CDM
5400 Glenwood Avenue, Suite 300
Raleigh, North Carolina 27612
tel: 919 787-5620
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September 24, 2008

Mr. Ed Mussler, P.E.
North Carolina Department of Natural Resources
Solid Waste Section
Division of Solid Waste Management
401 Oberlin Road Raleigh, NC 27605

Subject: Wake County, North Carolina
North Wake Landfill Phase 2 Closure
CDM Project No. 6172-55752
Post Closure Permit Modification

Dear Ed:

As discussed during out meeting on September 4, 2008 CDM is presenting herewith, and behalf of Wake County the Leachate Pond Decommissioning Plan for the Permit Modification to the Post Closure Plan for your review. Included with the plan is a check for the $3,000.00 to cover the fee for the Permit Modification. The complete Permit Modification to the Post Closure Plan to which the Leachate Pond Decommissioning Plan will be attached, will follow under separate cover.

Should you have any questions or comments, please contact me at (919) 787-5620.

Very truly yours,

W. Michael Brinchek, P.E
Project Manager
Camp Dresser & McKee

cc: J. Beal, Wake County
J. Roberson, Wake County
E. Staehle, Wake County
B. Starkey, OBS
B. Brossoie, CDM
B. Buckley, CDM

Appendix A
Leachate Pond Decommissioning

Introduction

The existing leachate storage pond at the North Wake Subtitle D Landfill (landfill) is a lined storage pond used for temporary containment of the leachate generated from the Subtitle D landfill. The leachate storage pond liner system is an impervious liner system that includes from bottom to top:

- a 24-inch-thick compacted clay liner (hydraulic conductivity (k_v) $\leq 1 \times 10^{-7}$ centimeters per second (cm/sec));
- a 60-mil high density polyethylene (HDPE) geomembrane; and
- a concrete fabric formed lined protective layer.

Leachate from the leachate storage pond gravity drains into a leachate pump station west of the existing leachate storage pond, which discharges into the City of Raleigh Sanitary Sewer System as allowed under Permit No. NC0029033 (**See Attachment 1**). As permitted, Wake County is allowed to discharge at a rate up to 125,000 gallons per day (gpd) to the City of Raleigh Sanitary Sewer System provided that pH is between 6.0 and 10.0 standard units and the sum of total toxic organic compounds (TTO) with detected concentrations greater than .01 mg/L is less than 2.13 mg/L.

Completion of the North Wake Landfill closure will eliminate stormwater infiltration, consequently reducing leachate generation and eliminating the need for future onsite leachate storage. For this reason, the leachate storage pond is proposed for decommissioning under the current Phase 2 closure contract. The decommissioning of the pond under the current contract will allow disposal of debris generated from the decommissioning of the leachate storage pond within the landfill.

Leachate Data and Calculations

Quantity Validation

Since the proposed plan is to decommission the leachate storage pond under the current Phase 2 closure contract prior to complete closure of the landfill, the leachate generation rates were modeled for waste depths at the time of closure and prior to installation of the cover cap system. Using the US EPA HELP Model 3.07 the North Wake Subtitle D Landfill was modeled to estimate the peak daily and average annual leachate generation rates at the time of closure. The HELP Model results included in **Attachment No. 2** estimate that the average and peak daily leachate generation rates are 122.5 and 710 gallons per acre per day (gpad), respectively.

With a lined footprint of approximately 67 acres, the resulting average and peak daily leachate discharge rates for the landfill at closure are 8,210 and 47,570 gpd respectively. These rates are well below the maximum permitted discharge rate of 125,000 gpd.

