O2,CV (no Q: WR,SP)
341.02	1.46	Free Outfall		O2,CV (no Q: WR,SP)
341.03	1.47	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.04	1.47	Free Outfall		O2,CV (no Q: WR,SP)
341.05	1.47	Free Outfall		O2,CV (no Q: WR,SP)
341.06	1.48	Free Outfall		O2,CV (no Q: WR,SP)
341.07	1.48	Free Outfall		O2,CV (no Q: WR,SP)
341.08	1.48	Free Outfall		O2,CV (no Q: WR,SP)
341.09	1.49	Free Outfall		O2,CV (no Q: WR,SP)
341.10	1.49	Free Outfall		O2,CV (no Q: WR,SP)
341.11	1.50	Free Outfall		O2,CV (no Q: WR,SP)
341.12	1.50	Free Outfall		O2,CV (no Q: WR,SP)
341.13	1.50	Free Outfall		O2,CV (no Q: WR,SP)
341.14	1.51	Free Outfall		O2,CV (no Q: WR,SP)
341.15	1.51	Free Outfall		O2,CV (no Q: WR,SP)
341.16	1.51	Free Outfall		O2,CV (no Q: WR,SP)
341.17	1.52	Free Outfall		O2,CV (no Q: WR,SP)
341.18	1.52	Free Outfall		O2,CV (no Q: WR,SP)
341.19	1.52	Free Outfall		O2,CV (no Q: WR,SP)
341.20	1.53	Free Outfall		O2,CV (no Q: WR,SP)
341.21	1.53	Free Outfall		O2,CV (no Q: WR,SP)
341.22	1.54	Free Outfall		O2,CV (no Q: WR,SP)
341.23	1.54	Free Outfall		O2,CV (no Q: WR,SP)
341.24	1.54	Free Outfall		O2,CV (no Q: WR,SP)
341.25	1.55	Free Outfall		O2,CV (no Q: WR,SP)
341.26	1.55	Free Outfall		O2,CV (no Q: WR,SP)
341.27	1.55	Free Outfall		O2,CV (no Q: WR,SP)
341.28	1.56	Free Outfall		O2,CV (no Q: WR,SP)
341.29	1.56	Free Outfall		O2,CV (no Q: WR,SP)
341.30	1.56	Free Outfall		O2,CV (no Q: WR,SP)
341.31	1.57	Free Outfall		O2,CV (no Q: WR,SP)
341.32	1.57	Free Outfall		O2,CV (no Q: WR,SP)
341.33	1.57	Free Outfall		O2,CV (no Q: WR,SP)
341.34	1.58	Free Outfall		O2,CV (no Q: WR,SP)
341.35	1.58	Free Outfall		O2,CV (no Q: WR,SP)
341.36	1.58	Free Outfall		O2,CV (no Q: WR,SP)
341.37	1.59	Free Outfall		O2,CV (no Q: WR,SP)
341.38	1.59	Free Outfall		O2,CV (no Q: WR,SP)
341.39	1.60	Free Outfall		O2,CV (no Q: WR,SP)
341.40	1.60	Free Outfall		O2,CV (no Q: WR,SP)
341.41	1.60	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q, Q cfs	Converge, TW Elev ft	Converge, Error +/-ft	Notes, Contributing Structures
341.42	1.61	Free Outfall		O2,CV (no Q: WR,SP)
341.43	1.61	Free Outfall		O2,CV (no Q: WR,SP)
341.44	1.61	Free Outfall		O2,CV (no Q: WR,SP)
341.45	1.62	Free Outfall		O2,CV (no Q: WR,SP)
341.46	1.62	Free Outfall		O2,CV (no Q: WR,SP)
341.47	1.62	Free Outfall		O2,CV (no Q: WR,SP)
341.48	1.63	Free Outfall		O2,CV (no Q: WR,SP)
341.49	1.63	Free Outfall		O2,CV (no Q: WR,SP)
341.50	1.63	Free Outfall		O2,CV (no Q: WR,SP)
341.51	1.64	Free Outfall		O2,CV (no Q: WR,SP)
341.52	1.64	Free Outfall		O2,CV (no Q: WR,SP)
341.53	1.64	Free Outfall		O2,CV (no Q: WR,SP)
341.54	1.65	Free Outfall		O2,CV (no Q: WR,SP)
341.55	1.65	Free Outfall		O2,CV (no Q: WR,SP)
341.56	1.65	Free Outfall		O2,CV (no Q: WR,SP)
341.57	1.66	Free Outfall		O2,CV (no Q: WR,SP)
341.58	1.66	Free Outfall		O2,CV (no Q: WR,SP)
341.59	1.66	Free Outfall		O2,CV (no Q: WR,SP)
341.60	1.67	Free Outfall		O2,CV (no Q: WR,SP)
341.61	1.67	Free Outfall		O2,CV (no Q: WR,SP)
341.62	1.67	Free Outfall		O2,CV (no Q: WR,SP)
341.63	1.68	Free Outfall		O2,CV (no Q: WR,SP)
341.64	1.68	Free Outfall		O2,CV (no Q: WR,SP)
341.65	1.68	Free Outfall		O2,CV (no Q: WR,SP)
341.66	1.69	Free Outfall		O2,CV (no Q: WR,SP)
341.67	1.69	Free Outfall		O2,CV (no Q: WR,SP)
341.68	1.69	Free Outfall		O2,CV (no Q: WR,SP)
341.69	1.70	Free Outfall		O2,CV (no Q: WR,SP)
341.70	1.70	Free Outfall		O2,CV (no Q: WR,SP)
341.71	1.70	Free Outfall		O2,CV (no Q: WR,SP)
341.72	1.71	Free Outfall		O2,CV (no Q: WR,SP)
341.73	1.71	Free Outfall		O2,CV (no Q: WR,SP)
341.74	1.71	Free Outfall		O2,CV (no Q: WR,SP)
341.75	1.72	Free Outfall		O2,CV (no Q: WR,SP)
341.76	1.72	Free Outfall		O2,CV (no Q: WR,SP)
341.77	1.72	Free Outfall		O2,CV (no Q: WR,SP)
341.78	1.73	Free Outfall		O2,CV (no Q: WR,SP)
341.79	1.73	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
341.80	1.73	Free Outfall		O2,CV (no Q: WR,SP)
341.81	1.73	Free Outfall		O2,CV (no Q: WR,SP)
341.82	1.74	Free Outfall		O2,CV (no Q: WR,SP)
341.83	1.74	Free Outfall		O2,CV (no Q: WR,SP)
341.84	1.74	Free Outfall		O2,CV (no Q: WR,SP)
341.85	1.75	Free Outfall		O2,CV (no Q: WR,SP)
341.86	1.75	Free Outfall		O2,CV (no Q: WR,SP)
341.87	1.75	Free Outfall		O2,CV (no Q: WR,SP)
341.88	1.76	Free Outfall		O2,CV (no Q: WR,SP)
341.89	1.76	Free Outfall		O2,CV (no Q: WR,SP)
341.90	1.76	Free Outfall		O2,CV (no Q: WR,SP)
341.91	1.77	Free Outfall		O2,CV (no Q: WR,SP)
341.92	1.77	Free Outfall		O2,CV (no Q: WR,SP)
341.93	1.77	Free Outfall		O2,CV (no Q: WR,SP)
341.94	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.95	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.96	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.97	1.78	Free Outfall		O2,CV (no Q: WR,SP)
341.98	1.79	Free Outfall		O2,CV (no Q: WR,SP)
341.99	1.79	Free Outfall		O2,CV (no Q: WR,SP)
342.00	1.79	Free Outfall		O2,CV (no Q: WR,SP)
342.01	1.80	Free Outfall		O2,CV (no Q: WR,SP)
342.02	1.80	Free Outfall		O2,CV (no Q: WR,SP)
342.03	1.80	Free Outfall		O2,CV (no Q: WR,SP)
342.04	1.81	Free Outfall		O2,CV (no Q: WR,SP)
342.05	1.81	Free Outfall		O2,CV (no Q: WR,SP)
342.06	1.81	Free Outfall		O2,CV (no Q: WR,SP)
342.07	1.82	Free Outfall		O2,CV (no Q: WR,SP)
342.08	1.82	Free Outfall		O2,CV (no Q: WR,SP)
342.09	1.82	Free Outfall		O2,CV (no Q: WR,SP)
342.10	1.82	Free Outfall		O2,CV (no Q: WR,SP)
342.11	1.83	Free Outfall		O2,CV (no Q: WR,SP)
342.12	1.83	Free Outfall		O2,CV (no Q: WR,SP)
342.13	1.83	Free Outfall		O2,CV (no Q: WR,SP)
342.14	1.84	Free Outfall		O2,CV (no Q: WR,SP)
342.15	1.84	Free Outfall		O2,CV (no Q: WR,SP)
342.16	1.84	Free Outfall		O2,CV (no Q: WR,SP)
342.17	1.85	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.18	1.85	Free Outfall		O2,CV (no Q: WR,SP)
342.19	1.85	Free Outfall		O2,CV (no Q: WR,SP)
342.20	1.85	Free Outfall		O2,CV (no Q: WR,SP)
342.21	1.86	Free Outfall		O2,CV (no Q: WR,SP)
342.22	1.86	Free Outfall		O2,CV (no Q: WR,SP)
342.23	1.86	Free Outfall		O2,CV (no Q: WR,SP)
342.24	1.87	Free Outfall		O2,CV (no Q: WR,SP)
342.25	1.87	Free Outfall		O2,CV (no Q: WR,SP)
342.26	1.87	Free Outfall		O2,CV (no Q: WR,SP)
342.27	1.88	Free Outfall		O2,CV (no Q: WR,SP)
342.28	1.88	Free Outfall		O2,CV (no Q: WR,SP)
342.29	1.88	Free Outfall		O2,CV (no Q: WR,SP)
342.30	1.88	Free Outfall		O2,CV (no Q: WR,SP)
342.31	1.89	Free Outfall		O2,CV (no Q: WR,SP)
342.32	1.89	Free Outfall		O2,CV (no Q: WR,SP)
342.33	1.89	Free Outfall		O2,CV (no Q: WR,SP)
342.34	1.90	Free Outfall		O2,CV (no Q: WR,SP)
342.35	1.90	Free Outfall		O2,CV (no Q: WR,SP)
342.36	1.90	Free Outfall		O2,CV (no Q: WR,SP)
342.37	1.90	Free Outfall		O2,CV (no Q: WR,SP)
342.38	1.91	Free Outfall		O2,CV (no Q: WR,SP)
342.39	1.91	Free Outfall		O2,CV (no Q: WR,SP)
342.40	1.91	Free Outfall		O2,CV (no Q: WR,SP)
342.41	1.92	Free Outfall		O2,CV (no Q: WR,SP)
342.42	1.92	Free Outfall		O2,CV (no Q: WR,SP)
342.43	1.92	Free Outfall		O2,CV (no Q: WR,SP)
342.44	1.92	Free Outfall		O2,CV (no Q: WR,SP)
342.45	1.93	Free Outfall		O2,CV (no Q: WR,SP)
342.46	1.93	Free Outfall		O2,CV (no Q: WR,SP)
342.47	1.93	Free Outfall		O2,CV (no Q: WR,SP)
342.48	1.94	Free Outfall		O2,CV (no Q: WR,SP)
342.49	1.94	Free Outfall		O2,CV (no Q: WR,SP)
342.50	1.94	Free Outfall		O2,CV (no Q: WR,SP)
342.51	1.94	Free Outfall		O2,CV (no Q: WR,SP)
342.52	1.95	Free Outfall		O2,CV (no Q: WR,SP)
342.53	1.95	Free Outfall		O2,CV (no Q: WR,SP)
342.54	1.95	Free Outfall		O2,CV (no Q: WR,SP)
342.55	1.96	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.56	1.96	Free Outfall		O2,CV (no Q: WR,SP)
342.57	1.96	Free Outfall		O2,CV (no Q: WR,SP)
342.58	1.96	Free Outfall		O2,CV (no Q: WR,SP)
342.59	1.97	Free Outfall		O2,CV (no Q: WR,SP)
342.60	1.97	Free Outfall		O2,CV (no Q: WR,SP)
342.61	1.97	Free Outfall		O2,CV (no Q: WR,SP)
342.62	1.98	Free Outfall		O2,CV (no Q: WR,SP)
342.63	1.98	Free Outfall		O2,CV (no Q: WR,SP)
342.64	1.98	Free Outfall		O2,CV (no Q: WR,SP)
342.65	1.98	Free Outfall		O2,CV (no Q: WR,SP)
342.66	1.99	Free Outfall		O2,CV (no Q: WR,SP)
342.67	1.99	Free Outfall		O2,CV (no Q: WR,SP)
342.68	1.99	Free Outfall		O2,CV (no Q: WR,SP)
342.69	1.99	Free Outfall		O2,CV (no Q: WR,SP)
342.70	2.00	Free Outfall		O2,CV (no Q: WR,SP)
342.71	2.00	Free Outfall		O2,CV (no Q: WR,SP)
342.72	2.00	Free Outfall		O2,CV (no Q: WR,SP)
342.73	2.01	Free Outfall		O2,CV (no Q: WR,SP)
342.74	2.01	Free Outfall		O2,CV (no Q: WR,SP)
342.75	2.01	Free Outfall		O2,CV (no Q: WR,SP)
342.76	2.01	Free Outfall		O2,CV (no Q: WR,SP)
342.77	2.02	Free Outfall		O2,CV (no Q: WR,SP)
342.78	2.02	Free Outfall		O2,CV (no Q: WR,SP)
342.79	2.02	Free Outfall		O2,CV (no Q: WR,SP)
342.80	2.03	Free Outfall		O2,CV (no Q: WR,SP)
342.81	2.03	Free Outfall		O2,CV (no Q: WR,SP)
342.82	2.03	Free Outfall		O2,CV (no Q: WR,SP)
342.83	2.03	Free Outfall		O2,CV (no Q: WR,SP)
342.84	2.04	Free Outfall		O2,CV (no Q: WR,SP)
342.85	2.04	Free Outfall		O2,CV (no Q: WR,SP)
342.86	2.04	Free Outfall		O2,CV (no Q: WR,SP)
342.87	2.04	Free Outfall		O2,CV (no Q: WR,SP)
342.88	2.05	Free Outfall		O2,CV (no Q: WR,SP)
342.89	2.05	Free Outfall		O2,CV (no Q: WR,SP)
342.90	2.05	Free Outfall		O2,CV (no Q: WR,SP)
342.91	2.05	Free Outfall		O2,CV (no Q: WR,SP)
342.92	2.06	Free Outfall		O2,CV (no Q: WR,SP)
342.93	2.06	Free Outfall		O2,CV (no Q: WR,SP)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
342.94	2.06	Free Outfall		O2,CV (no Q: WR,SP)
342.95	2.07	Free Outfall		O2,CV (no Q: WR,SP)
342.96	2.07	Free Outfall		O2,CV (no Q: WR,SP)
342.97	2.07	Free Outfall		O2,CV (no Q: WR,SP)
342.98	2.07	Free Outfall		O2,CV (no Q: WR,SP)
342.99	2.08	Free Outfall		O2,CV (no Q: WR,SP)
343.00	2.08	Free Outfall		O2,CV (no Q: WR,SP)
343.01	2.13	Free Outfall		SP,O2,CV (no Q: WR)
343.02	2.22	Free Outfall		SP,O2,CV (no Q: WR)
343.03	2.33	Free Outfall		SP,O2,CV (no Q: WR)
343.04	2.47	Free Outfall		SP,O2,CV (no Q: WR)
343.05	2.62	Free Outfall		SP,O2,CV (no Q: WR)
343.06	2.79	Free Outfall		SP,O2,CV (no Q: WR)
343.07	2.97	Free Outfall		SP,O2,CV (no Q: WR)
343.08	3.17	Free Outfall		SP,O2,CV (no Q: WR)
343.09	3.37	Free Outfall		SP,O2,CV (no Q: WR)
343.10	3.60	Free Outfall		SP,O2,CV (no Q: WR)
343.11	3.83	Free Outfall		SP,O2,CV (no Q: WR)
343.12	4.06	Free Outfall		SP,O2,CV (no Q: WR)
343.13	4.31	Free Outfall		SP,O2,CV (no Q: WR)
343.14	4.56	Free Outfall		SP,O2,CV (no Q: WR)
343.15	4.82	Free Outfall		SP,O2,CV (no Q: WR)
343.16	5.09	Free Outfall		SP,O2,CV (no Q: WR)
343.17	5.37	Free Outfall		SP,O2,CV (no Q: WR)
343.18	5.66	Free Outfall		SP,O2,CV (no Q: WR)
343.19	5.96	Free Outfall		SP,O2,CV (no Q: WR)
343.20	6.26	Free Outfall		SP,O2,CV (no Q: WR)
343.21	6.58	Free Outfall		SP,O2,CV (no Q: WR)
343.22	6.90	Free Outfall		SP,O2,CV (no Q: WR)
343.23	7.23	Free Outfall		SP,O2,CV (no Q: WR)
343.24	7.56	Free Outfall		SP,O2,CV (no Q: WR)
343.25	7.90	Free Outfall		SP,O2,CV (no Q: WR)
343.26	8.25	Free Outfall		SP,O2,CV (no Q: WR)
343.27	8.61	Free Outfall		SP,O2,CV (no Q: WR)
343.28	8.97	Free Outfall		SP,O2,CV (no Q: WR)
343.29	9.34	Free Outfall		SP,O2,CV (no Q: WR)
343.30	9.72	Free Outfall		SP,O2,CV (no Q: WR)
343.31	10.10	Free Outfall		SP,O2,CV (no Q: WR)

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.32	10.49	Free Outfall		SP,O2,CV (no Q: WR)
343.33	10.88	Free Outfall		SP,O2,CV (no Q: WR)
343.34	11.29	Free Outfall		SP,O2,CV (no Q: WR)
343.35	11.69	Free Outfall		SP,O2,CV (no Q: WR)
343.36	12.10	Free Outfall		SP,O2,CV (no Q: WR)
343.37	12.52	Free Outfall		SP,O2,CV (no Q: WR)
343.38	12.95	Free Outfall		SP,O2,CV (no Q: WR)
343.39	13.38	Free Outfall		SP,O2,CV (no Q: WR)
343.40	13.81	Free Outfall		SP,O2,CV (no Q: WR)
343.41	14.25	Free Outfall		SP,O2,CV (no Q: WR)
343.42	14.70	Free Outfall		SP,O2,CV (no Q: WR)
343.43	15.15	Free Outfall		SP,O2,CV (no Q: WR)
343.44	15.61	Free Outfall		SP,O2,CV (no Q: WR)
343.45	16.07	Free Outfall		SP,O2,CV (no Q: WR)
343.46	16.54	Free Outfall		SP,O2,CV (no Q: WR)
343.47	17.01	Free Outfall		SP,O2,CV (no Q: WR)
343.48	17.49	Free Outfall		SP,O2,CV (no Q: WR)
343.49	17.97	Free Outfall		SP,O2,CV (no Q: WR)
343.50	18.45	Free Outfall		SP,O2,CV (no Q: WR)
343.51	19.03	Free Outfall		WR,SP,O2,CV
343.52	19.69	Free Outfall		WR,SP,O2,CV
343.53	20.41	Free Outfall		WR,SP,O2,CV
343.54	21.16	Free Outfall		WR,SP,O2,CV
343.55	21.96	Free Outfall		WR,SP,O2,CV
343.56	22.79	Free Outfall		WR,SP,O2,CV
343.57	23.65	Free Outfall		WR,SP,O2,CV
343.58	24.54	Free Outfall		WR,SP,O2,CV
343.59	25.46	Free Outfall		WR,SP,O2,CV
343.60	26.40	Free Outfall		WR,SP,O2,CV
343.61	27.37	Free Outfall		WR,SP,O2,CV
343.62	28.37	Free Outfall		WR,SP,O2,CV
343.63	29.38	Free Outfall		WR,SP,O2,CV
343.64	30.41	Free Outfall		WR,SP,O2,CV
343.65	31.46	Free Outfall		WR,SP,O2,CV
343.66	32.54	Free Outfall		WR,SP,O2,CV
343.67	33.63	Free Outfall		WR,SP,O2,CV
343.68	34.74	Free Outfall		WR,SP,O2,CV
343.69	35.87	Free Outfall		WR,SP,O2,CV

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***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
343.70	37.02	Free Outfall		WR,SP,O2,CV
343.71	38.18	Free Outfall		WR,SP,O2,CV
343.72	39.37	Free Outfall		WR,SP,O2,CV
343.73	40.57	Free Outfall		WR,SP,O2,CV
343.74	41.78	Free Outfall		WR,SP,O2,CV
343.75	43.02	Free Outfall		WR,SP,O2,CV
343.76	44.26	Free Outfall		WR,SP,O2,CV
343.77	45.52	Free Outfall		WR,SP,O2,CV
343.78	46.80	Free Outfall		WR,SP,O2,CV
343.79	48.09	Free Outfall		WR,SP,O2,CV
343.80	49.38	Free Outfall		WR,SP,CV (no Q: O2)
343.81	50.69	Free Outfall		WR,SP,CV (no Q: O2)
343.82	52.01	Free Outfall		WR,SP,CV (no Q: O2)
343.83	53.34	Free Outfall		WR,SP,CV (no Q: O2)
343.84	54.68	Free Outfall		WR,SP,CV (no Q: O2)
343.85	56.03	Free Outfall		WR,SP,CV (no Q: O2)
343.86	57.95	Free Outfall		WR,SP,CV (no Q: O2)
343.87	58.81	Free Outfall		WR,SP,CV (no Q: O2)
343.88	59.68	Free Outfall		WR,SP,CV (no Q: O2)
343.89	60.56	Free Outfall		WR,SP,CV (no Q: O2)
343.90	61.44	Free Outfall		WR,SP,CV (no Q: O2)
343.91	62.35	Free Outfall		WR,SP,CV (no Q: O2)
343.92	63.26	Free Outfall		WR,SP,CV (no Q: O2)
343.93	64.18	Free Outfall		WR,SP,CV (no Q: O2)
343.94	65.12	Free Outfall		WR,SP,CV (no Q: O2)
343.95	66.05	Free Outfall		WR,SP,CV (no Q: O2)
343.96	67.00	Free Outfall		WR,SP,CV (no Q: O2)
343.97	67.96	Free Outfall		WR,SP,CV (no Q: O2)
343.98	68.94	Free Outfall		WR,SP,CV (no Q: O2)
343.99	69.91	Free Outfall		WR,SP,CV (no Q: O2)
344.00	70.91	Free Outfall		WR,SP,CV (no Q: O2)
344.01	71.91	Free Outfall		WR,SP,CV (no Q: O2)
344.02	72.91	Free Outfall		WR,SP,CV (no Q: O2)
344.03	73.93	Free Outfall		WR,SP,CV (no Q: O2)
344.04	74.95	Free Outfall		WR,SP,CV (no Q: O2)
344.05	76.00	Free Outfall		WR,SP,CV (no Q: O2)
344.06	77.04	Free Outfall		WR,SP,CV (no Q: O2)
344.07	78.09	Free Outfall		WR,SP,CV (no Q: O2)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.08	79.15	Free Outfall		WR,SP,CV (no Q: O2)
344.09	80.23	Free Outfall		WR,SP,CV (no Q: O2)
344.10	81.31	Free Outfall		WR,SP,CV (no Q: O2)
344.11	82.39	Free Outfall		WR,SP,CV (no Q: O2)
344.12	83.49	Free Outfall		WR,SP,CV (no Q: O2)
344.13	84.60	Free Outfall		WR,SP,CV (no Q: O2)
344.14	85.71	Free Outfall		WR,SP,CV (no Q: O2)
344.15	86.83	Free Outfall		WR,SP,CV (no Q: O2)
344.16	87.95	Free Outfall		WR,SP,CV (no Q: O2)
344.17	89.09	Free Outfall		WR,SP,CV (no Q: O2)
344.18	90.23	Free Outfall		WR,SP,CV (no Q: O2)
344.19	91.39	Free Outfall		WR,SP,CV (no Q: O2)
344.20	92.55	Free Outfall		WR,SP,CV (no Q: O2)
344.21	93.72	Free Outfall		WR,SP,CV (no Q: O2)
344.22	94.90	Free Outfall		WR,SP,CV (no Q: O2)
344.23	96.08	Free Outfall		WR,SP,CV (no Q: O2)
344.24	97.27	Free Outfall		WR,SP,CV (no Q: O2)
344.25	98.48	Free Outfall		WR,SP,CV (no Q: O2)
344.26	99.69	Free Outfall		WR,SP,CV (no Q: O2)
344.27	100.90	Free Outfall		WR,SP,CV (no Q: O2)
344.28	102.12	Free Outfall		WR,SP,CV (no Q: O2)
344.29	103.35	Free Outfall		WR,SP,CV (no Q: O2)
344.30	104.59	Free Outfall		WR,SP,CV (no Q: O2)
344.31	105.83	Free Outfall		WR,SP,CV (no Q: O2)
344.32	107.09	Free Outfall		WR,SP,CV (no Q: O2)
344.33	108.34	Free Outfall		WR,SP,CV (no Q: O2)
344.34	109.61	Free Outfall		WR,SP,CV (no Q: O2)
344.35	110.89	Free Outfall		WR,SP,CV (no Q: O2)
344.36	112.16	Free Outfall		WR,SP,CV (no Q: O2)
344.37	113.45	Free Outfall		WR,SP,CV (no Q: O2)
344.38	114.74	Free Outfall		WR,SP,CV (no Q: O2)
344.39	116.04	Free Outfall		WR,SP,CV (no Q: O2)
344.40	117.35	Free Outfall		WR,SP,CV (no Q: O2)
344.41	118.66	Free Outfall		WR,SP,CV (no Q: O2)
344.42	119.98	Free Outfall		WR,SP,CV (no Q: O2)
344.43	121.31	Free Outfall		WR,SP,CV (no Q: O2)
344.44	122.65	Free Outfall		WR,SP,CV (no Q: O2)
344.45	124.00	Free Outfall		WR,SP,CV (no Q: O2)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Total Q		Notes		
		Converge		
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
344.46	125.34	Free Outfall		WR,SP,CV (no Q: O2)
344.47	126.70	Free Outfall		WR,SP,CV (no Q: O2)
344.48	128.06	Free Outfall		WR,SP,CV (no Q: O2)
344.49	129.43	Free Outfall		WR,SP,CV (no Q: O2)
344.50	130.81	Free Outfall		WR,SP,CV (no Q: O2)
344.51	132.18	Free Outfall		WR,SP,CV (no Q: O2)
344.52	133.57	Free Outfall		WR,SP,CV (no Q: O2)
344.53	134.97	Free Outfall		WR,SP,CV (no Q: O2)
344.54	136.37	Free Outfall		WR,SP,CV (no Q: O2)
344.55	137.77	Free Outfall		WR,SP,CV (no Q: O2)
344.56	139.19	Free Outfall		WR,SP,CV (no Q: O2)
344.57	140.61	Free Outfall		WR,SP,CV (no Q: O2)
344.58	142.03	Free Outfall		WR,SP,CV (no Q: O2)
344.59	143.47	Free Outfall		WR,SP,CV (no Q: O2)
344.60	144.91	Free Outfall		WR,SP,CV (no Q: O2)
344.61	146.35	Free Outfall		WR,SP,CV (no Q: O2)
344.62	147.80	Free Outfall		WR,SP,CV (no Q: O2)
344.63	149.26	Free Outfall		WR,SP,CV (no Q: O2)
344.64	150.71	Free Outfall		WR,SP,CV (no Q: O2)
344.65	152.18	Free Outfall		WR,SP,CV (no Q: O2)
344.66	153.65	Free Outfall		WR,SP,CV (no Q: O2)
344.67	155.14	Free Outfall		WR,SP,CV (no Q: O2)
344.68	156.63	Free Outfall		WR,SP,CV (no Q: O2)
344.69	158.12	Free Outfall		WR,SP,CV (no Q: O2)
344.70	159.62	Free Outfall		WR,SP,CV (no Q: O2)
344.71	161.13	Free Outfall		WR,SP,CV (no Q: O2)
344.72	162.64	Free Outfall		WR,SP,CV (no Q: O2)
344.73	164.15	Free Outfall		WR,SP,CV (no Q: O2)
344.74	165.67	Free Outfall		WR,SP,CV (no Q: O2)
344.75	167.20	Free Outfall		WR,SP,CV (no Q: O2)
344.76	168.74	Free Outfall		WR,SP,CV (no Q: O2)
344.77	170.27	Free Outfall		WR,SP,CV (no Q: O2)
344.78	171.82	Free Outfall		WR,SP,CV (no Q: O2)
344.79	173.38	Free Outfall		WR,SP,CV (no Q: O2)
344.80	174.93	Free Outfall		WR,SP,CV (no Q: O2)
344.81	176.50	Free Outfall		WR,SP,CV (no Q: O2)
344.82	178.06	Free Outfall		WR,SP,CV (no Q: O2)
344.83	179.63	Free Outfall		WR,SP,CV (no Q: O2)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

***** COMPOSITE OUTFLOW SUMMARY ****

WS Elev, Elev. ft	Total Q Q cfs	Notes: Converge TW Elev ft	Converge Error +/-ft	Contributing Structures	
344.84	181.22	Free Outfall		WR,SP,CV	(no Q: O2)
344.85	182.80	Free Outfall		WR,SP,CV	(no Q: O2)
344.86	184.39	Free Outfall		WR,SP,CV	(no Q: O2)
344.87	185.99	Free Outfall		WR,SP,CV	(no Q: O2)
344.88	187.59	Free Outfall		WR,SP,CV	(no Q: O2)
344.89	189.20	Free Outfall		WR,SP,CV	(no Q: O2)
344.90	190.80	Free Outfall		WR,SP,CV	(no Q: O2)
344.91	192.42	Free Outfall		WR,SP,CV	(no Q: O2)
344.92	194.05	Free Outfall		WR,SP,CV	(no Q: O2)
344.93	195.67	Free Outfall		WR,SP,CV	(no Q: O2)
344.94	197.31	Free Outfall		WR,SP,CV	(no Q: O2)
344.95	198.94	Free Outfall		WR,SP,CV	(no Q: O2)
344.96	200.58	Free Outfall		WR,SP,CV	(no Q: O2)
344.97	202.22	Free Outfall		WR,SP,CV	(no Q: O2)
344.98	203.88	Free Outfall		WR,SP,CV	(no Q: O2)
344.99	205.54	Free Outfall		WR,SP,CV	(no Q: O2)
345.00	207.20	Free Outfall		WR,SP,CV	(no Q: O2)

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.00	.00	.000	.8000	.00	.00	.00
338.01	.00	.008	.8006	.00	.00	19.39
338.02	.00	.016	.8013	.00	.00	38.73
338.03	.00	.024	.8019	.00	.00	58.15
338.04	.00	.032	.8026	.00	.00	77.58
338.05	.00	.040	.8032	.00	.00	96.97
338.06	.00	.048	.8039	.00	.00	116.44
338.07	.00	.056	.8045	.00	.00	135.92
338.08	.00	.064	.8052	.00	.00	155.36
338.09	.00	.072	.8058	.00	.00	174.87
338.10	.00	.080	.8065	.00	.00	194.40
338.11	.00	.088	.8071	.00	.00	213.88
338.12	.02	.096	.8078	.00	.02	233.46
338.13	.02	.105	.8084	.00	.02	253.04
338.14	.02	.113	.8091	.00	.02	272.63
338.15	.02	.121	.8097	.00	.02	292.17
338.16	.05	.129	.8104	.00	.05	311.83
338.17	.12	.137	.8110	.00	.12	331.54
338.18	.12	.145	.8117	.00	.12	351.13
338.19	.15	.153	.8124	.00	.15	370.83

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.20	.17	.161	.8130	.00	.17	390.54
338.21	.17	.169	.8137	.00	.17	410.18
338.22	.17	.178	.8143	.00	.17	429.90
338.23	.17	.186	.8150	.00	.17	449.63
338.24	.24	.194	.8156	.00	.24	469.38
338.25	.24	.202	.8163	.00	.24	489.16
338.26	.25	.210	.8169	.00	.25	508.94
338.27	.25	.218	.8176	.00	.25	528.68
338.28	.25	.227	.8182	.00	.25	548.50
338.29	.26	.235	.8189	.00	.26	568.34
338.30	.26	.243	.8195	.00	.26	588.12
338.31	.26	.251	.8202	.00	.26	607.98
338.32	.39	.259	.8209	.00	.39	627.99
338.33	.39	.268	.8215	.00	.39	647.82
338.34	.39	.276	.8222	.00	.39	667.73
338.35	.39	.284	.8228	.00	.39	687.65
338.36	.42	.292	.8235	.00	.42	707.56
338.37	.43	.300	.8241	.00	.43	727.52
338.38	.53	.309	.8248	.00	.53	747.60
338.39	.53	.317	.8255	.00	.53	767.58

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.40	.55	.325	.8261	.00	.55	787.55
338.41	.55	.333	.8268	.00	.55	807.57
338.42	.57	.342	.8274	.00	.57	827.62
338.43	.58	.350	.8281	.00	.58	847.62
338.44	.58	.358	.8287	.00	.58	867.69
338.45	.58	.367	.8294	.00	.58	887.77
338.46	.58	.375	.8301	.00	.58	907.81
338.47	.58	.383	.8307	.00	.58	927.92
338.48	.58	.392	.8314	.00	.58	948.06
338.49	.58	.400	.8320	.00	.58	968.14
338.50	.58	.408	.8327	.00	.58	988.31
338.51	.58	.416	.8334	.00	.58	1008.48
338.52	.58	.425	.8340	.00	.58	1028.62
338.53	.58	.433	.8347	.00	.58	1048.83
338.54	.58	.442	.8354	.00	.58	1069.06
338.55	.58	.450	.8360	.00	.58	1089.24
338.56	.58	.458	.8367	.00	.58	1109.50
338.57	.58	.467	.8373	.00	.58	1129.77
338.58	.58	.475	.8380	.00	.58	1150.00
338.59	.58	.483	.8387	.00	.58	1170.31

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.60	.58	.492	.8393	.00	.58	1190.63
338.61	.58	.500	.8400	.00	.58	1210.91
338.62	.58	.509	.8407	.00	.58	1231.27
338.63	.58	.517	.8413	.00	.58	1251.64
338.64	.58	.525	.8420	.00	.58	1272.03
338.65	.58	.534	.8426	.00	.58	1292.37
338.66	.58	.542	.8433	.00	.58	1312.79
338.67	.58	.551	.8440	.00	.58	1333.22
338.68	.58	.559	.8446	.00	.58	1353.61
338.69	.58	.568	.8453	.00	.58	1374.08
338.70	.58	.576	.8460	.00	.58	1394.57
338.71	.58	.584	.8466	.00	.58	1415.00
338.72	.58	.593	.8473	.00	.58	1435.52
338.73	.58	.601	.8480	.00	.58	1456.05
338.74	.59	.610	.8486	.00	.59	1476.54
338.75	.59	.618	.8493	.00	.59	1497.12
338.76	.60	.627	.8500	.00	.60	1517.70
338.77	.60	.635	.8506	.00	.60	1538.24
338.78	.61	.644	.8513	.00	.61	1558.86
338.79	.61	.652	.8520	.00	.61	1579.49

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.80	.62	.661	.8526	.00	.62	1600.08
338.81	.63	.669	.8533	.00	.63	1620.75
338.82	.63	.678	.8540	.00	.63	1641.44
338.83	.64	.687	.8547	.00	.64	1662.08
338.84	.65	.695	.8553	.00	.65	1682.80
338.85	.65	.704	.8560	.00	.65	1703.53
338.86	.66	.712	.8567	.00	.66	1724.22
338.87	.67	.721	.8573	.00	.67	1744.98
338.88	.68	.729	.8580	.00	.68	1765.77
338.89	.68	.738	.8587	.00	.68	1786.56
338.90	.69	.747	.8593	.00	.69	1807.32
338.91	.69	.755	.8600	.00	.69	1828.15
338.92	.70	.764	.8607	.00	.70	1848.99
338.93	.71	.772	.8614	.00	.71	1869.79
338.94	.71	.781	.8620	.00	.71	1890.67
338.95	.72	.790	.8627	.00	.72	1911.57
338.96	.73	.798	.8634	.00	.73	1932.42
338.97	.73	.807	.8640	.00	.73	1953.35
338.98	.74	.816	.8647	.00	.74	1974.29
338.99	.75	.824	.8654	.00	.75	1995.19

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.00	.75	.833	.8661	.00	.75	2016.17
339.01	.76	.841	.8667	.00	.76	2037.16
339.02	.76	.850	.8674	.00	.76	2058.11
339.03	.78	.859	.8681	.00	.78	2079.14
339.04	.79	.868	.8688	.00	.79	2100.18
339.05	.81	.876	.8694	.00	.81	2121.19
339.06	.83	.885	.8701	.00	.83	2142.28
339.07	.84	.894	.8708	.00	.84	2163.38
339.08	.87	.902	.8715	.00	.87	2184.45
339.09	.89	.911	.8721	.00	.89	2205.59
339.10	.91	.920	.8728	.00	.91	2226.75
339.11	.94	.928	.8735	.00	.94	2247.86
339.12	.96	.937	.8742	.00	.96	2269.05
339.13	.99	.946	.8749	.00	.99	2290.26
339.14	1.02	.955	.8755	.00	1.02	2311.49
339.15	1.05	.963	.8762	.00	1.05	2332.67
339.16	1.08	.972	.8769	.00	1.08	2353.93
339.17	1.11	.981	.8776	.00	1.11	2375.22
339.18	1.13	.990	.8782	.00	1.13	2396.44
339.19	1.15	.999	.8789	.00	1.15	2417.74

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.20	1.17	1.007	.8796	.00	1.17	2439.06
339.21	1.19	1.016	.8803	.00	1.19	2460.33
339.22	1.21	1.025	.8810	.00	1.21	2481.68
339.23	1.23	1.034	.8816	.00	1.23	2503.05
339.24	1.25	1.043	.8823	.00	1.25	2524.36
339.25	1.26	1.051	.8830	.00	1.26	2545.76
339.26	1.28	1.060	.8837	.00	1.28	2567.18
339.27	1.30	1.069	.8844	.00	1.30	2588.54
339.28	1.31	1.078	.8850	.00	1.31	2609.99
339.29	1.33	1.087	.8857	.00	1.33	2631.45
339.30	1.34	1.096	.8864	.00	1.34	2652.86
339.31	1.36	1.105	.8871	.00	1.36	2674.36
339.32	1.37	1.113	.8878	.00	1.37	2695.87
339.33	1.39	1.122	.8884	.00	1.39	2717.33
339.34	1.40	1.131	.8891	.00	1.40	2738.88
339.35	1.42	1.140	.8898	.00	1.42	2760.44
339.36	1.43	1.149	.8905	.00	1.43	2781.95
339.37	1.45	1.158	.8912	.00	1.45	2803.54
339.38	1.47	1.167	.8919	.00	1.47	2825.16
339.39	1.48	1.176	.8925	.00	1.48	2846.79

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.40	1.49	1.185	.8932	.00	1.49	2868.36
339.41	1.51	1.194	.8939	.00	1.51	2890.02
339.42	1.52	1.203	.8946	.00	1.52	2911.70
339.43	1.53	1.211	.8953	.00	1.53	2933.32
339.44	1.55	1.220	.8960	.00	1.55	2955.03
339.45	1.56	1.229	.8967	.00	1.56	2976.76
339.46	1.57	1.238	.8973	.00	1.57	2998.43
339.47	1.59	1.247	.8980	.00	1.59	3020.19
339.48	1.60	1.256	.8987	.00	1.60	3041.96
339.49	1.61	1.265	.8994	.00	1.61	3063.69
339.50	1.62	1.274	.9001	.00	1.62	3085.50
339.51	1.64	1.283	.9008	.00	1.64	3107.32
339.52	1.65	1.292	.9015	.00	1.65	3129.09
339.53	1.66	1.301	.9021	.00	1.66	3150.95
339.54	1.67	1.310	.9028	.00	1.67	3172.83
339.55	1.69	1.319	.9035	.00	1.69	3194.65
339.56	1.70	1.328	.9042	.00	1.70	3216.56
339.57	1.71	1.338	.9049	.00	1.71	3238.48
339.58	1.72	1.347	.9056	.00	1.72	3260.35
339.59	1.73	1.356	.9063	.00	1.73	3282.31

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.60	1.74	1.365	.9070	.00	1.74	3304.28
339.61	1.76	1.374	.9077	.00	1.76	3326.21
339.62	1.77	1.383	.9083	.00	1.77	3348.21
339.63	1.78	1.392	.9090	.00	1.78	3370.23
339.64	1.79	1.401	.9097	.00	1.79	3392.27
339.65	1.80	1.410	.9104	.00	1.80	3414.26
339.66	1.81	1.419	.9111	.00	1.81	3436.34
339.67	1.82	1.428	.9118	.00	1.82	3458.43
339.68	1.83	1.437	.9125	.00	1.83	3480.47
339.69	1.85	1.447	.9132	.00	1.85	3502.59
339.70	1.86	1.456	.9139	.00	1.86	3524.73
339.71	1.87	1.465	.9146	.00	1.87	3546.82
339.72	1.88	1.474	.9153	.00	1.88	3568.99
339.73	1.89	1.483	.9160	.00	1.89	3591.18
339.74	1.90	1.492	.9166	.00	1.90	3613.32
339.75	1.91	1.502	.9173	.00	1.91	3635.54
339.76	1.92	1.511	.9180	.00	1.92	3657.78
339.77	1.93	1.520	.9187	.00	1.93	3679.97
339.78	1.94	1.529	.9194	.00	1.94	3702.25
339.79	1.95	1.538	.9201	.00	1.95	3724.54