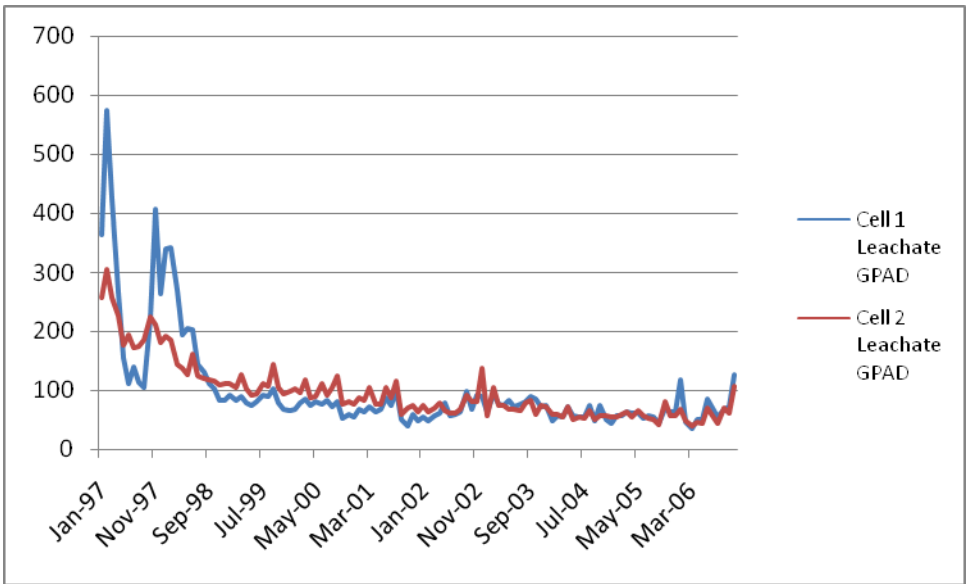
To validate the HELP Model Results and provide further justification that removal of the leachate storage pond is appropriate, data from Cells 1 and 2 of the Delaware Solid Waste Authority's (DSWA's) Southern Solid Waste Management Center (SSWMC) has been provided for comparison. The data was selected as a comparison data set because the DSWA maintains complete records of leachate flow from individual landfill cells; Cells 1 and 2 are Subtitle D lined cells closed in 1997 using an exposed geomembrane; and the average annual precipitation for Raleigh, North Carolina, and Georgetown, Delaware are similar at 42.46 inches and 43.36 inches per year respectively. The similarity in annual precipitation supports selecting the DSWA SSWMC Cells 1 and 2 for comparison because precipitation is the main component in leachate generation.

Figure A-1 shows a line graph of leachate flow from the SSWMC Cells 1 and 2 at the time of closure through November, 2006. As shown, the maximum average daily flow rate occurs near the closure date and is approximately 600 gpad, which is less than the peak daily leachate generation rate of 710 gpad as estimated by the HELP Model results. Review of the complete leachate flow records for Cells 1 and 2 provided in **Attachment 3** shows that the maximum average leachate flow rate for any one month during the life of Cells 1 and 2 is 1,420 gpad. Although not anticipated, if this flow rate were realized at the North Wake Landfill following the decommissioning of the leachate pond, the total flow rate for the landfill would be approximately 95,140 gpd, which is still less than the maximum allowable discharge rate of 125,000 gpd.

In addition, current monitoring of leachate generation rates indicates that leachate generation rates are below the permitted discharge rate of 125,000 gpd. As the area of the impermeable cover cap system is increased through the ongoing closure project leachate generation potential is constantly decreasing.

Figure A-1

SSWMF Cells 1 and 2 Leachate Flow



Quality Validation

As indicated above, leachate discharged from the landfill is required to have a pH between 6.0 and 10.0 standard units and the sum of TTOs with detected concentrations greater than .01 mg/L, must be less than 2.13 mg/L. The historical leachate quality data has not exceeded these conditions to date and based on the nature of leachate, the leachate quality is not expected to fall outside of the limits following landfill closure.

The pH of leachate varies with age of the waste. Leachate pH drops during the acid formation phase of anaerobic decomposition which typically occurs within the first year and then increases during the methane formation phase until it stabilizes in a range between 6 and 8. Since no new waste is being placed in the landfill it is reasonable to assume that the pH will remain within the required limits for discharge to the City of Raleigh Sanitary Sewer System.

The TTOs are not expected to vary significantly from the historical data. Therefore, an exceedance in TTOs is not expected

Sampling and Analysis Plan

In preparation for the decommissioning of the existing leachate storage pond, the following sampling plan has been developed to determine if any contamination has occurred below the leachate storage pond liner.