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.80	1.96	1.547	.9208	.00	1.96	3746.78
339.81	1.97	1.557	.9215	.00	1.97	3769.10
339.82	1.98	1.566	.9222	.00	1.98	3791.44
339.83	1.99	1.575	.9229	.00	1.99	3813.73
339.84	2.00	1.584	.9236	.00	2.00	3836.11
339.85	2.01	1.594	.9243	.00	2.01	3858.50
339.86	2.02	1.603	.9250	.00	2.02	3880.84
339.87	2.03	1.612	.9257	.00	2.03	3903.26
339.88	2.04	1.621	.9264	.00	2.04	3925.70
339.89	2.05	1.631	.9271	.00	2.05	3948.16
339.90	2.06	1.640	.9278	.00	2.06	3970.57
339.91	2.07	1.649	.9285	.00	2.07	3993.06
339.92	2.08	1.658	.9292	.00	2.08	4015.57
339.93	2.09	1.668	.9299	.00	2.09	4038.03
339.94	2.10	1.677	.9306	.00	2.10	4060.57
339.95	2.11	1.686	.9313	.00	2.11	4083.13
339.96	2.12	1.696	.9320	.00	2.12	4105.64
339.97	2.13	1.705	.9327	.00	2.13	4128.23
339.98	2.14	1.714	.9334	.00	2.14	4150.84
339.99	2.15	1.724	.9341	.00	2.15	4173.40

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.00	2.16	1.733	.9348	.00	2.16	4196.05
340.01	2.17	1.742	.9355	.00	2.17	4218.71
340.02	2.18	1.752	.9362	.00	2.18	4241.32
340.03	2.19	1.761	.9369	.00	2.19	4264.01
340.04	2.20	1.770	.9376	.00	2.20	4286.72
340.05	2.21	1.780	.9383	.00	2.21	4309.38
340.06	2.22	1.789	.9390	.00	2.22	4332.13
340.07	2.22	1.799	.9397	.00	2.22	4354.89
340.08	2.23	1.808	.9404	.00	2.23	4377.60
340.09	2.24	1.817	.9411	.00	2.24	4400.40
340.10	2.25	1.827	.9418	.00	2.25	4423.21
340.11	2.26	1.836	.9425	.00	2.26	4445.97
340.12	2.27	1.846	.9432	.00	2.27	4468.82
340.13	2.28	1.855	.9439	.00	2.28	4491.69
340.14	2.29	1.865	.9446	.00	2.29	4514.57
340.15	2.30	1.874	.9453	.00	2.30	4537.40
340.16	2.31	1.883	.9460	.00	2.31	4560.31
340.17	2.31	1.893	.9467	.00	2.31	4583.24
340.18	2.32	1.902	.9474	.00	2.32	4606.12
340.19	2.33	1.912	.9481	.00	2.33	4629.09

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.20	2.34	1.921	.9488	.00	2.34	4652.07
340.21	2.35	1.931	.9495	.00	2.35	4675.01
340.22	2.36	1.940	.9502	.00	2.36	4698.02
340.23	2.37	1.950	.9509	.00	2.37	4721.06
340.24	2.38	1.959	.9516	.00	2.38	4744.04
340.25	2.38	1.969	.9523	.00	2.38	4767.11
340.26	2.39	1.978	.9530	.00	2.39	4790.20
340.27	2.40	1.988	.9538	.00	2.40	4813.23
340.28	2.41	1.997	.9545	.00	2.41	4836.35
340.29	2.42	2.007	.9552	.00	2.42	4859.49
340.30	2.43	2.017	.9559	.00	2.43	4882.57
340.31	2.44	2.026	.9566	.00	2.44	4905.74
340.32	2.44	2.036	.9573	.00	2.44	4928.93
340.33	2.45	2.045	.9580	.00	2.45	4952.07
340.34	2.46	2.055	.9587	.00	2.46	4975.29
340.35	2.47	2.064	.9594	.00	2.47	4998.53
340.36	2.48	2.074	.9601	.00	2.48	5021.72
340.37	2.49	2.084	.9608	.00	2.49	5044.99
340.38	2.49	2.093	.9615	.00	2.49	5068.28
340.39	2.50	2.103	.9623	.00	2.50	5091.59

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.40	2.51	2.113	.9630	.00	2.51	5114.85
340.41	2.52	2.122	.9637	.00	2.52	5138.19
340.42	2.53	2.132	.9644	.00	2.53	5161.55
340.43	2.54	2.141	.9651	.00	2.54	5184.86
340.44	2.54	2.151	.9658	.00	2.54	5208.25
340.45	2.55	2.161	.9665	.00	2.55	5231.67
340.46	2.56	2.170	.9672	.00	2.56	5255.02
340.47	2.57	2.180	.9679	.00	2.57	5278.47
340.48	2.58	2.190	.9687	.00	2.58	5301.93
340.49	2.58	2.199	.9694	.00	2.58	5325.34
340.50	2.59	2.209	.9701	.00	2.59	5348.84
340.51	2.60	2.219	.9708	.00	2.60	5372.36
340.52	2.61	2.229	.9715	.00	2.61	5395.82
340.53	2.62	2.238	.9722	.00	2.62	5419.37
340.54	2.62	2.248	.9729	.00	2.62	5442.94
340.55	2.63	2.258	.9737	.00	2.63	5466.45
340.56	2.64	2.268	.9744	.00	2.64	5490.05
340.57	2.65	2.277	.9751	.00	2.65	5513.67
340.58	2.65	2.287	.9758	.00	2.65	5537.24
340.59	2.66	2.297	.9765	.00	2.66	5560.89

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.60	2.67	2.307	.9772	.00	2.67	5584.56
340.61	2.68	2.316	.9779	.00	2.68	5608.18
340.62	2.69	2.326	.9787	.00	2.69	5631.88
340.63	2.69	2.336	.9794	.00	2.69	5655.61
340.64	2.70	2.346	.9801	.00	2.70	5679.35
340.65	2.71	2.356	.9808	.00	2.71	5703.03
340.66	2.72	2.365	.9815	.00	2.72	5726.81
340.67	2.72	2.375	.9822	.00	2.72	5750.60
340.68	2.73	2.385	.9830	.00	2.73	5774.34
340.69	2.74	2.395	.9837	.00	2.74	5798.16
340.70	2.75	2.405	.9844	.00	2.75	5822.01
340.71	2.75	2.414	.9851	.00	2.75	5845.80
340.72	2.76	2.424	.9858	.00	2.76	5869.68
340.73	2.77	2.434	.9866	.00	2.77	5893.57
340.74	2.78	2.444	.9873	.00	2.78	5917.42
340.75	2.78	2.454	.9880	.00	2.78	5941.35
340.76	2.79	2.464	.9887	.00	2.79	5965.30
340.77	2.80	2.474	.9894	.00	2.80	5989.19
340.78	2.81	2.484	.9902	.00	2.81	6013.17
340.79	2.81	2.494	.9909	.00	2.81	6037.17

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.80	2.82	2.503	.9916	.00	2.82	6061.12
340.81	2.83	2.513	.9923	.00	2.83	6085.16
340.82	2.84	2.523	.9930	.00	2.84	6109.21
340.83	2.84	2.533	.9938	.00	2.84	6133.21
340.84	2.85	2.543	.9945	.00	2.85	6157.30
340.85	2.86	2.553	.9952	.00	2.86	6181.40
340.86	2.87	2.563	.9959	.00	2.87	6205.45
340.87	2.87	2.573	.9966	.00	2.87	6229.59
340.88	2.88	2.583	.9974	.00	2.88	6253.75
340.89	2.89	2.593	.9981	.00	2.89	6277.93
340.90	2.89	2.603	.9988	.00	2.89	6302.05
340.91	2.90	2.613	.9995	.00	2.90	6326.26
340.92	2.91	2.623	1.0003	.00	2.91	6350.49
340.93	2.92	2.633	1.0010	.00	2.92	6374.66
340.94	2.92	2.643	1.0017	.00	2.92	6398.92
340.95	2.93	2.653	1.0024	.00	2.93	6423.20
340.96	2.94	2.663	1.0032	.00	2.94	6447.43
340.97	2.95	2.673	1.0039	.00	2.95	6471.74
340.98	2.95	2.683	1.0046	.00	2.95	6496.08
340.99	2.96	2.693	1.0053	.00	2.96	6520.36

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.00	2.97	2.703	1.0061	.00	2.97	6544.72
341.01	2.97	2.713	1.0068	.00	2.97	6569.11
341.02	2.98	2.723	1.0075	.00	2.98	6593.44
341.03	2.99	2.733	1.0082	.00	2.99	6617.86
341.04	2.99	2.744	1.0090	.00	2.99	6642.30
341.05	3.00	2.754	1.0097	.00	3.00	6666.68
341.06	3.01	2.764	1.0104	.00	3.01	6691.16
341.07	3.02	2.774	1.0112	.00	3.02	6715.65
341.08	3.02	2.784	1.0119	.00	3.02	6740.08
341.09	3.03	2.794	1.0126	.00	3.03	6764.61
341.10	3.04	2.804	1.0133	.00	3.04	6789.16
341.11	3.04	2.814	1.0141	.00	3.04	6813.65
341.12	3.05	2.824	1.0148	.00	3.05	6838.23
341.13	3.06	2.835	1.0155	.00	3.06	6862.82
341.14	3.06	2.845	1.0163	.00	3.06	6887.44
341.15	3.07	2.855	1.0170	.00	3.07	6912.00
341.16	3.08	2.865	1.0177	.00	3.08	6936.65
341.17	3.08	2.875	1.0185	.00	3.08	6961.32
341.18	3.09	2.885	1.0192	.00	3.09	6985.93
341.19	3.10	2.896	1.0199	.00	3.10	7010.63

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.20	3.10	2.906	1.0206	.00	3.10	7035.35
341.21	3.11	2.916	1.0214	.00	3.11	7060.02
341.22	3.12	2.926	1.0221	.00	3.12	7084.77
341.23	3.12	2.937	1.0228	.00	3.12	7109.55
341.24	3.13	2.947	1.0236	.00	3.13	7134.27
341.25	3.14	2.957	1.0243	.00	3.14	7159.08
341.26	3.14	2.967	1.0250	.00	3.14	7183.90
341.27	3.15	2.977	1.0258	.00	3.15	7208.67
341.28	3.16	2.988	1.0265	.00	3.16	7233.54
341.29	3.16	2.998	1.0272	.00	3.16	7258.42
341.30	3.17	3.008	1.0280	.00	3.17	7283.24
341.31	3.18	3.019	1.0287	.00	3.18	7308.16
341.32	3.18	3.029	1.0294	.00	3.18	7333.09
341.33	3.19	3.039	1.0302	.00	3.19	7357.97
341.34	3.20	3.049	1.0309	.00	3.20	7382.94
341.35	3.20	3.060	1.0316	.00	3.20	7407.93
341.36	3.21	3.070	1.0324	.00	3.21	7432.86
341.37	3.22	3.080	1.0331	.00	3.22	7457.88
341.38	3.22	3.091	1.0339	.00	3.22	7482.92
341.39	3.23	3.101	1.0346	.00	3.23	7507.98

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.40	3.24	3.111	1.0353	.00	3.24	7532.98
341.41	3.24	3.122	1.0361	.00	3.24	7558.08
341.42	3.25	3.132	1.0368	.00	3.25	7583.19
341.43	3.26	3.143	1.0375	.00	3.26	7608.24
341.44	3.26	3.153	1.0383	.00	3.26	7633.39
341.45	3.27	3.163	1.0390	.00	3.27	7658.56
341.46	3.28	3.174	1.0398	.00	3.28	7683.66
341.47	3.28	3.184	1.0405	.00	3.28	7708.87
341.48	3.29	3.195	1.0412	.00	3.29	7734.09
341.49	3.29	3.205	1.0420	.00	3.29	7759.25
341.50	3.30	3.215	1.0427	.00	3.30	7784.50
341.51	3.31	3.226	1.0434	.00	3.31	7809.78
341.52	3.31	3.236	1.0442	.00	3.31	7834.99
341.53	3.32	3.247	1.0449	.00	3.32	7860.30
341.54	3.33	3.257	1.0457	.00	3.33	7885.63
341.55	3.33	3.268	1.0464	.00	3.33	7910.89
341.56	3.34	3.278	1.0471	.00	3.34	7936.26
341.57	3.35	3.289	1.0479	.00	3.35	7961.64
341.58	3.35	3.299	1.0486	.00	3.35	7986.96
341.59	3.36	3.310	1.0494	.00	3.36	8012.38

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.60	3.36	3.320	1.0501	.00	3.36	8037.81
341.61	3.37	3.331	1.0509	.00	3.37	8063.19
341.62	3.38	3.341	1.0516	.00	3.38	8088.66
341.63	3.38	3.352	1.0523	.00	3.38	8114.15
341.64	3.39	3.362	1.0531	.00	3.39	8139.65
341.65	3.40	3.373	1.0538	.00	3.40	8165.10
341.66	3.40	3.383	1.0546	.00	3.40	8190.64
341.67	3.41	3.394	1.0553	.00	3.41	8216.20
341.68	3.41	3.404	1.0561	.00	3.41	8241.70
341.69	3.42	3.415	1.0568	.00	3.42	8267.30
341.70	3.43	3.425	1.0575	.00	3.43	8292.92
341.71	3.43	3.436	1.0583	.00	3.43	8318.47
341.72	3.44	3.447	1.0590	.00	3.44	8344.12
341.73	3.45	3.457	1.0598	.00	3.45	8369.79
341.74	3.45	3.468	1.0605	.00	3.45	8395.40
341.75	3.46	3.478	1.0613	.00	3.46	8421.10
341.76	3.46	3.489	1.0620	.00	3.46	8446.83
341.77	3.47	3.500	1.0628	.00	3.47	8472.49
341.78	3.48	3.510	1.0635	.00	3.48	8498.25
341.79	3.48	3.521	1.0643	.00	3.48	8524.02

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.80	3.49	3.532	1.0650	.00	3.49	8549.74
341.81	3.49	3.542	1.0658	.00	3.49	8575.56
341.82	3.50	3.553	1.0665	.00	3.50	8601.39
341.83	3.51	3.563	1.0672	.00	3.51	8627.16
341.84	3.51	3.574	1.0680	.00	3.51	8653.03
341.85	3.52	3.585	1.0687	.00	3.52	8678.91
341.86	3.52	3.596	1.0695	.00	3.52	8704.73
341.87	3.53	3.606	1.0702	.00	3.53	8730.66
341.88	3.54	3.617	1.0710	.00	3.54	8756.60
341.89	3.54	3.628	1.0717	.00	3.54	8782.56
341.90	3.55	3.638	1.0725	.00	3.55	8808.45
341.91	3.55	3.649	1.0732	.00	3.55	8834.45
341.92	3.56	3.660	1.0740	.00	3.56	8860.46
341.93	3.57	3.671	1.0747	.00	3.57	8886.41
341.94	3.57	3.681	1.0755	.00	3.57	8912.46
341.95	3.58	3.692	1.0762	.00	3.58	8938.53
341.96	3.58	3.703	1.0770	.00	3.58	8964.53
341.97	3.59	3.714	1.0777	.00	3.59	8990.64
341.98	3.60	3.724	1.0785	.00	3.60	9016.76
341.99	3.60	3.735	1.0792	.00	3.60	9042.82

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.00	3.61	3.746	1.0800	.00	3.61	9068.98
342.01	3.61	3.757	1.0807	.00	3.61	9095.16
342.02	3.62	3.768	1.0815	.00	3.62	9121.27
342.03	3.62	3.778	1.0822	.00	3.62	9147.48
342.04	3.63	3.789	1.0829	.00	3.63	9173.71
342.05	3.64	3.800	1.0836	.00	3.64	9199.88
342.06	3.64	3.811	1.0844	.00	3.64	9226.14
342.07	3.65	3.822	1.0851	.00	3.65	9252.42
342.08	3.65	3.833	1.0858	.00	3.65	9278.64
342.09	3.66	3.844	1.0865	.00	3.66	9304.96
342.10	3.67	3.854	1.0873	.00	3.67	9331.29
342.11	3.67	3.865	1.0880	.00	3.67	9357.56
342.12	3.68	3.876	1.0887	.00	3.68	9383.93
342.13	3.68	3.887	1.0895	.00	3.68	9410.32
342.14	3.69	3.898	1.0902	.00	3.69	9436.73
342.15	3.69	3.909	1.0909	.00	3.69	9463.07
342.16	3.70	3.920	1.0916	.00	3.70	9489.51
342.17	3.71	3.931	1.0924	.00	3.71	9515.97
342.18	3.71	3.942	1.0931	.00	3.71	9542.36
342.19	3.72	3.953	1.0938	.00	3.72	9568.86

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.20	3.72	3.963	1.0946	.00	3.72	9595.37
342.21	3.73	3.974	1.0953	.00	3.73	9621.81
342.22	3.73	3.985	1.0960	.00	3.73	9648.36
342.23	3.74	3.996	1.0968	.00	3.74	9674.93
342.24	3.75	4.007	1.0975	.00	3.75	9701.43
342.25	3.75	4.018	1.0982	.00	3.75	9728.03
342.26	3.76	4.029	1.0989	.00	3.76	9754.64
342.27	3.76	4.040	1.0997	.00	3.76	9781.20
342.28	3.77	4.051	1.1004	.00	3.77	9807.85
342.29	3.77	4.062	1.1011	.00	3.77	9834.52
342.30	3.78	4.073	1.1019	.00	3.78	9861.13
342.31	3.78	4.084	1.1026	.00	3.78	9887.83
342.32	3.79	4.095	1.1033	.00	3.79	9914.56
342.33	3.80	4.106	1.1041	.00	3.80	9941.22
342.34	3.80	4.117	1.1048	.00	3.80	9967.98
342.35	3.81	4.128	1.1055	.00	3.81	9994.75
342.36	3.81	4.140	1.1063	.00	3.81	10021.47
342.37	3.82	4.151	1.1070	.00	3.82	10048.28
342.38	3.82	4.162	1.1078	.00	3.82	10075.11
342.39	3.83	4.173	1.1085	.00	3.83	10101.96

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.40	3.84	4.184	1.1092	.00	3.84	10128.74
342.41	3.84	4.195	1.1100	.00	3.84	10155.63
342.42	3.85	4.206	1.1107	.00	3.85	10182.53
342.43	3.85	4.217	1.1114	.00	3.85	10209.36
342.44	3.86	4.228	1.1122	.00	3.86	10236.30
342.45	3.86	4.239	1.1129	.00	3.86	10263.26
342.46	3.87	4.251	1.1136	.00	3.87	10290.15
342.47	3.87	4.262	1.1144	.00	3.87	10317.14
342.48	3.88	4.273	1.1151	.00	3.88	10344.15
342.49	3.88	4.284	1.1158	.00	3.88	10371.09
342.50	3.89	4.295	1.1166	.00	3.89	10398.14
342.51	3.90	4.306	1.1173	.00	3.90	10425.20
342.52	3.90	4.317	1.1181	.00	3.90	10452.20
342.53	3.91	4.329	1.1188	.00	3.91	10479.29
342.54	3.91	4.340	1.1195	.00	3.91	10506.41
342.55	3.92	4.351	1.1203	.00	3.92	10533.46
342.56	3.92	4.362	1.1210	.00	3.92	10560.61
342.57	3.93	4.373	1.1218	.00	3.93	10587.78
342.58	3.93	4.385	1.1225	.00	3.93	10614.88
342.59	3.94	4.396	1.1232	.00	3.94	10642.09

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.60	3.94	4.407	1.1240	.00	3.94	10669.31
342.61	3.95	4.418	1.1247	.00	3.95	10696.47
342.62	3.95	4.430	1.1255	.00	3.95	10723.73
342.63	3.96	4.441	1.1262	.00	3.96	10751.00
342.64	3.96	4.452	1.1269	.00	3.96	10778.30
342.65	3.97	4.463	1.1277	.00	3.97	10805.53
342.66	3.97	4.475	1.1284	.00	3.97	10832.86
342.67	3.98	4.486	1.1292	.00	3.98	10860.21
342.68	3.98	4.497	1.1299	.00	3.98	10887.49
342.69	3.99	4.509	1.1306	.00	3.99	10914.88
342.70	3.99	4.520	1.1314	.00	3.99	10942.28
342.71	4.00	4.531	1.1321	.00	4.00	10969.61
342.72	4.00	4.543	1.1329	.00	4.00	10997.05
342.73	4.01	4.554	1.1336	.00	4.01	11024.51
342.74	4.01	4.565	1.1344	.00	4.01	11051.90
342.75	4.02	4.577	1.1351	.00	4.02	11079.39
342.76	4.02	4.588	1.1359	.00	4.02	11106.90
342.77	4.03	4.599	1.1366	.00	4.03	11134.35
342.78	4.03	4.611	1.1373	.00	4.03	11161.90
342.79	4.04	4.622	1.1381	.00	4.04	11189.46