First, the leachate storage pond will be drained and a bypass pumping system will be installed to divert leachate around the pond and into the leachate pump station west of the leachate storage pond. Then, three cored samples will be extracted from the leachate pond liner system. The concrete and geomembrane components will be visually inspected and a written summary of the visual inspection will be provided in the final sampling report. The clay liner will be analyzed for North Carolina Appendix II Total Volatile Organic Compounds (VOCs) by EPA Method 8260, Total Semi Volatile Organic Compounds (SVOCs) by EPA Method 8270, Polychlorinated biphenyls (PCBs) by EPA Method 8082, Pesticides by EPA Method 8081, Herbicides by EPA Method 8151, and Total Metals by EPA Method 6010 and Mercury by EPA Method 7470. All analytical results will be included in the final sampling report.

If the analytical results do not detect contaminant concentrations greater than the respective remediation goals provided in the North Carolina Division of Waste Management Inactive Hazardous Site Branch Health Based Soil Remediation Goals Table, then no further action will be required.

If analytical results indicate concentrations of contaminants higher than the respective remediation goal, the concrete and geomembrane liners will be completely removed and properly disposed within the landfill. Upon removal of the concrete and geomembrane liners, the vertical and horizontal contamination limits in the underlying clay and subgrade soils will be delineated through visual and analytical techniques.

Leachate Storage Pond Decommissioning Plan

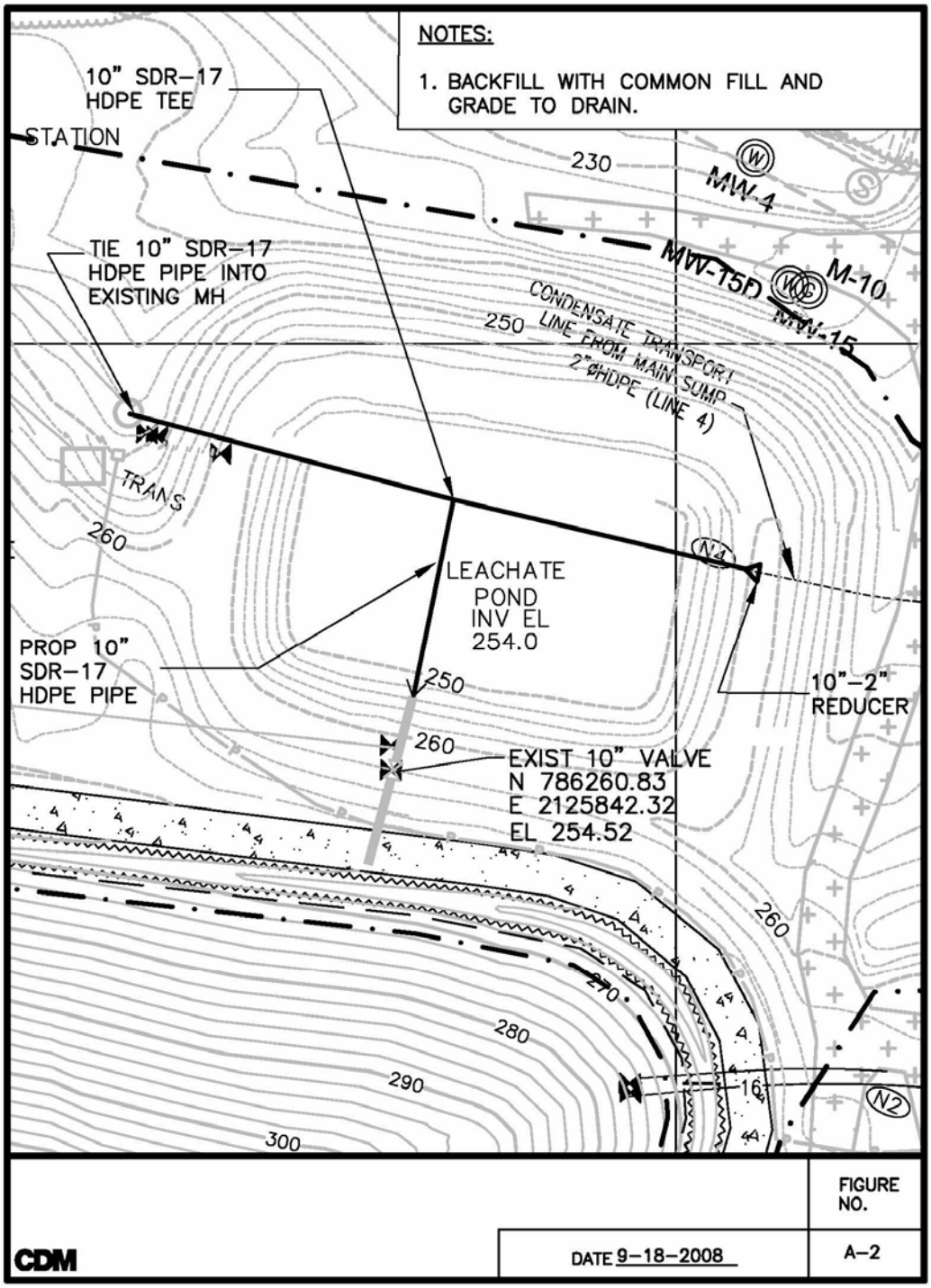
Prior to decommissioning the leachate storage pond, the landfill's leachate collection system will be connected to the leachate pump station west of the existing leachate storage pond using solid 10-inch HDPE pipe as shown in Figure A-2. A connection to the gas condensate line will also be installed in the new section of HDPE pipe, which will allow the condensate from the gas collection system to discharge into the pump station.