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.80	4.04	4.633	1.1388	.00	4.04	11216.96
342.81	4.05	4.645	1.1396	.00	4.05	11244.56
342.82	4.05	4.656	1.1403	.00	4.05	11272.18
342.83	4.06	4.668	1.1411	.00	4.06	11299.73
342.84	4.06	4.679	1.1418	.00	4.06	11327.38
342.85	4.07	4.690	1.1426	.00	4.07	11355.06
342.86	4.07	4.702	1.1433	.00	4.07	11382.66
342.87	4.08	4.713	1.1441	.00	4.08	11410.37
342.88	4.08	4.725	1.1448	.00	4.08	11438.10
342.89	4.09	4.736	1.1455	.00	4.09	11465.85
342.90	4.09	4.748	1.1463	.00	4.09	11493.52
342.91	4.10	4.759	1.1470	.00	4.10	11521.31
342.92	4.10	4.771	1.1478	.00	4.10	11549.11
342.93	4.11	4.782	1.1485	.00	4.11	11576.84
342.94	4.11	4.794	1.1493	.00	4.11	11604.67
342.95	4.12	4.805	1.1500	.00	4.12	11632.53
342.96	4.12	4.817	1.1508	.00	4.12	11660.31
342.97	4.13	4.828	1.1515	.00	4.13	11688.21
342.98	4.13	4.840	1.1523	.00	4.13	11716.11
342.99	4.14	4.851	1.1530	.00	4.14	11743.96

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.00	4.14	4.863	1.1538	.00	4.14	11771.90
343.01	4.19	4.874	1.1545	.00	4.19	11799.91
343.02	4.28	4.886	1.1553	.00	4.28	11827.88
343.03	4.39	4.897	1.1560	.00	4.39	11855.99
343.04	4.51	4.909	1.1568	.00	4.51	11884.13
343.05	4.66	4.920	1.1575	.00	4.66	11912.21
343.06	4.82	4.932	1.1583	.00	4.82	11940.42
343.07	4.99	4.944	1.1590	.00	4.99	11968.66
343.08	5.18	4.955	1.1598	.00	5.18	11996.85
343.09	5.38	4.967	1.1605	.00	5.38	12025.15
343.10	5.58	4.978	1.1613	.00	5.58	12053.48
343.11	5.80	4.990	1.1620	.00	5.80	12081.75
343.12	6.03	5.002	1.1628	.00	6.03	12110.14
343.13	6.28	5.013	1.1636	.00	6.28	12138.56
343.14	6.53	5.025	1.1643	.00	6.53	12167.01
343.15	6.79	5.037	1.1651	.00	6.79	12195.39
343.16	7.05	5.048	1.1658	.00	7.05	12223.89
343.17	7.33	5.060	1.1666	.00	7.33	12252.42
343.18	7.61	5.072	1.1673	.00	7.61	12280.88
343.19	7.90	5.083	1.1681	.00	7.90	12309.46

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.20	8.20	5.095	1.1688	.00	8.20	12338.06
343.21	8.51	5.107	1.1696	.00	8.51	12366.61
343.22	8.83	5.118	1.1703	.00	8.83	12395.26
343.23	9.15	5.130	1.1711	.00	9.15	12423.94
343.24	9.48	5.142	1.1719	.00	9.48	12452.56
343.25	9.81	5.154	1.1726	.00	9.81	12481.29
343.26	10.16	5.165	1.1734	.00	10.16	12510.05
343.27	10.51	5.177	1.1741	.00	10.51	12538.75
343.28	10.86	5.189	1.1749	.00	10.86	12567.55
343.29	11.23	5.200	1.1756	.00	11.23	12596.39
343.30	11.60	5.212	1.1764	.00	11.60	12625.16
343.31	11.97	5.224	1.1771	.00	11.97	12654.04
343.32	12.35	5.236	1.1779	.00	12.35	12682.94
343.33	12.74	5.248	1.1787	.00	12.74	12711.78
343.34	13.13	5.259	1.1794	.00	13.13	12740.74
343.35	13.53	5.271	1.1802	.00	13.53	12769.72
343.36	13.94	5.283	1.1809	.00	13.94	12798.63
343.37	14.35	5.295	1.1817	.00	14.35	12827.66
343.38	14.76	5.307	1.1825	.00	14.76	12856.71
343.39	15.19	5.318	1.1832	.00	15.19	12885.78

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.40	15.61	5.330	1.1840	.00	15.61	12914.79
343.41	16.04	5.342	1.1847	.00	16.04	12943.91
343.42	16.48	5.354	1.1855	.00	16.48	12973.06
343.43	16.92	5.366	1.1863	.00	16.92	13002.14
343.44	17.37	5.378	1.1870	.00	17.37	13031.33
343.45	17.82	5.390	1.1878	.00	17.82	13060.55
343.46	18.28	5.401	1.1885	.00	18.28	13089.70
343.47	18.74	5.413	1.1893	.00	18.74	13118.96
343.48	19.21	5.425	1.1901	.00	19.21	13148.24
343.49	19.67	5.437	1.1908	.00	19.67	13177.46
343.50	20.15	5.449	1.1916	.00	20.15	13206.79
343.51	20.72	5.461	1.1923	.00	20.72	13236.23
343.52	21.37	5.473	1.1931	.00	21.37	13265.68
343.53	22.07	5.485	1.1939	.00	22.07	13295.30
343.54	22.81	5.497	1.1946	.00	22.81	13324.97
343.55	23.59	5.509	1.1954	.00	23.59	13354.61
343.56	24.40	5.521	1.1962	.00	24.40	13384.39
343.57	25.25	5.533	1.1969	.00	25.25	13414.22
343.58	26.12	5.545	1.1977	.00	26.12	13444.00
343.59	27.02	5.557	1.1985	.00	27.02	13473.92

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.60	27.94	5.569	1.1992	.00	27.94	13503.88
343.61	28.87	5.581	1.2000	.00	28.87	13533.79
343.62	29.83	5.593	1.2007	.00	29.83	13563.83
343.63	30.82	5.605	1.2015	.00	30.82	13593.90
343.64	31.82	5.617	1.2023	.00	31.82	13624.02
343.65	32.84	5.629	1.2030	.00	32.84	13654.09
343.66	33.88	5.641	1.2038	.00	33.88	13684.28
343.67	34.94	5.653	1.2046	.00	34.94	13714.50
343.68	36.01	5.665	1.2053	.00	36.01	13744.68
343.69	37.11	5.677	1.2061	.00	37.11	13774.98
343.70	38.22	5.689	1.2069	.00	38.22	13805.32
343.71	39.35	5.701	1.2076	.00	39.35	13835.61
343.72	40.49	5.713	1.2084	.00	40.49	13866.01
343.73	41.65	5.725	1.2092	.00	41.65	13896.45
343.74	42.82	5.737	1.2099	.00	42.82	13926.83
343.75	44.01	5.749	1.2107	.00	44.01	13957.34
343.76	46.32	5.761	1.2115	.00	46.32	13988.99
343.77	47.56	5.774	1.2123	.00	47.56	14019.50
343.78	48.83	5.786	1.2130	.00	48.83	14050.14
343.79	50.10	5.798	1.2138	.00	50.10	14080.80

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.80	50.94	5.810	1.2146	.00	50.94	14110.96
343.81	52.24	5.822	1.2153	.00	52.24	14141.69
343.82	53.09	5.834	1.2161	.00	53.09	14171.99
343.83	54.42	5.846	1.2169	.00	54.42	14202.70
343.84	56.26	5.859	1.2176	.00	56.26	14234.03
343.85	57.10	5.871	1.2184	.00	57.10	14264.37
343.86	57.95	5.883	1.2192	.00	57.95	14294.66
343.87	58.81	5.895	1.2200	.00	58.81	14325.06
343.88	59.68	5.907	1.2207	.00	59.68	14355.49
343.89	60.56	5.920	1.2215	.00	60.56	14385.95
343.90	61.44	5.932	1.2223	.00	61.44	14416.34
343.91	62.35	5.944	1.2230	.00	62.35	14446.87
343.92	63.26	5.956	1.2238	.00	63.26	14477.41
343.93	64.18	5.968	1.2246	.00	64.18	14507.90
343.94	65.12	5.981	1.2254	.00	65.12	14538.50
343.95	66.05	5.993	1.2261	.00	66.05	14569.13
343.96	67.00	6.005	1.2269	.00	67.00	14599.70
343.97	67.96	6.018	1.2277	.00	67.96	14630.39
343.98	68.94	6.030	1.2285	.00	68.94	14661.12
343.99	69.91	6.042	1.2292	.00	69.91	14691.77

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.00	70.91	6.054	1.2300	.00	70.91	14722.54
344.01	71.91	6.067	1.2308	.00	71.91	14753.35
344.02	72.91	6.079	1.2316	.00	72.91	14784.09
344.03	73.93	6.091	1.2324	.00	73.93	14814.95
344.04	74.95	6.104	1.2332	.00	74.95	14845.84
344.05	76.00	6.116	1.2339	.00	76.00	14876.67
344.06	77.04	6.128	1.2347	.00	77.04	14907.61
344.07	78.09	6.141	1.2355	.00	78.09	14938.58
344.08	79.15	6.153	1.2363	.00	79.15	14969.49
344.09	80.23	6.165	1.2371	.00	80.23	15000.52
344.10	81.31	6.178	1.2379	.00	81.31	15031.58
344.11	82.39	6.190	1.2387	.00	82.39	15062.57
344.12	83.49	6.203	1.2395	.00	83.49	15093.68
344.13	84.60	6.215	1.2403	.00	84.60	15124.82
344.14	85.71	6.227	1.2410	.00	85.71	15155.99
344.15	86.83	6.240	1.2418	.00	86.83	15187.08
344.16	87.95	6.252	1.2426	.00	87.95	15218.30
344.17	89.09	6.265	1.2434	.00	89.09	15249.55
344.18	90.23	6.277	1.2442	.00	90.23	15280.73
344.19	91.39	6.290	1.2450	.00	91.39	15312.04

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.20	92.55	6.302	1.2458	.00	92.55	15343.37
344.21	93.72	6.314	1.2466	.00	93.72	15374.63
344.22	94.90	6.327	1.2474	.00	94.90	15406.02
344.23	96.08	6.339	1.2482	.00	96.08	15437.43
344.24	97.27	6.352	1.2490	.00	97.27	15468.77
344.25	98.48	6.364	1.2498	.00	98.48	15500.23
344.26	99.69	6.377	1.2506	.00	99.69	15531.73
344.27	100.90	6.389	1.2514	.00	100.90	15563.15
344.28	102.12	6.402	1.2521	.00	102.12	15594.69
344.29	103.35	6.414	1.2529	.00	103.35	15626.27
344.30	104.59	6.427	1.2537	.00	104.59	15657.77
344.31	105.83	6.439	1.2545	.00	105.83	15689.39
344.32	107.09	6.452	1.2553	.00	107.09	15721.05
344.33	108.34	6.465	1.2561	.00	108.34	15752.63
344.34	109.61	6.477	1.2569	.00	109.61	15784.34
344.35	110.89	6.490	1.2577	.00	110.89	15816.07
344.36	112.16	6.502	1.2585	.00	112.16	15847.73
344.37	113.45	6.515	1.2593	.00	113.45	15879.51
344.38	114.74	6.528	1.2601	.00	114.74	15911.31
344.39	116.04	6.540	1.2609	.00	116.04	15943.15

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.40	117.35	6.553	1.2617	.00	117.35	15974.92
344.41	118.66	6.565	1.2625	.00	118.66	16006.81
344.42	119.98	6.578	1.2633	.00	119.98	16038.72
344.43	121.31	6.591	1.2641	.00	121.31	16070.57
344.44	122.65	6.603	1.2649	.00	122.65	16102.53
344.45	124.00	6.616	1.2657	.00	124.00	16134.53
344.46	125.34	6.629	1.2665	.00	125.34	16166.45
344.47	126.70	6.641	1.2673	.00	126.70	16198.49
344.48	128.06	6.654	1.2681	.00	128.06	16230.56
344.49	129.43	6.667	1.2689	.00	129.43	16262.57
344.50	130.81	6.679	1.2697	.00	130.81	16294.69
344.51	132.18	6.692	1.2705	.00	132.18	16326.84
344.52	133.57	6.705	1.2713	.00	133.57	16358.92
344.53	134.97	6.717	1.2721	.00	134.97	16391.12
344.54	136.37	6.730	1.2729	.00	136.37	16423.34
344.55	137.77	6.743	1.2737	.00	137.77	16455.49
344.56	139.19	6.756	1.2745	.00	139.19	16487.78
344.57	140.61	6.768	1.2753	.00	140.61	16520.08
344.58	142.03	6.781	1.2761	.00	142.03	16552.31
344.59	143.47	6.794	1.2769	.00	143.47	16584.66

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.60	144.91	6.807	1.2777	.00	144.91	16617.05
344.61	146.35	6.819	1.2785	.00	146.35	16649.36
344.62	147.80	6.832	1.2793	.00	147.80	16681.79
344.63	149.26	6.845	1.2801	.00	149.26	16714.24
344.64	150.71	6.858	1.2809	.00	150.71	16746.71
344.65	152.18	6.871	1.2817	.00	152.18	16779.12
344.66	153.65	6.883	1.2825	.00	153.65	16811.65
344.67	155.14	6.896	1.2833	.00	155.14	16844.22
344.68	156.63	6.909	1.2841	.00	156.63	16876.71
344.69	158.12	6.922	1.2849	.00	158.12	16909.31
344.70	159.62	6.935	1.2857	.00	159.62	16941.95
344.71	161.13	6.948	1.2865	.00	161.13	16974.52
344.72	162.64	6.961	1.2873	.00	162.64	17007.20
344.73	164.15	6.973	1.2882	.00	164.15	17039.91
344.74	165.67	6.986	1.2890	.00	165.67	17072.55
344.75	167.20	6.999	1.2898	.00	167.20	17105.31
344.76	168.74	7.012	1.2906	.00	168.74	17138.10
344.77	170.27	7.025	1.2914	.00	170.27	17170.81
344.78	171.82	7.038	1.2922	.00	171.82	17203.65
344.79	173.38	7.051	1.2930	.00	173.38	17236.52

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.80	174.93	7.064	1.2938	.00	174.93	17269.30
344.81	176.50	7.077	1.2946	.00	176.50	17302.22
344.82	178.06	7.090	1.2954	.00	178.06	17335.16
344.83	179.63	7.103	1.2962	.00	179.63	17368.03
344.84	181.22	7.116	1.2970	.00	181.22	17401.02
344.85	182.80	7.129	1.2978	.00	182.80	17434.03
344.86	184.39	7.142	1.2986	.00	184.39	17466.97
344.87	185.99	7.155	1.2995	.00	185.99	17500.03
344.88	187.59	7.168	1.3003	.00	187.59	17533.13
344.89	189.20	7.181	1.3011	.00	189.20	17566.24
344.90	190.80	7.194	1.3019	.00	190.80	17599.28
344.91	192.42	7.207	1.3027	.00	192.42	17632.45
344.92	194.05	7.220	1.3035	.00	194.05	17665.63
344.93	195.67	7.233	1.3043	.00	195.67	17698.75
344.94	197.31	7.246	1.3051	.00	197.31	17731.99
344.95	198.94	7.259	1.3059	.00	198.94	17765.25
344.96	200.58	7.272	1.3068	.00	200.58	17798.43
344.97	202.22	7.285	1.3076	.00	202.22	17831.74
344.98	203.88	7.298	1.3084	.00	203.88	17865.08
344.99	205.54	7.311	1.3092	.00	205.54	17898.35

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 IN 2
Outflow HYG file = NONE STORED - POND 2 OUT 2

Pond Node Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 338.00 ft
Starting Volume = .000 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
345.00	207.20	7.324	1.3100	.00	207.20	17931.74

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2       IN  2
Outflow HYG file = NONE STORED - POND 2       OUT 2

Pond Node   Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.00 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =     73.80 cfs    at   11.9200 hrs
Peak Outflow      =      2.80 cfs    at   13.6800 hrs
-----------------------------------------------------
Peak Elevation    =    340.77 ft
Peak Storage =          2.472 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =       4.008
- Infiltration =        .000
- HYG Vol OUT  =       2.561
- Retained Vol =       1.447
                   ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2       IN  10
Outflow HYG file = NONE STORED - POND 2       OUT 10

Pond Node   Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.00 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    121.54 cfs    at   11.9200 hrs
Peak Outflow      =      3.91 cfs    at   13.7900 hrs
-----------------------------------------------------
Peak Elevation    =    342.53 ft
Peak Storage =          4.326 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =       6.715
- Infiltration =        .000
- HYG Vol OUT  =       3.859
- Retained Vol =       2.856
                 ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2       IN  25
Outflow HYG file = NONE STORED - POND 2       OUT 25

Pond Node   Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.00 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    148.06 cfs    at   11.9200 hrs
Peak Outflow      =      8.37 cfs    at   12.9000 hrs
-----------------------------------------------------
Peak Elevation    =    343.21 ft
Peak Storage =          5.101 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =       8.259
- Infiltration =        .000
- HYG Vol OUT  =       4.796
- Retained Vol =       3.463
                   ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2       IN  100
Outflow HYG file = NONE STORED - POND 2       OUT 100

Pond Node   Data = POND 2
Pond Volume Data = POND 2
Pond Outlet Data = Riser/Barrel

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   338.00 ft
Starting Volume    =     .000 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    190.30 cfs    at   11.9200 hrs
Peak Outflow      =     48.69 cfs    at   12.1000 hrs
-----------------------------------------------------
Peak Elevation    =    343.78 ft
Peak Storage =          5.784 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =        .000
+ HYG Vol IN   =      10.759
- Infiltration =        .000
- HYG Vol OUT  =       6.986
- Retained Vol =       3.773
                  ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.00	.00	.000	.8000	.00	.00	.00
338.01	.00	.008	.8006	.00	.00	19.39
338.02	.00	.016	.8013	.00	.00	38.73
338.03	.00	.024	.8019	.00	.00	58.15
338.04	.00	.032	.8026	.00	.00	77.58
338.05	.00	.040	.8032	.00	.00	96.97
338.06	.00	.048	.8039	.00	.00	116.44
338.07	.00	.056	.8045	.00	.00	135.92
338.08	.00	.064	.8052	.00	.00	155.36
338.09	.00	.072	.8058	.00	.00	174.87
338.10	.00	.080	.8065	.00	.00	194.40
338.11	.00	.088	.8071	.00	.00	213.88
338.12	.00	.096	.8078	.00	.00	233.44
338.13	.00	.105	.8084	.00	.00	253.02
338.14	.00	.113	.8091	.00	.00	272.61
338.15	.00	.121	.8097	.00	.00	292.15
338.16	.00	.129	.8104	.00	.00	311.78
338.17	.00	.137	.8110	.00	.00	331.42
338.18	.00	.145	.8117	.00	.00	351.01
338.19	.00	.153	.8124	.00	.00	370.68

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.20	.00	.161	.8130	.00	.00	390.37
338.21	.00	.169	.8137	.00	.00	410.01
338.22	.00	.178	.8143	.00	.00	429.73
338.23	.00	.186	.8150	.00	.00	449.46
338.24	.00	.194	.8156	.00	.00	469.15
338.25	.00	.202	.8163	.00	.00	488.91
338.26	.00	.210	.8169	.00	.00	508.69
338.27	.00	.218	.8176	.00	.00	528.43
338.28	.00	.227	.8182	.00	.00	548.24
338.29	.00	.235	.8189	.00	.00	568.07
338.30	.00	.243	.8195	.00	.00	587.86
338.31	.00	.251	.8202	.00	.00	607.72
338.32	.00	.259	.8209	.00	.00	627.59
338.33	.00	.268	.8215	.00	.00	647.42
338.34	.00	.276	.8222	.00	.00	667.33
338.35	.00	.284	.8228	.00	.00	687.26
338.36	.00	.292	.8235	.00	.00	707.14
338.37	.00	.300	.8241	.00	.00	727.09
338.38	.00	.309	.8248	.00	.00	747.06
338.39	.00	.317	.8255	.00	.00	767.05

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.40	.00	.325	.8261	.00	.00	786.99
338.41	.00	.333	.8268	.00	.00	807.01
338.42	.00	.342	.8274	.00	.00	827.05
338.43	.00	.350	.8281	.00	.00	847.04
338.44	.00	.358	.8287	.00	.00	867.10
338.45	.00	.367	.8294	.00	.00	887.19
338.46	.00	.375	.8301	.00	.00	907.23
338.47	.00	.383	.8307	.00	.00	927.34
338.48	.00	.392	.8314	.00	.00	947.47
338.49	.00	.400	.8320	.00	.00	967.56
338.50	.00	.408	.8327	.00	.00	987.72
338.51	.00	.416	.8334	.00	.00	1007.90
338.52	.00	.425	.8340	.00	.00	1028.03
338.53	.00	.433	.8347	.00	.00	1048.25
338.54	.00	.442	.8354	.00	.00	1068.47
338.55	.00	.450	.8360	.00	.00	1088.65
338.56	.00	.458	.8367	.00	.00	1108.91
338.57	.00	.467	.8373	.00	.00	1129.19
338.58	.00	.475	.8380	.00	.00	1149.42
338.59	.00	.483	.8387	.00	.00	1169.73

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.60	.00	.492	.8393	.00	.00	1190.05
338.61	.00	.500	.8400	.00	.00	1210.33
338.62	.00	.509	.8407	.00	.00	1230.68
338.63	.00	.517	.8413	.00	.00	1251.05
338.64	.00	.525	.8420	.00	.00	1271.44
338.65	.00	.534	.8426	.00	.00	1291.78
338.66	.00	.542	.8433	.00	.00	1312.20
338.67	.00	.551	.8440	.00	.00	1332.64
338.68	.00	.559	.8446	.00	.00	1353.03
338.69	.00	.568	.8453	.00	.00	1373.50
338.70	.00	.576	.8460	.00	.00	1393.98
338.71	.00	.584	.8466	.00	.00	1414.42
338.72	.00	.593	.8473	.00	.00	1434.94
338.73	.00	.601	.8480	.00	.00	1455.47
338.74	.00	.610	.8486	.00	.00	1475.96
338.75	.00	.618	.8493	.00	.00	1496.52
338.76	.00	.627	.8500	.00	.00	1517.10
338.77	.00	.635	.8506	.00	.00	1537.64
338.78	.00	.644	.8513	.00	.00	1558.25
338.79	.00	.652	.8520	.00	.00	1578.88

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
338.80	.00	.661	.8526	.00	.00	1599.47
338.81	.00	.669	.8533	.00	.00	1620.13
338.82	.00	.678	.8540	.00	.00	1640.81
338.83	.00	.687	.8547	.00	.00	1661.44
338.84	.00	.695	.8553	.00	.00	1682.15
338.85	.00	.704	.8560	.00	.00	1702.88
338.86	.00	.712	.8567	.00	.00	1723.56
338.87	.00	.721	.8573	.00	.00	1744.32
338.88	.00	.729	.8580	.00	.00	1765.09
338.89	.00	.738	.8587	.00	.00	1785.88
338.90	.00	.747	.8593	.00	.00	1806.63
338.91	.00	.755	.8600	.00	.00	1827.45
338.92	.00	.764	.8607	.00	.00	1848.29
338.93	.00	.772	.8614	.00	.00	1869.09
338.94	.00	.781	.8620	.00	.00	1889.96
338.95	.00	.790	.8627	.00	.00	1910.85
338.96	.00	.798	.8634	.00	.00	1931.69
338.97	.00	.807	.8640	.00	.00	1952.61
338.98	.00	.816	.8647	.00	.00	1973.55
338.99	.00	.824	.8654	.00	.00	1994.44

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.00	.00	.833	.8661	.00	.00	2015.41
339.01	.00	.841	.8667	.00	.00	2036.40
339.02	.00	.850	.8674	.00	.00	2057.34
339.03	.01	.859	.8681	.00	.01	2078.37
339.04	.01	.868	.8688	.00	.01	2099.41
339.05	.03	.876	.8694	.00	.03	2120.42
339.06	.05	.885	.8701	.00	.05	2141.50
339.07	.06	.894	.8708	.00	.06	2162.60
339.08	.08	.902	.8715	.00	.08	2183.66
339.09	.10	.911	.8721	.00	.10	2204.80
339.10	.12	.920	.8728	.00	.12	2225.96
339.11	.14	.928	.8735	.00	.14	2247.06
339.12	.17	.937	.8742	.00	.17	2268.25
339.13	.19	.946	.8749	.00	.19	2289.46
339.14	.22	.955	.8755	.00	.22	2310.69
339.15	.25	.963	.8762	.00	.25	2331.87
339.16	.27	.972	.8769	.00	.27	2353.13
339.17	.31	.981	.8776	.00	.31	2374.41
339.18	.33	.990	.8782	.00	.33	2395.63
339.19	.34	.999	.8789	.00	.34	2416.93

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.20	.36	1.007	.8796	.00	.36	2438.25
339.21	.37	1.016	.8803	.00	.37	2459.51
339.22	.39	1.025	.8810	.00	.39	2480.86
339.23	.40	1.034	.8816	.00	.40	2502.22
339.24	.42	1.043	.8823	.00	.42	2523.53
339.25	.43	1.051	.8830	.00	.43	2544.93
339.26	.44	1.060	.8837	.00	.44	2566.34
339.27	.45	1.069	.8844	.00	.45	2587.70
339.28	.47	1.078	.8850	.00	.47	2609.14
339.29	.48	1.087	.8857	.00	.48	2630.60
339.30	.49	1.096	.8864	.00	.49	2652.01
339.31	.50	1.105	.8871	.00	.50	2673.50
339.32	.51	1.113	.8878	.00	.51	2695.01
339.33	.52	1.122	.8884	.00	.52	2716.47
339.34	.53	1.131	.8891	.00	.53	2738.01
339.35	.54	1.140	.8898	.00	.54	2759.56
339.36	.55	1.149	.8905	.00	.55	2781.07
339.37	.56	1.158	.8912	.00	.56	2802.66
339.38	.57	1.167	.8919	.00	.57	2824.27
339.39	.58	1.176	.8925	.00	.58	2845.89

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelwlft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.40	.59	1.185	.8932	.00	.59	2867.46
339.41	.60	1.194	.8939	.00	.60	2889.11
339.42	.61	1.203	.8946	.00	.61	2910.79
339.43	.62	1.211	.8953	.00	.62	2932.41
339.44	.63	1.220	.8960	.00	.63	2954.11
339.45	.64	1.229	.8967	.00	.64	2975.83
339.46	.64	1.238	.8973	.00	.64	2997.50
339.47	.65	1.247	.8980	.00	.65	3019.26
339.48	.66	1.256	.8987	.00	.66	3041.03
339.49	.67	1.265	.8994	.00	.67	3062.75
339.50	.68	1.274	.9001	.00	.68	3084.55
339.51	.69	1.283	.9008	.00	.69	3106.37
339.52	.69	1.292	.9015	.00	.69	3128.14
339.53	.70	1.301	.9021	.00	.70	3149.99
339.54	.71	1.310	.9028	.00	.71	3171.86
339.55	.72	1.319	.9035	.00	.72	3193.68
339.56	.73	1.328	.9042	.00	.73	3215.58
339.57	.73	1.338	.9049	.00	.73	3237.50
339.58	.74	1.347	.9056	.00	.74	3259.37
339.59	.75	1.356	.9063	.00	.75	3281.32

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.60	.76	1.365	.9070	.00	.76	3303.29
339.61	.76	1.374	.9077	.00	.76	3325.21
339.62	.77	1.383	.9083	.00	.77	3347.21
339.63	.78	1.392	.9090	.00	.78	3369.23
339.64	.78	1.401	.9097	.00	.78	3391.27
339.65	.79	1.410	.9104	.00	.79	3413.25
339.66	.80	1.419	.9111	.00	.80	3435.32
339.67	.80	1.428	.9118	.00	.80	3457.41
339.68	.81	1.437	.9125	.00	.81	3479.44
339.69	.82	1.447	.9132	.00	.82	3501.56
339.70	.82	1.456	.9139	.00	.82	3523.70
339.71	.83	1.465	.9146	.00	.83	3545.78
339.72	.84	1.474	.9153	.00	.84	3567.95
339.73	.84	1.483	.9160	.00	.84	3590.14
339.74	.85	1.492	.9166	.00	.85	3612.27
339.75	.86	1.502	.9173	.00	.86	3634.49
339.76	.86	1.511	.9180	.00	.86	3656.73
339.77	.87	1.520	.9187	.00	.87	3678.91
339.78	.88	1.529	.9194	.00	.88	3701.18
339.79	.88	1.538	.9201	.00	.88	3723.47

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
339.80	.89	1.547	.9208	.00	.89	3745.70
339.81	.90	1.557	.9215	.00	.90	3768.03
339.82	.90	1.566	.9222	.00	.90	3790.36
339.83	.91	1.575	.9229	.00	.91	3812.65
339.84	.91	1.584	.9236	.00	.91	3835.02
339.85	.92	1.594	.9243	.00	.92	3857.40
339.86	.93	1.603	.9250	.00	.93	3879.74
339.87	.93	1.612	.9257	.00	.93	3902.16
339.88	.94	1.621	.9264	.00	.94	3924.60
339.89	.94	1.631	.9271	.00	.94	3947.05
339.90	.95	1.640	.9278	.00	.95	3969.46
339.91	.96	1.649	.9285	.00	.96	3991.94
339.92	.96	1.658	.9292	.00	.96	4014.45
339.93	.97	1.668	.9299	.00	.97	4036.90
339.94	.97	1.677	.9306	.00	.97	4059.44
339.95	.98	1.686	.9313	.00	.98	4082.00
339.96	.98	1.696	.9320	.00	.98	4104.50
339.97	.99	1.705	.9327	.00	.99	4127.09
339.98	.99	1.714	.9334	.00	.99	4149.70
339.99	1.00	1.724	.9341	.00	1.00	4172.25

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.00	1.01	1.733	.9348	.00	1.01	4194.89
340.01	1.01	1.742	.9355	.00	1.01	4217.55
340.02	1.02	1.752	.9362	.00	1.02	4240.15
340.03	1.02	1.761	.9369	.00	1.02	4262.85
340.04	1.03	1.770	.9376	.00	1.03	4285.55
340.05	1.03	1.780	.9383	.00	1.03	4308.21
340.06	1.04	1.789	.9390	.00	1.04	4330.95
340.07	1.04	1.799	.9397	.00	1.04	4353.71
340.08	1.05	1.808	.9404	.00	1.05	4376.42
340.09	1.05	1.817	.9411	.00	1.05	4399.21
340.10	1.06	1.827	.9418	.00	1.06	4422.02
340.11	1.06	1.836	.9425	.00	1.06	4444.78
340.12	1.07	1.846	.9432	.00	1.07	4467.62
340.13	1.07	1.855	.9439	.00	1.07	4490.48
340.14	1.08	1.865	.9446	.00	1.08	4513.36
340.15	1.08	1.874	.9453	.00	1.08	4536.18
340.16	1.09	1.883	.9460	.00	1.09	4559.10
340.17	1.10	1.893	.9467	.00	1.10	4582.02
340.18	1.10	1.902	.9474	.00	1.10	4604.90
340.19	1.11	1.912	.9481	.00	1.11	4627.86

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.20	1.11	1.921	.9488	.00	1.11	4650.84
340.21	1.11	1.931	.9495	.00	1.11	4673.77
340.22	1.12	1.940	.9502	.00	1.12	4696.79
340.23	1.12	1.950	.9509	.00	1.12	4719.82
340.24	1.13	1.959	.9516	.00	1.13	4742.79
340.25	1.13	1.969	.9523	.00	1.13	4765.86
340.26	1.14	1.978	.9530	.00	1.14	4788.94
340.27	1.14	1.988	.9538	.00	1.14	4811.97
340.28	1.15	1.997	.9545	.00	1.15	4835.09
340.29	1.15	2.007	.9552	.00	1.15	4858.22
340.30	1.16	2.017	.9559	.00	1.16	4881.30
340.31	1.16	2.026	.9566	.00	1.16	4904.47
340.32	1.17	2.036	.9573	.00	1.17	4927.66
340.33	1.17	2.045	.9580	.00	1.17	4950.79
340.34	1.18	2.055	.9587	.00	1.18	4974.01
340.35	1.18	2.064	.9594	.00	1.18	4997.24
340.36	1.19	2.074	.9601	.00	1.19	5020.43
340.37	1.19	2.084	.9608	.00	1.19	5043.70
340.38	1.20	2.093	.9615	.00	1.20	5066.99
340.39	1.20	2.103	.9623	.00	1.20	5090.29

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.40	1.21	2.113	.9630	.00	1.21	5113.54
340.41	1.21	2.122	.9637	.00	1.21	5136.88
340.42	1.21	2.132	.9644	.00	1.21	5160.24
340.43	1.22	2.141	.9651	.00	1.22	5183.54
340.44	1.22	2.151	.9658	.00	1.22	5206.93
340.45	1.23	2.161	.9665	.00	1.23	5230.34
340.46	1.23	2.170	.9672	.00	1.23	5253.70
340.47	1.24	2.180	.9679	.00	1.24	5277.14
340.48	1.24	2.190	.9687	.00	1.24	5300.60
340.49	1.25	2.199	.9694	.00	1.25	5324.01
340.50	1.25	2.209	.9701	.00	1.25	5347.50
340.51	1.25	2.219	.9708	.00	1.25	5371.01
340.52	1.26	2.229	.9715	.00	1.26	5394.47
340.53	1.26	2.238	.9722	.00	1.26	5418.02
340.54	1.27	2.248	.9729	.00	1.27	5441.58
340.55	1.27	2.258	.9737	.00	1.27	5465.09
340.56	1.28	2.268	.9744	.00	1.28	5488.69
340.57	1.28	2.277	.9751	.00	1.28	5512.30
340.58	1.29	2.287	.9758	.00	1.29	5535.87
340.59	1.29	2.297	.9765	.00	1.29	5559.52

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.60	1.29	2.307	.9772	.00	1.29	5583.18
340.61	1.30	2.316	.9779	.00	1.30	5606.80
340.62	1.30	2.326	.9787	.00	1.30	5630.50
340.63	1.31	2.336	.9794	.00	1.31	5654.22
340.64	1.31	2.346	.9801	.00	1.31	5677.96
340.65	1.31	2.356	.9808	.00	1.31	5701.64
340.66	1.32	2.365	.9815	.00	1.32	5725.41
340.67	1.32	2.375	.9822	.00	1.32	5749.20
340.68	1.33	2.385	.9830	.00	1.33	5772.93
340.69	1.33	2.395	.9837	.00	1.33	5796.76
340.70	1.34	2.405	.9844	.00	1.34	5820.60
340.71	1.34	2.414	.9851	.00	1.34	5844.38
340.72	1.34	2.424	.9858	.00	1.34	5868.26
340.73	1.35	2.434	.9866	.00	1.35	5892.15
340.74	1.35	2.444	.9873	.00	1.35	5915.99
340.75	1.36	2.454	.9880	.00	1.36	5939.92
340.76	1.36	2.464	.9887	.00	1.36	5963.86
340.77	1.36	2.474	.9894	.00	1.36	5987.75
340.78	1.37	2.484	.9902	.00	1.37	6011.73
340.79	1.37	2.494	.9909	.00	1.37	6035.73

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
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Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
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No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
340.80	1.38	2.503	.9916	.00	1.38	6059.67
340.81	1.38	2.513	.9923	.00	1.38	6083.71
340.82	1.38	2.523	.9930	.00	1.38	6107.76
340.83	1.39	2.533	.9938	.00	1.39	6131.75
340.84	1.39	2.543	.9945	.00	1.39	6155.84
340.85	1.40	2.553	.9952	.00	1.40	6179.94
340.86	1.40	2.563	.9959	.00	1.40	6203.99
340.87	1.40	2.573	.9966	.00	1.40	6228.12
340.88	1.41	2.583	.9974	.00	1.41	6252.28
340.89	1.41	2.593	.9981	.00	1.41	6276.45
340.90	1.42	2.603	.9988	.00	1.42	6300.57
340.91	1.42	2.613	.9995	.00	1.42	6324.78
340.92	1.42	2.623	1.0003	.00	1.42	6349.00
340.93	1.43	2.633	1.0010	.00	1.43	6373.17
340.94	1.43	2.643	1.0017	.00	1.43	6397.43
340.95	1.44	2.653	1.0024	.00	1.44	6421.71
340.96	1.44	2.663	1.0032	.00	1.44	6445.93
340.97	1.44	2.673	1.0039	.00	1.44	6470.24
340.98	1.45	2.683	1.0046	.00	1.45	6494.57
340.99	1.45	2.693	1.0053	.00	1.45	6518.85

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.00	1.45	2.703	1.0061	.00	1.45	6543.21
341.01	1.46	2.713	1.0068	.00	1.46	6567.60
341.02	1.46	2.723	1.0075	.00	1.46	6591.92
341.03	1.47	2.733	1.0082	.00	1.47	6616.34
341.04	1.47	2.744	1.0090	.00	1.47	6640.78
341.05	1.47	2.754	1.0097	.00	1.47	6665.16
341.06	1.48	2.764	1.0104	.00	1.48	6689.63
341.07	1.48	2.774	1.0112	.00	1.48	6714.12
341.08	1.48	2.784	1.0119	.00	1.48	6738.55
341.09	1.49	2.794	1.0126	.00	1.49	6763.07
341.10	1.49	2.804	1.0133	.00	1.49	6787.61
341.11	1.50	2.814	1.0141	.00	1.50	6812.10
341.12	1.50	2.824	1.0148	.00	1.50	6836.67
341.13	1.50	2.835	1.0155	.00	1.50	6861.27
341.14	1.51	2.845	1.0163	.00	1.51	6885.88
341.15	1.51	2.855	1.0170	.00	1.51	6910.44
341.16	1.51	2.865	1.0177	.00	1.51	6935.08
341.17	1.52	2.875	1.0185	.00	1.52	6959.75
341.18	1.52	2.885	1.0192	.00	1.52	6984.36
341.19	1.52	2.896	1.0199	.00	1.52	7009.06

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.20	1.53	2.906	1.0206	.00	1.53	7033.78
341.21	1.53	2.916	1.0214	.00	1.53	7058.44
341.22	1.54	2.926	1.0221	.00	1.54	7083.19
341.23	1.54	2.937	1.0228	.00	1.54	7107.96
341.24	1.54	2.947	1.0236	.00	1.54	7132.68
341.25	1.55	2.957	1.0243	.00	1.55	7157.48
341.26	1.55	2.967	1.0250	.00	1.55	7182.31
341.27	1.55	2.977	1.0258	.00	1.55	7207.08
341.28	1.56	2.988	1.0265	.00	1.56	7231.94
341.29	1.56	2.998	1.0272	.00	1.56	7256.81
341.30	1.56	3.008	1.0280	.00	1.56	7281.63
341.31	1.57	3.019	1.0287	.00	1.57	7306.55
341.32	1.57	3.029	1.0294	.00	1.57	7331.48
341.33	1.57	3.039	1.0302	.00	1.57	7356.35
341.34	1.58	3.049	1.0309	.00	1.58	7381.32
341.35	1.58	3.060	1.0316	.00	1.58	7406.30
341.36	1.58	3.070	1.0324	.00	1.58	7431.23
341.37	1.59	3.080	1.0331	.00	1.59	7456.25
341.38	1.59	3.091	1.0339	.00	1.59	7481.29
341.39	1.60	3.101	1.0346	.00	1.60	7506.35

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
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Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.40	1.60	3.111	1.0353	.00	1.60	7531.34
341.41	1.60	3.122	1.0361	.00	1.60	7556.43
341.42	1.61	3.132	1.0368	.00	1.61	7581.54
341.43	1.61	3.143	1.0375	.00	1.61	7606.60
341.44	1.61	3.153	1.0383	.00	1.61	7631.74
341.45	1.62	3.163	1.0390	.00	1.62	7656.90
341.46	1.62	3.174	1.0398	.00	1.62	7682.01
341.47	1.62	3.184	1.0405	.00	1.62	7707.21
341.48	1.63	3.195	1.0412	.00	1.63	7732.42
341.49	1.63	3.205	1.0420	.00	1.63	7757.58
341.50	1.63	3.215	1.0427	.00	1.63	7782.83
341.51	1.64	3.226	1.0434	.00	1.64	7808.10
341.52	1.64	3.236	1.0442	.00	1.64	7833.32
341.53	1.64	3.247	1.0449	.00	1.64	7858.62
341.54	1.65	3.257	1.0457	.00	1.65	7883.95
341.55	1.65	3.268	1.0464	.00	1.65	7909.21
341.56	1.65	3.278	1.0471	.00	1.65	7934.57
341.57	1.66	3.289	1.0479	.00	1.66	7959.95
341.58	1.66	3.299	1.0486	.00	1.66	7985.27
341.59	1.66	3.310	1.0494	.00	1.66	8010.68