Based on results of the pond liner system analysis the pond will be decommissioned in one of two ways. If analysis indicates that no contaminants are present in the clay liner, then the concrete lined surface of the leachate storage pond will be cleaned to remove all traces of waste. The cleaned fabric formed concrete liner and the geomembrane and clay liners will be penetrated multiple times along the pond bottom to promote drainage through the leachate storage pond bottom. The liner penetrations will be filled with common fill and the leachate storage pond will be backfilled to grade, and sloped to provide a natural drainage pattern for surface

runoff. In addition, approximately five feet of the fabric formed concrete and underlying geomembrane and clay liners will be removed from the rim of the leachate storage pond to promote lateral migration of stormwater infiltration and prevent saturation of the surface materials.

If contaminants are present in the clay liner, the fabric formed concrete and geomembrane liner system will be completely removed and properly disposed within the landfill. As previously discussed in the sampling plan, the vertical and horizontal contamination limits in the underlying clay and subgrade soils will be delineated through visual and analytical techniques and the contaminated soil materials will be removed and disposed within the landfill. The vertical and horizontal contamination limits will be recorded and provided in the final sampling report.

Figure A-2


NOTES:
1. BACKFILL WITH COMMON FILL AND GRADE TO DRAIN.
10" SDR-17 HDPE TEE
STATION
TIE 10" SDR-17 HDPE PIPE INTO EXISTING MH
230
MW-4
MW-15D
M-10
MW-15
CONDENSATE TRANSPORT LINE FROM MAIN SUMP 2"ØHDPE (LINE 4)
250
TRANS
260
LEACHATE POND INV EL 254.0
PROP 10" SDR-17 HDPE PIPE
250
10"-2" REDUCER
260
EXIST 10" VALVE
N 786260.83
E 2125842.32
EL 254.52
260
270
280
290
300
N1
N2
CDM
DATE 9-18-2008
FIGURE NO.
A-2

ATTACHMENT 1

City of Raleigh

PERMIT

Industrial User Pretreatment Permit (IUP)
To Discharge Wastewater Under the
Industrial Pretreatment Program

LAND
IUP Number

40 CFR Category

In compliance with the provisions of North Carolina General Statute 143-215.1, any applicable federal categorical pretreatment regulations, all other lawful standards and regulations promulgated and adopted by the North Carolina Environmental Management Commission, and the City of Raleigh Sanitary Sewer Use Ordinance. The following Industry, hereafter referred to by name or as the permittee:

Industry name, permittee:	**Wake County North Wake Landfill**
Facility Located at Street Address	**9004 Deponie Drive**
City	**Raleigh**
State, Zip	**North Carolina 27614**

is hereby authorized to discharge wastewater from the facility located at the above listed address into the sanitary sewer collection system and the wastewater treatment facility of the City of Raleigh listed below:

WWTP name:	**Neuse River Wastewater Treatment Plant**
NPDES Number:	**NC0029033**
WWTP Address:	**P. O. Box 590**
City, State, Zip	**Raleigh, North Carolina, 27602**

in accordance with effluent limitations, monitoring requirements, and all other conditions set forth in Parts I, II, and III of this Industrial User Pretreatment Permit (IUP).