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
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Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
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Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.60	1.67	3.320	1.0501	.00	1.67	8036.11
341.61	1.67	3.331	1.0509	.00	1.67	8061.49
341.62	1.67	3.341	1.0516	.00	1.67	8086.95
341.63	1.68	3.352	1.0523	.00	1.68	8112.44
341.64	1.68	3.362	1.0531	.00	1.68	8137.94
341.65	1.68	3.373	1.0538	.00	1.68	8163.39
341.66	1.69	3.383	1.0546	.00	1.69	8188.93
341.67	1.69	3.394	1.0553	.00	1.69	8214.48
341.68	1.69	3.404	1.0561	.00	1.69	8239.98
341.69	1.70	3.415	1.0568	.00	1.70	8265.58
341.70	1.70	3.425	1.0575	.00	1.70	8291.19
341.71	1.70	3.436	1.0583	.00	1.70	8316.74
341.72	1.71	3.447	1.0590	.00	1.71	8342.39
341.73	1.71	3.457	1.0598	.00	1.71	8368.05
341.74	1.71	3.468	1.0605	.00	1.71	8393.66
341.75	1.72	3.478	1.0613	.00	1.72	8419.36
341.76	1.72	3.489	1.0620	.00	1.72	8445.08
341.77	1.72	3.500	1.0628	.00	1.72	8470.74
341.78	1.73	3.510	1.0635	.00	1.73	8496.50
341.79	1.73	3.521	1.0643	.00	1.73	8522.27

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
341.80	1.73	3.532	1.0650	.00	1.73	8547.99
341.81	1.73	3.542	1.0658	.00	1.73	8573.80
341.82	1.74	3.553	1.0665	.00	1.74	8599.62
341.83	1.74	3.563	1.0672	.00	1.74	8625.39
341.84	1.74	3.574	1.0680	.00	1.74	8651.26
341.85	1.75	3.585	1.0687	.00	1.75	8677.14
341.86	1.75	3.596	1.0695	.00	1.75	8702.96
341.87	1.75	3.606	1.0702	.00	1.75	8728.88
341.88	1.76	3.617	1.0710	.00	1.76	8754.82
341.89	1.76	3.628	1.0717	.00	1.76	8780.77
341.90	1.76	3.638	1.0725	.00	1.76	8806.67
341.91	1.77	3.649	1.0732	.00	1.77	8832.66
341.92	1.77	3.660	1.0740	.00	1.77	8858.67
341.93	1.77	3.671	1.0747	.00	1.77	8884.62
341.94	1.78	3.681	1.0755	.00	1.78	8910.67
341.95	1.78	3.692	1.0762	.00	1.78	8936.73
341.96	1.78	3.703	1.0770	.00	1.78	8962.73
341.97	1.78	3.714	1.0777	.00	1.78	8988.83
341.98	1.79	3.724	1.0785	.00	1.79	9014.95
341.99	1.79	3.735	1.0792	.00	1.79	9041.01

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.00	1.79	3.746	1.0800	.00	1.79	9067.17
342.01	1.80	3.757	1.0807	.00	1.80	9093.34
342.02	1.80	3.768	1.0815	.00	1.80	9119.45
342.03	1.80	3.778	1.0822	.00	1.80	9145.66
342.04	1.81	3.789	1.0829	.00	1.81	9171.89
342.05	1.81	3.800	1.0836	.00	1.81	9198.05
342.06	1.81	3.811	1.0844	.00	1.81	9224.31
342.07	1.82	3.822	1.0851	.00	1.82	9250.59
342.08	1.82	3.833	1.0858	.00	1.82	9276.81
342.09	1.82	3.844	1.0865	.00	1.82	9303.12
342.10	1.82	3.854	1.0873	.00	1.82	9329.45
342.11	1.83	3.865	1.0880	.00	1.83	9355.72
342.12	1.83	3.876	1.0887	.00	1.83	9382.09
342.13	1.83	3.887	1.0895	.00	1.83	9408.47
342.14	1.84	3.898	1.0902	.00	1.84	9434.88
342.15	1.84	3.909	1.0909	.00	1.84	9461.21
342.16	1.84	3.920	1.0916	.00	1.84	9487.65
342.17	1.85	3.931	1.0924	.00	1.85	9514.11
342.18	1.85	3.942	1.0931	.00	1.85	9540.50
342.19	1.85	3.953	1.0938	.00	1.85	9566.99

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.20	1.85	3.963	1.0946	.00	1.85	9593.50
342.21	1.86	3.974	1.0953	.00	1.86	9619.94
342.22	1.86	3.985	1.0960	.00	1.86	9646.49
342.23	1.86	3.996	1.0968	.00	1.86	9673.05
342.24	1.87	4.007	1.0975	.00	1.87	9699.55
342.25	1.87	4.018	1.0982	.00	1.87	9726.14
342.26	1.87	4.029	1.0989	.00	1.87	9752.76
342.27	1.88	4.040	1.0997	.00	1.88	9779.31
342.28	1.88	4.051	1.1004	.00	1.88	9805.96
342.29	1.88	4.062	1.1011	.00	1.88	9832.63
342.30	1.88	4.073	1.1019	.00	1.88	9859.23
342.31	1.89	4.084	1.1026	.00	1.89	9885.94
342.32	1.89	4.095	1.1033	.00	1.89	9912.66
342.33	1.89	4.106	1.1041	.00	1.89	9939.31
342.34	1.90	4.117	1.1048	.00	1.90	9966.07
342.35	1.90	4.128	1.1055	.00	1.90	9992.84
342.36	1.90	4.140	1.1063	.00	1.90	10019.55
342.37	1.90	4.151	1.1070	.00	1.90	10046.36
342.38	1.91	4.162	1.1078	.00	1.91	10073.19
342.39	1.91	4.173	1.1085	.00	1.91	10100.04

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.40	1.91	4.184	1.1092	.00	1.91	10126.82
342.41	1.92	4.195	1.1100	.00	1.92	10153.70
342.42	1.92	4.206	1.1107	.00	1.92	10180.60
342.43	1.92	4.217	1.1114	.00	1.92	10207.43
342.44	1.92	4.228	1.1122	.00	1.92	10234.37
342.45	1.93	4.239	1.1129	.00	1.93	10261.32
342.46	1.93	4.251	1.1136	.00	1.93	10288.21
342.47	1.93	4.262	1.1144	.00	1.93	10315.20
342.48	1.94	4.273	1.1151	.00	1.94	10342.20
342.49	1.94	4.284	1.1158	.00	1.94	10369.14
342.50	1.94	4.295	1.1166	.00	1.94	10396.19
342.51	1.94	4.306	1.1173	.00	1.94	10423.25
342.52	1.95	4.317	1.1181	.00	1.95	10450.24
342.53	1.95	4.329	1.1188	.00	1.95	10477.34
342.54	1.95	4.340	1.1195	.00	1.95	10504.45
342.55	1.96	4.351	1.1203	.00	1.96	10531.50
342.56	1.96	4.362	1.1210	.00	1.96	10558.65
342.57	1.96	4.373	1.1218	.00	1.96	10585.81
342.58	1.96	4.385	1.1225	.00	1.96	10612.92
342.59	1.97	4.396	1.1232	.00	1.97	10640.12

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.60	1.97	4.407	1.1240	.00	1.97	10667.34
342.61	1.97	4.418	1.1247	.00	1.97	10694.49
342.62	1.98	4.430	1.1255	.00	1.98	10721.75
342.63	1.98	4.441	1.1262	.00	1.98	10749.03
342.64	1.98	4.452	1.1269	.00	1.98	10776.32
342.65	1.98	4.463	1.1277	.00	1.98	10803.54
342.66	1.99	4.475	1.1284	.00	1.99	10830.87
342.67	1.99	4.486	1.1292	.00	1.99	10858.22
342.68	1.99	4.497	1.1299	.00	1.99	10885.50
342.69	1.99	4.509	1.1306	.00	1.99	10912.88
342.70	2.00	4.520	1.1314	.00	2.00	10940.28
342.71	2.00	4.531	1.1321	.00	2.00	10967.62
342.72	2.00	4.543	1.1329	.00	2.00	10995.05
342.73	2.01	4.554	1.1336	.00	2.01	11022.51
342.74	2.01	4.565	1.1344	.00	2.01	11049.90
342.75	2.01	4.577	1.1351	.00	2.01	11077.39
342.76	2.01	4.588	1.1359	.00	2.01	11104.89
342.77	2.02	4.599	1.1366	.00	2.02	11132.34
342.78	2.02	4.611	1.1373	.00	2.02	11159.88
342.79	2.02	4.622	1.1381	.00	2.02	11187.44

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
342.80	2.03	4.633	1.1388	.00	2.03	11214.94
342.81	2.03	4.645	1.1396	.00	2.03	11242.54
342.82	2.03	4.656	1.1403	.00	2.03	11270.15
342.83	2.03	4.668	1.1411	.00	2.03	11297.70
342.84	2.04	4.679	1.1418	.00	2.04	11325.36
342.85	2.04	4.690	1.1426	.00	2.04	11353.03
342.86	2.04	4.702	1.1433	.00	2.04	11380.63
342.87	2.04	4.713	1.1441	.00	2.04	11408.34
342.88	2.05	4.725	1.1448	.00	2.05	11436.06
342.89	2.05	4.736	1.1455	.00	2.05	11463.81
342.90	2.05	4.748	1.1463	.00	2.05	11491.48
342.91	2.05	4.759	1.1470	.00	2.05	11519.26
342.92	2.06	4.771	1.1478	.00	2.06	11547.06
342.93	2.06	4.782	1.1485	.00	2.06	11574.79
342.94	2.06	4.794	1.1493	.00	2.06	11602.62
342.95	2.07	4.805	1.1500	.00	2.07	11630.47
342.96	2.07	4.817	1.1508	.00	2.07	11658.26
342.97	2.07	4.828	1.1515	.00	2.07	11686.15
342.98	2.07	4.840	1.1523	.00	2.07	11714.05
342.99	2.08	4.851	1.1530	.00	2.08	11741.89

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.00	2.08	4.863	1.1538	.00	2.08	11769.83
343.01	2.13	4.874	1.1545	.00	2.13	11797.84
343.02	2.22	4.886	1.1553	.00	2.22	11825.82
343.03	2.33	4.897	1.1560	.00	2.33	11853.93
343.04	2.47	4.909	1.1568	.00	2.47	11882.08
343.05	2.62	4.920	1.1575	.00	2.62	11910.17
343.06	2.79	4.932	1.1583	.00	2.79	11938.39
343.07	2.97	4.944	1.1590	.00	2.97	11966.64
343.08	3.17	4.955	1.1598	.00	3.17	11994.84
343.09	3.37	4.967	1.1605	.00	3.37	12023.15
343.10	3.60	4.978	1.1613	.00	3.60	12051.49
343.11	3.83	4.990	1.1620	.00	3.83	12079.78
343.12	4.06	5.002	1.1628	.00	4.06	12108.17
343.13	4.31	5.013	1.1636	.00	4.31	12136.59
343.14	4.56	5.025	1.1643	.00	4.56	12165.04
343.15	4.82	5.037	1.1651	.00	4.82	12193.43
343.16	5.09	5.048	1.1658	.00	5.09	12221.93
343.17	5.37	5.060	1.1666	.00	5.37	12250.46
343.18	5.66	5.072	1.1673	.00	5.66	12278.93
343.19	5.96	5.083	1.1681	.00	5.96	12307.51

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.20	6.26	5.095	1.1688	.00	6.26	12336.12
343.21	6.58	5.107	1.1696	.00	6.58	12364.67
343.22	6.90	5.118	1.1703	.00	6.90	12393.33
343.23	7.23	5.130	1.1711	.00	7.23	12422.02
343.24	7.56	5.142	1.1719	.00	7.56	12450.65
343.25	7.90	5.154	1.1726	.00	7.90	12479.38
343.26	8.25	5.165	1.1734	.00	8.25	12508.15
343.27	8.61	5.177	1.1741	.00	8.61	12536.85
343.28	8.97	5.189	1.1749	.00	8.97	12565.66
343.29	9.34	5.200	1.1756	.00	9.34	12594.50
343.30	9.72	5.212	1.1764	.00	9.72	12623.28
343.31	10.10	5.224	1.1771	.00	10.10	12652.17
343.32	10.49	5.236	1.1779	.00	10.49	12681.08
343.33	10.88	5.248	1.1787	.00	10.88	12709.93
343.34	11.29	5.259	1.1794	.00	11.29	12738.89
343.35	11.69	5.271	1.1802	.00	11.69	12767.88
343.36	12.10	5.283	1.1809	.00	12.10	12796.80
343.37	12.52	5.295	1.1817	.00	12.52	12825.83
343.38	12.95	5.307	1.1825	.00	12.95	12854.89
343.39	13.38	5.318	1.1832	.00	13.38	12883.98

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.40	13.81	5.330	1.1840	.00	13.81	12912.99
343.41	14.25	5.342	1.1847	.00	14.25	12942.12
343.42	14.70	5.354	1.1855	.00	14.70	12971.28
343.43	15.15	5.366	1.1863	.00	15.15	13000.37
343.44	15.61	5.378	1.1870	.00	15.61	13029.57
343.45	16.07	5.390	1.1878	.00	16.07	13058.79
343.46	16.54	5.401	1.1885	.00	16.54	13087.95
343.47	17.01	5.413	1.1893	.00	17.01	13117.23
343.48	17.49	5.425	1.1901	.00	17.49	13146.52
343.49	17.97	5.437	1.1908	.00	17.97	13175.75
343.50	18.45	5.449	1.1916	.00	18.45	13205.09
343.51	19.03	5.461	1.1923	.00	19.03	13234.55
343.52	19.69	5.473	1.1931	.00	19.69	13264.01
343.53	20.41	5.485	1.1939	.00	20.41	13293.64
343.54	21.16	5.497	1.1946	.00	21.16	13323.32
343.55	21.96	5.509	1.1954	.00	21.96	13352.98
343.56	22.79	5.521	1.1962	.00	22.79	13382.77
343.57	23.65	5.533	1.1969	.00	23.65	13412.62
343.58	24.54	5.545	1.1977	.00	24.54	13442.42
343.59	25.46	5.557	1.1985	.00	25.46	13472.36

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.60	26.40	5.569	1.1992	.00	26.40	13502.35
343.61	27.37	5.581	1.2000	.00	27.37	13532.29
343.62	28.37	5.593	1.2007	.00	28.37	13562.36
343.63	29.38	5.605	1.2015	.00	29.38	13592.47
343.64	30.41	5.617	1.2023	.00	30.41	13622.62
343.65	31.46	5.629	1.2030	.00	31.46	13652.71
343.66	32.54	5.641	1.2038	.00	32.54	13682.94
343.67	33.63	5.653	1.2046	.00	33.63	13713.20
343.68	34.74	5.665	1.2053	.00	34.74	13743.41
343.69	35.87	5.677	1.2061	.00	35.87	13773.74
343.70	37.02	5.689	1.2069	.00	37.02	13804.12
343.71	38.18	5.701	1.2076	.00	38.18	13834.44
343.72	39.37	5.713	1.2084	.00	39.37	13864.89
343.73	40.57	5.725	1.2092	.00	40.57	13895.37
343.74	41.78	5.737	1.2099	.00	41.78	13925.79
343.75	43.02	5.749	1.2107	.00	43.02	13956.35
343.76	44.26	5.761	1.2115	.00	44.26	13986.93
343.77	45.52	5.774	1.2123	.00	45.52	14017.46
343.78	46.80	5.786	1.2130	.00	46.80	14048.11
343.79	48.09	5.798	1.2138	.00	48.09	14078.79

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
343.80	49.38	5.810	1.2146	.00	49.38	14109.41
343.81	50.69	5.822	1.2153	.00	50.69	14140.15
343.82	52.01	5.834	1.2161	.00	52.01	14170.92
343.83	53.34	5.846	1.2169	.00	53.34	14201.63
343.84	54.68	5.859	1.2176	.00	54.68	14232.44
343.85	56.03	5.871	1.2184	.00	56.03	14263.31
343.86	57.95	5.883	1.2192	.00	57.95	14294.66
343.87	58.81	5.895	1.2200	.00	58.81	14325.06
343.88	59.68	5.907	1.2207	.00	59.68	14355.49
343.89	60.56	5.920	1.2215	.00	60.56	14385.95
343.90	61.44	5.932	1.2223	.00	61.44	14416.34
343.91	62.35	5.944	1.2230	.00	62.35	14446.87
343.92	63.26	5.956	1.2238	.00	63.26	14477.41
343.93	64.18	5.968	1.2246	.00	64.18	14507.90
343.94	65.12	5.981	1.2254	.00	65.12	14538.50
343.95	66.05	5.993	1.2261	.00	66.05	14569.13
343.96	67.00	6.005	1.2269	.00	67.00	14599.70
343.97	67.96	6.018	1.2277	.00	67.96	14630.39
343.98	68.94	6.030	1.2285	.00	68.94	14661.12
343.99	69.91	6.042	1.2292	.00	69.91	14691.77

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.00	70.91	6.054	1.2300	.00	70.91	14722.54
344.01	71.91	6.067	1.2308	.00	71.91	14753.35
344.02	72.91	6.079	1.2316	.00	72.91	14784.09
344.03	73.93	6.091	1.2324	.00	73.93	14814.95
344.04	74.95	6.104	1.2332	.00	74.95	14845.84
344.05	76.00	6.116	1.2339	.00	76.00	14876.67
344.06	77.04	6.128	1.2347	.00	77.04	14907.61
344.07	78.09	6.141	1.2355	.00	78.09	14938.58
344.08	79.15	6.153	1.2363	.00	79.15	14969.49
344.09	80.23	6.165	1.2371	.00	80.23	15000.52
344.10	81.31	6.178	1.2379	.00	81.31	15031.58
344.11	82.39	6.190	1.2387	.00	82.39	15062.57
344.12	83.49	6.203	1.2395	.00	83.49	15093.68
344.13	84.60	6.215	1.2403	.00	84.60	15124.82
344.14	85.71	6.227	1.2410	.00	85.71	15155.99
344.15	86.83	6.240	1.2418	.00	86.83	15187.08
344.16	87.95	6.252	1.2426	.00	87.95	15218.30
344.17	89.09	6.265	1.2434	.00	89.09	15249.55
344.18	90.23	6.277	1.2442	.00	90.23	15280.73
344.19	91.39	6.290	1.2450	.00	91.39	15312.04

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.20	92.55	6.302	1.2458	.00	92.55	15343.37
344.21	93.72	6.314	1.2466	.00	93.72	15374.63
344.22	94.90	6.327	1.2474	.00	94.90	15406.02
344.23	96.08	6.339	1.2482	.00	96.08	15437.43
344.24	97.27	6.352	1.2490	.00	97.27	15468.77
344.25	98.48	6.364	1.2498	.00	98.48	15500.23
344.26	99.69	6.377	1.2506	.00	99.69	15531.73
344.27	100.90	6.389	1.2514	.00	100.90	15563.15
344.28	102.12	6.402	1.2521	.00	102.12	15594.69
344.29	103.35	6.414	1.2529	.00	103.35	15626.27
344.30	104.59	6.427	1.2537	.00	104.59	15657.77
344.31	105.83	6.439	1.2545	.00	105.83	15689.39
344.32	107.09	6.452	1.2553	.00	107.09	15721.05
344.33	108.34	6.465	1.2561	.00	108.34	15752.63
344.34	109.61	6.477	1.2569	.00	109.61	15784.34
344.35	110.89	6.490	1.2577	.00	110.89	15816.07
344.36	112.16	6.502	1.2585	.00	112.16	15847.73
344.37	113.45	6.515	1.2593	.00	113.45	15879.51
344.38	114.74	6.528	1.2601	.00	114.74	15911.31
344.39	116.04	6.540	1.2609	.00	116.04	15943.15

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.40	117.35	6.553	1.2617	.00	117.35	15974.92
344.41	118.66	6.565	1.2625	.00	118.66	16006.81
344.42	119.98	6.578	1.2633	.00	119.98	16038.72
344.43	121.31	6.591	1.2641	.00	121.31	16070.57
344.44	122.65	6.603	1.2649	.00	122.65	16102.53
344.45	124.00	6.616	1.2657	.00	124.00	16134.53
344.46	125.34	6.629	1.2665	.00	125.34	16166.45
344.47	126.70	6.641	1.2673	.00	126.70	16198.49
344.48	128.06	6.654	1.2681	.00	128.06	16230.56
344.49	129.43	6.667	1.2689	.00	129.43	16262.57
344.50	130.81	6.679	1.2697	.00	130.81	16294.69
344.51	132.18	6.692	1.2705	.00	132.18	16326.84
344.52	133.57	6.705	1.2713	.00	133.57	16358.92
344.53	134.97	6.717	1.2721	.00	134.97	16391.12
344.54	136.37	6.730	1.2729	.00	136.37	16423.34
344.55	137.77	6.743	1.2737	.00	137.77	16455.49
344.56	139.19	6.756	1.2745	.00	139.19	16487.78
344.57	140.61	6.768	1.2753	.00	140.61	16520.08
344.58	142.03	6.781	1.2761	.00	142.03	16552.31
344.59	143.47	6.794	1.2769	.00	143.47	16584.66

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.60	144.91	6.807	1.2777	.00	144.91	16617.05
344.61	146.35	6.819	1.2785	.00	146.35	16649.36
344.62	147.80	6.832	1.2793	.00	147.80	16681.79
344.63	149.26	6.845	1.2801	.00	149.26	16714.24
344.64	150.71	6.858	1.2809	.00	150.71	16746.71
344.65	152.18	6.871	1.2817	.00	152.18	16779.12
344.66	153.65	6.883	1.2825	.00	153.65	16811.65
344.67	155.14	6.896	1.2833	.00	155.14	16844.22
344.68	156.63	6.909	1.2841	.00	156.63	16876.71
344.69	158.12	6.922	1.2849	.00	158.12	16909.31
344.70	159.62	6.935	1.2857	.00	159.62	16941.95
344.71	161.13	6.948	1.2865	.00	161.13	16974.52
344.72	162.64	6.961	1.2873	.00	162.64	17007.20
344.73	164.15	6.973	1.2882	.00	164.15	17039.91
344.74	165.67	6.986	1.2890	.00	165.67	17072.55
344.75	167.20	6.999	1.2898	.00	167.20	17105.31
344.76	168.74	7.012	1.2906	.00	168.74	17138.10
344.77	170.27	7.025	1.2914	.00	170.27	17170.81
344.78	171.82	7.038	1.2922	.00	171.82	17203.65
344.79	173.38	7.051	1.2930	.00	173.38	17236.52

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
344.80	174.93	7.064	1.2938	.00	174.93	17269.30
344.81	176.50	7.077	1.2946	.00	176.50	17302.22
344.82	178.06	7.090	1.2954	.00	178.06	17335.16
344.83	179.63	7.103	1.2962	.00	179.63	17368.03
344.84	181.22	7.116	1.2970	.00	181.22	17401.02
344.85	182.80	7.129	1.2978	.00	182.80	17434.03
344.86	184.39	7.142	1.2986	.00	184.39	17466.97
344.87	185.99	7.155	1.2995	.00	185.99	17500.03
344.88	187.59	7.168	1.3003	.00	187.59	17533.13
344.89	189.20	7.181	1.3011	.00	189.20	17566.24
344.90	190.80	7.194	1.3019	.00	190.80	17599.28
344.91	192.42	7.207	1.3027	.00	192.42	17632.45
344.92	194.05	7.220	1.3035	.00	194.05	17665.63
344.93	195.67	7.233	1.3043	.00	195.67	17698.75
344.94	197.31	7.246	1.3051	.00	197.31	17731.99
344.95	198.94	7.259	1.3059	.00	198.94	17765.25
344.96	200.58	7.272	1.3068	.00	200.58	17798.43
344.97	202.22	7.285	1.3076	.00	202.22	17831.74
344.98	203.88	7.298	1.3084	.00	203.88	17865.08
344.99	205.54	7.311	1.3092	.00	205.54	17898.35

LEVEL POOL ROUTING DATA

HYG Dir = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow HYG file = NONE STORED - POND 2 W/ 1FTIN 2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS

Starting WS Elev = 339.00 ft
Starting Volume = .833 ac-ft
Starting Outflow = .00 cfs
Starting Infiltr. = .00 cfs
Starting Total Qout= .00 cfs
Time Increment = .0100 hrs

Elevation ft	Outflow cfs	Storage ac-ft	Area acres	Infilt. cfs	Q Total cfs	2S/t + O cfs
345.00	207.20	7.324	1.3100	.00	207.20	17931.74

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2 W/ 1FTIN  2
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 2

Pond Node   Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   339.00 ft
Starting Volume    =     .833 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =     73.80 cfs    at   11.9200 hrs
Peak Outflow      =      1.72 cfs    at   15.0500 hrs
-----------------------------------------------------
Peak Elevation    =    341.77 ft
Peak Storage =          3.500 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =        .833
+ HYG Vol IN   =       4.008
- Infiltration =        .000
- HYG Vol OUT  =       1.804
- Retained Vol =       3.037
                   ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2 W/ 1FTIN  10
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 10

Pond Node   Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   339.00 ft
Starting Volume    =     .833 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    121.54 cfs    at   11.9200 hrs
Peak Outflow      =      5.80 cfs    at   13.1700 hrs
-----------------------------------------------------
Peak Elevation    =    343.18 ft
Peak Storage =          5.077 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =        .833
+ HYG Vol IN   =       6.715
- Infiltration =        .000
- HYG Vol OUT  =       3.039
- Retained Vol =       4.509
                  ----------
Unrouted Vol =        .000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2 W/ 1FTIN  25
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 25

Pond Node   Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   339.00 ft
Starting Volume    =     .833 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    148.06 cfs    at   11.9200 hrs
Peak Outflow      =     18.12 cfs    at   12.2800 hrs
-----------------------------------------------------
Peak Elevation    =    343.49 ft
Peak Storage =          5.441 ac-ft
=====================================================


MASS BALANCE (ac-ft)
--------------------------
+ Initial Vol  =        .833
+ HYG Vol IN   =       8.259
- Infiltration =        .000
- HYG Vol OUT  =       4.454
- Retained Vol =       4.638
               ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

LEVEL POOL ROUTING SUMMARY

```
HYG Dir          = K:\02\02-330\02334.01 LF Expansion\H&H\
Inflow  HYG file = NONE STORED - POND 2 W/ 1FTIN  100
Outflow HYG file = NONE STORED - POND 2 W/ 1FTOUT 100

Pond Node   Data = POND 2 W/ 1FT
Pond Volume Data = POND 2 W/ 1FT
Pond Outlet Data = Riser/Barrelw1ft

No Infiltration

INITIAL CONDITIONS
----------------------------------
Starting WS Elev   =   339.00 ft
Starting Volume    =     .833 ac-ft
Starting Outflow   =      .00 cfs
Starting Infiltr.  =      .00 cfs
Starting Total Qout=      .00 cfs
Time Increment     =    .0100 hrs


INFLOW/OUTFLOW HYDROGRAPH SUMMARY
=====================================================
Peak Inflow       =    190.30 cfs    at   11.9200 hrs
Peak Outflow      =     81.80 cfs    at   12.0600 hrs
-----------------------------------------------------
Peak Elevation    =    344.10 ft
Peak Storage =          6.183 ac-ft
=====================================================


MASS BALANCE (ac-ft)
---------------------------
+ Initial Vol  =        .833
+ HYG Vol IN   =      10.759
- Infiltration =        .000
- HYG Vol OUT  =       6.808
- Retained Vol =       4.783
                 ----------
Unrouted Vol =       -.000 ac-ft  (.000% of Inflow Volume)
```

Index of Starting Page Numbers for ID Names

----- H -----
Harnett County... 3.01

----- L -----
LF AREA 1... 4.01, 5.01, 6.03, 6.04,
6.05, 6.06
LF AREA 2... 4.03, 5.02, 6.07, 6.08,
6.09, 6.10
LF AREA1... 4.05, 5.03
LF AREA2... 4.07, 5.04

----- P -----
POND 2... 7.01, 9.01, 9.37, 9.38,
9.39, 9.40
POND 2 W/ 1FT... 7.02, 9.41, 9.77,
9.78, 9.79, 9.80
PRE DEV.... 4.09, 5.05, 6.11, 6.12,
6.13, 6.14

----- R -----
Riser/Barrel... 8.01, 8.05
Riser/Barrelw1ft... 8.24, 8.27

----- S -----
SP-2 DA... 4.11, 5.06, 6.15, 6.16,
6.17, 6.18
SP-2DA... 4.13, 5.07

----- W -----
Watershed... 1.01, 2.01, 2.03, 2.05,
2.07

Minimum Drain Time

==========
JOB TITLE
==========

Project Date: 10/4/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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********** TIME VS.ELEV **********

MIN DRAIN TIMOUT Time-Elev 1.01

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

TIME vs. ELEVATION (ft)

Output Time increment = .5000 hrs
Time on left represents time for first value in each row.

Time hrs					
.0000	339.72	339.64	339.56	339.48	339.41
2.5000	339.34	339.28	339.22	339.17	339.12
5.0000	339.07	339.04	339.00	338.96	338.93
7.5000	338.90	338.86	338.83	338.80	338.77
10.0000	338.74	338.71	338.69	338.66	338.63
12.5000	338.60	338.57	338.54	338.51	338.48
15.0000	338.46	338.43	338.40	338.37	338.35
17.5000	338.33	338.31	338.30	338.29	338.27
20.0000	338.26	338.25	338.24	338.23	338.22
22.5000	338.21	338.20	338.19	338.18	338.18
25.0000	338.17	338.17	338.16	338.16	338.16
27.5000	338.15	338.15	338.15	338.15	338.15
30.0000	338.15	338.15	338.15	338.15	338.14
32.5000	338.14	338.14	338.14	338.14	338.14
35.0000	338.14	338.14	338.14	338.14	338.13
37.5000	338.13	338.13	338.13	338.13	338.13
40.0000	338.13	338.13	338.13	338.13	338.13
42.5000	338.12	338.12	338.12	338.12	338.12
45.0000	338.12	338.12	338.12	338.12	338.12
47.5000	338.12	338.12	338.11	338.11	338.11
50.0000	338.11	338.11	338.11	338.11	338.11
52.5000	338.11	338.11	338.11	338.11	338.11
55.0000	338.11	338.11	338.11	338.11	338.11
57.5000	338.11	338.11	338.11	338.11	338.11
60.0000	338.11	338.11	338.11	338.11	338.11
62.5000	338.11	338.11	338.11	338.11	338.11
65.0000	338.11	338.11	338.11	338.11	338.11
67.5000	338.11	338.11	338.11	338.11	338.11
70.0000	338.11	338.11	338.11	338.11	338.11

Index of Starting Page Numbers for ID Names

----- M -----
MIN DRAIN TIMOUT... 1.01

First Flush Volume

==========
JOB TITLE
==========

Project Date: 10/4/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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********** BMP (BEST MGT. PRACTICES) **********

FIRST FLUSH..... 1st Flush 1.01

File.... K:\02\02-330\02334.01 LF Expansion\H&H\SEDIMENTATION POND 2-EC.PPW

BMP FIRST FLUSH CALCULATIONS

First Flush Depth = 1.0000 in
Drainage Area = 19.370 acres

Volume = Flush Depth * Drainage Area

First Flush volume = 1.614 ac-ft

Index of Starting Page Numbers for ID Names

----- F -----
FIRST FLUSH... 1.01

PondPack Ditch Sizing Results

==========
JOB TITLE
==========

Project Date: 9/23/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

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********** MASTER SUMMARY **********

Watershed..... Master Network Summary 1.01

********** NETWORK SUMMARIES (DETAILED) **********

Watershed..... 2-yr
Executive Summary (Links) 2.01

Watershed..... 10-yr
Executive Summary (Links) 2.03

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Executive Summary (Links) 2.05

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MASTER DESIGN STORM SUMMARY

Network Storm Collection: Harnett County

Return Event	Total Depth in	Rainfall Type	RNF ID
2-yr	3.7500	Synthetic Curve	TypeII 24hr
10-yr	5.6000	Synthetic Curve	TypeII 24hr
25-yr	6.6000	Synthetic Curve	TypeII 24hr
100-yr	8.2000	Synthetic Curve	TypeII 24hr

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
*AREA 1&2 OUT	JCT	2	3.381		11.9200	63.33		
*AREA 1&2 OUT	JCT	10	5.871		11.9200	107.81		
*AREA 1&2 OUT	JCT	25	7.257		11.9200	131.86		
*AREA 1&2 OUT	JCT	100	9.505		11.9200	170.15		
*AREA 3 OUT	JCT	2	1.917		11.9200	35.90		
*AREA 3 OUT	JCT	10	3.328		11.9200	61.12		
*AREA 3 OUT	JCT	25	4.114		11.9200	74.76		
*AREA 3 OUT	JCT	100	5.389		11.9200	96.46		
*AREA 4A OUT	JCT	2	.540		11.9200	10.11		
*AREA 4A OUT	JCT	10	.937		11.9200	17.21		
*AREA 4A OUT	JCT	25	1.158		11.9200	21.05		
*AREA 4A OUT	JCT	100	1.517		11.9200	27.16		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
*AREA 5A	JCT	2	1.870		11.9200	35.04		
*AREA 5A	JCT	10	3.248		11.9200	59.64		
*AREA 5A	JCT	25	4.014		11.9200	72.95		
*AREA 5A	JCT	100	5.258		11.9200	94.13		
*AREA4B&5 OUT	JCT	2	3.456		11.9200	64.74		
*AREA4B&5 OUT	JCT	10	6.002		11.9200	110.22		
*AREA4B&5 OUT	JCT	25	7.419		11.9200	134.80		
*AREA4B&5 OUT	JCT	100	9.717		11.9200	173.95		
LF AREA 1	AREA	2	1.551		11.9200	29.06		
LF AREA 1	AREA	10	2.694		11.9200	49.46		
LF AREA 1	AREA	25	3.329		11.9200	60.50		
LF AREA 1	AREA	100	4.361		11.9200	78.07		
LF AREA 2	AREA	2	1.830		11.9200	34.27		
LF AREA 2	AREA	10	3.177		11.9200	58.35		
LF AREA 2	AREA	25	3.927		11.9200	71.36		
LF AREA 2	AREA	100	5.144		11.9200	92.08		
LF AREA 3	AREA	2	1.917		11.9200	35.90		
LF AREA 3	AREA	10	3.328		11.9200	61.12		
LF AREA 3	AREA	25	4.114		11.9200	74.76		
LF AREA 3	AREA	100	5.389		11.9200	96.46		
LF AREA 4A	AREA	2	.540		11.9200	10.11		
LF AREA 4A	AREA	10	.937		11.9200	17.21		
LF AREA 4A	AREA	25	1.158		11.9200	21.05		
LF AREA 4A	AREA	100	1.517		11.9200	27.16		
LF AREA 4B	AREA	2	.364		11.9200	6.81		
LF AREA 4B	AREA	10	.631		11.9200	11.60		
LF AREA 4B	AREA	25	.780		11.9200	14.18		
LF AREA 4B	AREA	100	1.022		11.9200	18.30		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
LF AREA 5	AREA	2	3.093		11.9200	57.93		
LF AREA 5	AREA	10	5.370		11.9200	98.62		
LF AREA 5	AREA	25	6.638		11.9200	120.62		
LF AREA 5	AREA	100	8.695		11.9200	155.65		
LF AREA 5A	AREA	2	1.870		11.9200	35.04		
LF AREA 5A	AREA	10	3.248		11.9200	59.64		
LF AREA 5A	AREA	25	4.014		11.9200	72.95		
LF AREA 5A	AREA	100	5.258		11.9200	94.13		
PH3A-PIPE	AREA	2	1.151		11.9200	21.56		
PH3A-PIPE	AREA	10	1.998		11.9200	36.70		
PH3A-PIPE	AREA	25	2.470		11.9200	44.88		
PH3A-PIPE	AREA	100	3.235		11.9200	57.92		
*PH3A1 OUT	JCT	2	.689		11.9200	12.90		
*PH3A1 OUT	JCT	10	1.196		11.9200	21.96		
*PH3A1 OUT	JCT	25	1.478		11.9200	26.86		
*PH3A1 OUT	JCT	100	1.936		11.9200	34.65		
*PH3A2 OUT	JCT	2	.899		11.9200	16.85		
*PH3A2 OUT	JCT	10	1.562		11.9200	28.68		
*PH3A2 OUT	JCT	25	1.930		11.9200	35.08		
*PH3A2 OUT	JCT	100	2.529		11.9200	45.26		
*PH3B OUT	JCT	2	1.675		11.9600	28.64		
*PH3B OUT	JCT	10	2.909		11.9600	48.73		
*PH3B OUT	JCT	25	3.596		11.9600	59.59		
*PH3B OUT	JCT	100	4.710		11.9400	76.91		
PHASE 3A-1	AREA	2	.689		11.9200	12.90		
PHASE 3A-1	AREA	10	1.196		11.9200	21.96		
PHASE 3A-1	AREA	25	1.478		11.9200	26.86		
PHASE 3A-1	AREA	100	1.936		11.9200	34.65		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
PHASE 3A-2	AREA	2	.899		11.9200	16.85		
PHASE 3A-2	AREA	10	1.562		11.9200	28.68		
PHASE 3A-2	AREA	25	1.930		11.9200	35.08		
PHASE 3A-2	AREA	100	2.529		11.9200	45.26		
PHASE 3B	AREA	2	1.675		11.9600	28.64		
PHASE 3B	AREA	10	2.909		11.9600	48.73		
PHASE 3B	AREA	25	3.596		11.9600	59.59		
PHASE 3B	AREA	100	4.710		11.9400	76.91		
*PIPE3A OUT	JCT	2	1.151		11.9200	21.56		
*PIPE3A OUT	JCT	10	1.998		11.9200	36.70		
*PIPE3A OUT	JCT	25	2.470		11.9200	44.88		
*PIPE3A OUT	JCT	100	3.235		11.9200	57.92		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 2-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 2 yr
Total Rainfall Depth= 3.7500 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	1.917		11.9200	35.90	LF AREA 3
		DL	1.917		11.9200	35.90	
		DN	1.917		11.9200	35.90	AREA 3 OUT
ADDLINK 100	ADD	UN	.689		11.9200	12.90	PHASE 3A-1
		DL	.689		11.9200	12.90	
		DN	.689		11.9200	12.90	PH3A1 OUT
ADDLINK 110	ADD	UN	.899		11.9200	16.85	PHASE 3A-2
		DL	.899		11.9200	16.85	
		DN	.899		11.9200	16.85	PH3A2 OUT
ADDLINK 20	ADD	UN	1.551		11.9200	29.06	LF AREA 1
		DL	1.551		11.9200	29.06	
		DN	3.381		11.9200	63.33	AREA 1&2 OUT
ADDLINK 30	ADD	UN	1.830		11.9200	34.27	LF AREA 2
		DL	1.830		11.9200	34.27	
		DN	3.381		11.9200	63.33	AREA 1&2 OUT
ADDLINK 40	ADD	UN	1.870		11.9200	35.04	LF AREA 5A
		DL	1.870		11.9200	35.04	
		DN	1.870		11.9200	35.04	AREA 5A
ADDLINK 50	ADD	UN	1.675		11.9600	28.64	PHASE 3B
		DL	1.675		11.9600	28.64	
		DN	1.675		11.9600	28.64	PH3B OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	1.151		11.9200	21.56	PH3A-PIPE
		DL	1.151		11.9200	21.56	
		DN	1.151		11.9200	21.56	PIPE3A OUT
ADDLINK 70	ADD	UN	.364		11.9200	6.81	LF AREA 4B
		DL	.364		11.9200	6.81	
		DN	3.456		11.9200	64.74	AREA4B&5 OUT
ADDLINK 80	ADD	UN	3.093		11.9200	57.93	LF AREA 5
		DL	3.093		11.9200	57.93	
		DN	3.456		11.9200	64.74	AREA4B&5 OUT
ADDLINK 90	ADD	UN	.540		11.9200	10.11	LF AREA 4A
		DL	.540		11.9200	10.11	
		DN	.540		11.9200	10.11	AREA 4A OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 10-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 10 yr
Total Rainfall Depth= 5.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	3.328		11.9200	61.12	LF AREA 3
		DL	3.328		11.9200	61.12	
		DN	3.328		11.9200	61.12	AREA 3 OUT
ADDLINK 100	ADD	UN	1.196		11.9200	21.96	PHASE 3A-1
		DL	1.196		11.9200	21.96	
		DN	1.196		11.9200	21.96	PH3A1 OUT
ADDLINK 110	ADD	UN	1.562		11.9200	28.68	PHASE 3A-2
		DL	1.562		11.9200	28.68	
		DN	1.562		11.9200	28.68	PH3A2 OUT
ADDLINK 20	ADD	UN	2.694		11.9200	49.46	LF AREA 1
		DL	2.694		11.9200	49.46	
		DN	5.871		11.9200	107.81	AREA 1&2 OUT
ADDLINK 30	ADD	UN	3.177		11.9200	58.35	LF AREA 2
		DL	3.177		11.9200	58.35	
		DN	5.871		11.9200	107.81	AREA 1&2 OUT
ADDLINK 40	ADD	UN	3.248		11.9200	59.64	LF AREA 5A
		DL	3.248		11.9200	59.64	
		DN	3.248		11.9200	59.64	AREA 5A
ADDLINK 50	ADD	UN	2.909		11.9600	48.73	PHASE 3B
		DL	2.909		11.9600	48.73	
		DN	2.909		11.9600	48.73	PH3B OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	1.998		11.9200	36.70	PH3A-PIPE
		DL	1.998		11.9200	36.70	
		DN	1.998		11.9200	36.70	PIPE3A OUT
ADDLINK 70	ADD	UN	.631		11.9200	11.60	LF AREA 4B
		DL	.631		11.9200	11.60	
		DN	6.002		11.9200	110.22	AREA4B&5 OUT
ADDLINK 80	ADD	UN	5.370		11.9200	98.62	LF AREA 5
		DL	5.370		11.9200	98.62	
		DN	6.002		11.9200	110.22	AREA4B&5 OUT
ADDLINK 90	ADD	UN	.937		11.9200	17.21	LF AREA 4A
		DL	.937		11.9200	17.21	
		DN	.937		11.9200	17.21	AREA 4A OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 25-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 25 yr
Total Rainfall Depth= 6.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	4.114		11.9200	74.76	LF AREA 3
		DL	4.114		11.9200	74.76	
		DN	4.114		11.9200	74.76	AREA 3 OUT
ADDLINK 100	ADD	UN	1.478		11.9200	26.86	PHASE 3A-1
		DL	1.478		11.9200	26.86	
		DN	1.478		11.9200	26.86	PH3A1 OUT
ADDLINK 110	ADD	UN	1.930		11.9200	35.08	PHASE 3A-2
		DL	1.930		11.9200	35.08	
		DN	1.930		11.9200	35.08	PH3A2 OUT
ADDLINK 20	ADD	UN	3.329		11.9200	60.50	LF AREA 1
		DL	3.329		11.9200	60.50	
		DN	7.257		11.9200	131.86	AREA 1&2 OUT
ADDLINK 30	ADD	UN	3.927		11.9200	71.36	LF AREA 2
		DL	3.927		11.9200	71.36	
		DN	7.257		11.9200	131.86	AREA 1&2 OUT
ADDLINK 40	ADD	UN	4.014		11.9200	72.95	LF AREA 5A
		DL	4.014		11.9200	72.95	
		DN	4.014		11.9200	72.95	AREA 5A
ADDLINK 50	ADD	UN	3.596		11.9600	59.59	PHASE 3B
		DL	3.596		11.9600	59.59	
		DN	3.596		11.9600	59.59	PH3B OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	2.470		11.9200	44.88	PH3A-PIPE
		DL	2.470		11.9200	44.88	
		DN	2.470		11.9200	44.88	PIPE3A OUT
ADDLINK 70	ADD	UN	.780		11.9200	14.18	LF AREA 4B
		DL	.780		11.9200	14.18	
		DN	7.419		11.9200	134.80	AREA4B&5 OUT
ADDLINK 80	ADD	UN	6.638		11.9200	120.62	LF AREA 5
		DL	6.638		11.9200	120.62	
		DN	7.419		11.9200	134.80	AREA4B&5 OUT
ADDLINK 90	ADD	UN	1.158		11.9200	21.05	LF AREA 4A
		DL	1.158		11.9200	21.05	
		DN	1.158		11.9200	21.05	AREA 4A OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = Harnett County

Storm Tag Name = 100-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 100 yr
Total Rainfall Depth= 8.2000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 10	ADD	UN	5.389		11.9200	96.46	LF AREA 3
		DL	5.389		11.9200	96.46	
		DN	5.389		11.9200	96.46	AREA 3 OUT
ADDLINK 100	ADD	UN	1.936		11.9200	34.65	PHASE 3A-1
		DL	1.936		11.9200	34.65	
		DN	1.936		11.9200	34.65	PH3A1 OUT
ADDLINK 110	ADD	UN	2.529		11.9200	45.26	PHASE 3A-2
		DL	2.529		11.9200	45.26	
		DN	2.529		11.9200	45.26	PH3A2 OUT
ADDLINK 20	ADD	UN	4.361		11.9200	78.07	LF AREA 1
		DL	4.361		11.9200	78.07	
		DN	9.505		11.9200	170.15	AREA 1&2 OUT
ADDLINK 30	ADD	UN	5.144		11.9200	92.08	LF AREA 2
		DL	5.144		11.9200	92.08	
		DN	9.505		11.9200	170.15	AREA 1&2 OUT
ADDLINK 40	ADD	UN	5.258		11.9200	94.13	LF AREA 5A
		DL	5.258		11.9200	94.13	
		DN	5.258		11.9200	94.13	AREA 5A
ADDLINK 50	ADD	UN	4.710		11.9400	76.91	PHASE 3B
		DL	4.710		11.9400	76.91	
		DN	4.710		11.9400	76.91	PH3B OUT

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	3.235		11.9200	57.92	PH3A-PIPE
		DL	3.235		11.9200	57.92	
		DN	3.235		11.9200	57.92	PIPE3A OUT
ADDLINK 70	ADD	UN	1.022		11.9200	18.30	LF AREA 4B
		DL	1.022		11.9200	18.30	
		DN	9.717		11.9200	173.95	AREA4B&5 OUT
ADDLINK 80	ADD	UN	8.695		11.9200	155.65	LF AREA 5
		DL	8.695		11.9200	155.65	
		DN	9.717		11.9200	173.95	AREA4B&5 OUT
ADDLINK 90	ADD	UN	1.517		11.9200	27.16	LF AREA 4A
		DL	1.517		11.9200	27.16	
		DN	1.517		11.9200	27.16	AREA 4A OUT

File.... K:\02\02-330\02334.01 LF Expansion\H&H\
Title... Project Date: 9/23/2004
Project Engineer: wr
Project Title: Watershed
Project Comments:

DESIGN STORMS SUMMARY

Design Storm File,ID = Harnett County

Storm Tag Name = 2-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 2 yr
Total Rainfall Depth= 3.7500 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 10-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 10 yr
Total Rainfall Depth= 5.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 25-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 25 yr
Total Rainfall Depth= 6.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

Storm Tag Name = 100-yr

Data Type, File, ID = Synthetic Storm TypeII 24hr
Storm Frequency = 100 yr
Total Rainfall Depth= 8.2000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .1000 hrs End= 24.0000 hrs

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

::
TIME OF CONCENTRATION CALCULATOR
::

Segment #1: Tc: User Defined

Segment #1 Time: .0830 hrs

==========
Total Tc: .0830 hrs

Calculated Tc < Min.Tc:
Use Minimum Tc...
Use Tc = .0833 hrs
==========

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

Tc Equations used...

==== User Defined ==========

Tc = Value entered by user

Where: Tc = Time of concentration

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	8.020			86.00

COMPOSITE AREA & WEIGHTED CN ---> 8.020 86.00 (86)
::

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	86	9.460			86.00
COMPOSITE AREA & WEIGHTED CN --->		9.460			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	Impervious Adjustment %UC	Adjusted CN
	86	9.910			86.00

COMPOSITE AREA & WEIGHTED CN ---> 9.910 86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	2.790			86.00
COMPOSITE AREA & WEIGHTED CN --->		2.790			86.00 (86)

::

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	1.880			86.00

COMPOSITE AREA & WEIGHTED CN ---> 1.880 86.00 (86)

Name.... LF AREA 5

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	15.990			86.00
COMPOSITE AREA & WEIGHTED CN --->		15.990			86.00 (86)

File.... K:\02\02-330\02334.01 LF Expansion\H&H\DITCH SIZING.PPW

RUNOFF CURVE NUMBER DATA
::

Soil/Surface Description	CN	Area acres	Impervious Adjustment %C	%UC	Adjusted CN
	86	9.670			86.00

COMPOSITE AREA & WEIGHTED CN ---> 9.670 86.00 (86)
::

SCS UNIT HYDROGRAPH METHOD
(Computational Notes)

DEFINITION OF TERMS: ----------

```
  At    = Total area (acres):  At = Ai+Ap
  Ai    = Impervious area (acres)
  Ap    = Pervious area (acres)
  CNi   = Runoff curve number for impervious area
  CNp   = Runoff curve number for pervious area
  fLoss = f loss constant infiltration (depth/time)
  gKs   = Saturated Hydraulic Conductivity (depth/time)
  Md    = Volumetric Moisture Deficit
  Psi   = Capillary Suction (length)
  hK    = Horton Infiltration Decay Rate (time^-1)
  fo    = Initial Infiltration Rate (depth/time)
  fc    = Ultimate(capacity)Infiltration Rate (depth/time)
  Ia    = Initial Abstraction (length)
  dt    = Computational increment (duration of unit excess rainfall)
          Default dt is smallest value of 0.1333Tc, rtm, and th
          (Smallest dt is then adjusted to match up with Tp)
UDdt    = User specified override computational main time increment
          (only used if UDdt is => .1333Tc)
  D(t)  = Point on distribution curve (fraction of P) for time step t

  K     = 2 / (1 + (Tr/Tp)):  default K = 0.75: (for Tr/Tp = 1.67)
  Ks    = Hydrograph shape factor
        = Unit Conversions * K:
        = ((1hr/3600sec) * (1ft/12in) * ((5280ft)**2/sq.mi)) * K
          Default Ks = 645.333 * 0.75 = 484

  Lag   = Lag time from center of excess runoff (dt) to Tp:  Lag = 0.6Tc
  P     = Total precipitation depth, inches
  Pa(t) = Accumulated rainfall at time step t
  Pi(t) = Incremental rainfall at time step t
  qp    = Peak discharge (cfs) for 1in. runoff, for 1hr, for 1 sq.mi.
        = (Ks * A * Q) / Tp   (where Q = 1in. runoff, A=sq.mi.)
  Qu(t) = Unit hydrograph ordinate (cfs) at time step t
  Q(t)  = Final hydrograph ordinate (cfs) at time step t
  Rai(t)= Accumulated runoff (inches) at time step t for impervious area
  Rap(t)= Accumulated runoff (inches) at time step t for pervious area
  Rii(t)= Incremental runoff (inches) at time step t for impervious area
  Rip(t)= Incremental runoff (inches) at time step t for pervious area
  R(t)  = Incremental weighted total runoff (inches)
  Rtm   = Time increment for rainfall table
  Si    = S for impervious area:  Si = (1000/CNi) - 10
  Sp    = S for pervious area:    Sp = (1000/CNp) - 10
  t     = Time step (row) number
  Tc    = Time of concentration
  Tb    = Time (hrs) of entire unit hydrograph:  Tb = Tp + Tr
  Tp    = Time (hrs) to peak of a unit hydrograph:  Tp = (dt/2) + Lag
  Tr    = Time (hrs) of receding limb of unit hydrograph:  Tr = ratio of Tp
```

SCS UNIT HYDROGRAPH METHOD
(Computational Notes)

PRECIPITATION: ----------
Column (1): Time for time step t
Column (2): D(t) = Point on distribution curve for time step t
Column (3): Pi(t) = Pa(t) - Pa(t-1): Col.(4) - Preceding Col.(4)
Column (4): Pa(t) = D(t) x P: Col.(2) x P

PERVIOUS AREA RUNOFF (using SCS Runoff CN Method) ----------
Column (5): Rap(t) = Accumulated pervious runoff for time step t
If (Pa(t) is <= 0.2Sp) then use: Rap(t) = 0.0
If (Pa(t) is > 0.2Sp) then use:

Rap(t) = (Col.(4)-0.2Sp)**2 / (Col.(4)+0.8Sp)

Column (6): Rip(t) = Incremental pervious runoff for time step t
Rip(t) = Rap(t) - Rap(t-1)
Rip(t) = Col.(5) for current row - Col.(5) for preceding row.

IMPERVIOUS AREA RUNOFF ----------
Column (7 & 8)... Did not specify to use impervious areas.

INCREMENTAL WEIGHTED RUNOFF: ----------
Column (9): R(t) = (Ap/At) x Rip(t) + (Ai/At) x Rii(t)
R(t) = (Ap/At) x Col.(6) + (Ai/At) x Col.(8)

SCS UNIT HYDROGRAPH METHOD: ----------
Column (10): Q(t) is computed with the SCS unit hydrograph method
using R() and Qu().

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    29.07 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    29.06 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 1
               CN   =   86
               Area =       8.020 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        1.551 ac-ft

HYG Volume...          1.551 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    49.54 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    49.46 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 1
              CN   =   86
              Area =       8.020 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       2.694 ac-ft

HYG Volume...          2.694 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

==========================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    60.61 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    60.50 cfs
==========================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 1
               CN   =   86
               Area =       8.020 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        3.330 ac-ft

HYG Volume...          3.329 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 1 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 8.020 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    78.23 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    78.07 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 1
              CN   =   86
              Area =       8.020 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       4.361 ac-ft

HYG Volume...          4.361 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 1)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   109.09 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    34.30 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    34.27 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 2
               CN   =   86
               Area =       9.460 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        1.830 ac-ft

HYG Volume...          1.830 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =     .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    58.43 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    58.35 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:LF AREA 2
             CN   =   86
             Area =       9.460 acres
             S    =   1.6279 in
             0.2S =    .3256 in

              Cumulative Runoff
             -------------------
                     4.0305 in
                      3.177 ac-ft

HYG Volume...          3.177 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    71.49 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    71.36 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 2
               CN   =   86
               Area =       9.460 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        3.927 ac-ft

HYG Volume...           3.927 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 2 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.460 acres  Runoff CN= 86

==========================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    92.28 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    92.08 cfs
==========================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 2
              CN   =   86
              Area =       9.460 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       5.144 ac-ft

HYG Volume...          5.144 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 2)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   128.67 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    35.93 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    35.90 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 3
               CN    =   86
               Area =       9.910 acres
               S     =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        1.917 ac-ft

HYG Volume...           1.917 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   134.80 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    61.21 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    61.12 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 3
              CN   =   86
              Area =       9.910 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       3.328 ac-ft

HYG Volume...          3.328 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   134.80 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    74.89 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    74.76 cfs
=============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 3
               CN   =   86
               Area =       9.910 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        4.114 ac-ft

HYG Volume...           4.114 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   134.80 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 3 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.910 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    96.67 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    96.46 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:LF AREA 3
             CN    =   86
             Area =       9.910 acres
             S     =   1.6279 in
             0.2S =    .3256 in

              Cumulative Runoff
             -------------------
                     6.5254 in
                      5.389 ac-ft

HYG Volume...          5.389 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 3)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   134.80 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4A 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.790 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    10.11 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    10.11 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 4A
               CN   =   86
               Area =       2.790 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                         .540 ac-ft

HYG Volume...            .540 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    37.95 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4A 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.790 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    17.23 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    17.21 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:LF AREA 4A
             CN    =   86
             Area =       2.790 acres
             S     =   1.6279 in
             0.2S =    .3256 in

              Cumulative Runoff
             -------------------
                     4.0305 in
                       .937 ac-ft

HYG Volume...           .937 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    37.95 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4A 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.790 acres  Runoff CN= 86

==========================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    21.08 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    21.05 cfs
==========================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 4A
               CN   =   86
               Area =       2.790 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                        1.158 ac-ft

HYG Volume...          1.158 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    37.95 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4A 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 2.790 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    27.21 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    27.16 cfs
============================================

                 DRAINAGE AREA
              -------------------
              ID:LF AREA 4A
              CN   =   86
              Area =       2.790 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       1.517 ac-ft

HYG Volume...          1.517 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    37.95 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4B 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.880 acres  Runoff CN= 86

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =     6.82 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =     6.81 cfs
=============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 4B
               CN    =   86
               Area =       1.880 acres
               S     =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                         .364 ac-ft

HYG Volume...            .364 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4B)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.57 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4B 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.880 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    11.61 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    11.60 cfs
============================================

                DRAINAGE AREA
             -------------------
             ID:LF AREA 4B
             CN   =   86
             Area =       1.880 acres
             S    =   1.6279 in
             0.2S =    .3256 in

              Cumulative Runoff
             -------------------
                     4.0305 in
                       .631 ac-ft

HYG Volume...           .631 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4B)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.57 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4B 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.880 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    14.21 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    14.18 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 4B
               CN   =   86
               Area =       1.880 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       4.9819 in
                         .780 ac-ft

HYG Volume...            .780 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4B)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.57 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 4B 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 1.880 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    18.34 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    18.30 cfs
============================================

                 DRAINAGE AREA
              -------------------
              ID:LF AREA 4B
              CN   =   86
              Area =       1.880 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       1.022 ac-ft

HYG Volume...          1.022 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 4B)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =    25.57 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 15.990 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    57.97 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    57.93 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 5
               CN   =   86
               Area =      15.990 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        3.093 ac-ft

HYG Volume...          3.093 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =     .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =    .7491)

Unit peak,          qp =   217.50 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 15.990 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    98.77 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    98.62 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 5
              CN    =   86
              Area =      15.990 acres
              S     =   1.6279 in
              0.2S =     .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       5.371 ac-ft

HYG Volume...          5.370 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,           qp =   217.50 cfs
Unit peak time       Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,     Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 15.990 acres  Runoff CN= 86

=============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =   120.84 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =   120.62 cfs
=============================================

                 DRAINAGE AREA
              -------------------
              ID:LF AREA 5
              CN   =   86
              Area =      15.990 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.9819 in
                       6.638 ac-ft

HYG Volume...          6.638 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   217.50 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 15.990 acres  Runoff CN= 86

==========================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =   155.97 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =   155.65 cfs
==========================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 5
              CN   =   86
              Area =      15.990 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       8.695 ac-ft

HYG Volume...          8.695 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   217.50 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 2 year storm
Duration      = 24.0000 hrs     Rain Depth = 3.7500 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5A 2-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.670 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    35.06 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    35.04 cfs
============================================

                 DRAINAGE AREA
               -------------------
               ID:LF AREA 5A
               CN   =   86
               Area =       9.670 acres
               S    =   1.6279 in
               0.2S =    .3256 in

                Cumulative Runoff
               -------------------
                       2.3210 in
                        1.870 ac-ft

HYG Volume...           1.870 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   131.53 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 10 year storm
Duration      = 24.0000 hrs     Rain Depth = 5.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5A 10-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.670 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    59.73 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    59.64 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 5A
              CN   =   86
              Area =       9.670 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.0305 in
                       3.248 ac-ft

HYG Volume...          3.248 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =    .7491)

Unit peak,          qp =   131.53 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 25 year storm
Duration      = 24.0000 hrs     Rain Depth = 6.6000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5A 25-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.670 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    73.08 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    72.95 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 5A
              CN   =   86
              Area =       9.670 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      4.9819 in
                       4.015 ac-ft

HYG Volume...          4.014 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =    .7491)

Unit peak,          qp =   131.53 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

```
SCS UNIT HYDROGRAPH METHOD

STORM EVENT: 100 year storm
Duration      = 24.0000 hrs     Rain Depth = 8.2000 in
Rain Dir      = K:\02\02-330\02334.01 LF Expansion\H&H\
Rain File -ID =  - TypeII  24hr
Unit Hyd Type = Default Curvilinear
HYG Dir       = K:\02\02-330\02334.01 LF Expansion\H&H\
HYG File - ID =  - LF AREA 5A 100-yr
Tc (Min. Tc)  = .0833 hrs
Drainage Area = 9.670 acres  Runoff CN= 86

============================================
Computational Time Increment   =   .01111 hrs
Computed Peak Time             =  11.9175 hrs
Computed Peak Flow             =    94.32 cfs

Time Increment for HYG File    =    .0200 hrs
Peak Time, Interpolated Output =  11.9201 hrs
Peak Flow, Interpolated Output =    94.13 cfs
============================================

                DRAINAGE AREA
              -------------------
              ID:LF AREA 5A
              CN   =   86
              Area =       9.670 acres
              S    =   1.6279 in
              0.2S =    .3256 in

               Cumulative Runoff
              -------------------
                      6.5254 in
                       5.258 ac-ft

HYG Volume...          5.258 ac-ft (area under HYG curve)

***** SCS UNIT HYDROGRAPH PARAMETERS *****

Time Concentration, Tc =   .08330 hrs (ID: LF AREA 5A)
Computational Incr, Tm =   .01111 hrs = 0.20000 Tp

Unit Hyd. Shape Factor =  483.432 (37.46% under rising limb)
K =  483.43/645.333,  K =    .7491 (also, K = 2/(1+(Tr/Tp))
Receding/Rising, Tr/Tp =   1.6698 (solved from K =   .7491)

Unit peak,          qp =   131.53 cfs
Unit peak time      Tp =   .05553 hrs
Unit receding limb, Tr =   .22213 hrs
Total unit time,    Tb =   .27767 hrs
```

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