Effective date, this permit and the authorization to discharge shall become effective at midnight on this date:.	**July 1, 2004**
Expiration date, this permit and the authorization to discharge shall expire at midnight on this date:.	**December 31, 2008**

June 17, 2004
Date signed

H. Dale Crisp, Director, Public Utilities

ATTACHMENT 2

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 **              HYDROLOGIC EVALUATION OF LANDFILL PERFORMANCE
**
 **                HELP MODEL VERSION 3.07  (1 NOVEMBER 1997)
**
 **                  DEVELOPED BY ENVIRONMENTAL LABORATORY
**
 **                    USAE WATERWAYS EXPERIMENT STATION
**
 **             FOR USEPA RISK REDUCTION ENGINEERING LABORATORY
**
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 PRECIPITATION DATA FILE:    C:\HELP3\RAL_PREC.D4
 TEMPERATURE DATA FILE:      C:\HELP3\RAL_TEMP.D7
 SOLAR RADIATION DATA FILE:  C:\HELP3\RAL_SOL.D13
 EVAPOTRANSPIRATION DATA:    C:\HELP3\RAL_EVAP.D11
 SOIL AND DESIGN DATA FILE:  C:\HELP3\RAL_SOIL.D10
 OUTPUT DATA FILE:           C:\HELP3\ral\Ral_1.OUT


 TIME:  13:30     DATE:   4/14/2008



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      TITLE:     Wake County, North Wake Landfill
                 North Wake Subtitle D Landfill
                 Leachate Generation at Closure
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      NOTE:  INITIAL MOISTURE CONTENT OF THE LAYERS AND SNOW WATER WERE
               COMPUTED AS NEARLY STEADY-STATE VALUES BY THE PROGRAM.


                                LAYER  1
                                --------

                   TYPE 1 - VERTICAL PERCOLATION LAYER
                       MATERIAL TEXTURE NUMBER   0
          THICKNESS                   =     12.00   INCHES
          POROSITY                    =      0.4570 VOL/VOL
          FIELD CAPACITY              =      0.1310 VOL/VOL
          WILTING POINT               =      0.0580 VOL/VOL
          INITIAL SOIL WATER CONTENT  =      0.2458 VOL/VOL
          EFFECTIVE SAT. HYD. COND.   =  0.999999975000E-04 CM/SEC


                                LAYER  2
                                --------

                   TYPE 1 - VERTICAL PERCOLATION LAYER
                       MATERIAL TEXTURE NUMBER   0
          THICKNESS                   =   2160.00   INCHES
          POROSITY                    =      0.6710 VOL/VOL
          FIELD CAPACITY              =      0.2970 VOL/VOL
          WILTING POINT               =      0.0770 VOL/VOL
          INITIAL SOIL WATER CONTENT  =      0.3015 VOL/VOL
          EFFECTIVE SAT. HYD. COND.   =  0.999999975000E-04 CM/SEC


                                LAYER  3
                                --------

                     TYPE 2 - LATERAL DRAINAGE LAYER
                       MATERIAL TEXTURE NUMBER  21
          THICKNESS                   =     24.00   INCHES
          POROSITY                    =      0.3970 VOL/VOL
          FIELD CAPACITY              =      0.0320 VOL/VOL
          WILTING POINT               =      0.0130 VOL/VOL
          INITIAL SOIL WATER CONTENT  =      0.0434 VOL/VOL
          EFFECTIVE SAT. HYD. COND.   =  0.300000012000     CM/SEC
          SLOPE                       =      5.00   PERCENT
          DRAINAGE LENGTH             =   1300.0    FEET
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LAYER 4

TYPE 4 - FLEXIBLE MEMBRANE LINER
MATERIAL TEXTURE NUMBER 35

THICKNESS	=	0.06	INCHES
POROSITY	=	0.0000	VOL/VOL
FIELD CAPACITY	=	0.0000	VOL/VOL
WILTING POINT	=	0.0000	VOL/VOL
INITIAL SOIL WATER CONTENT	=	0.0000	VOL/VOL
EFFECTIVE SAT. HYD. COND.	=	0.199999996000E-12	CM/SEC
FML PINHOLE DENSITY	=	0.00	HOLES/ACRE
FML INSTALLATION DEFECTS	=	0.00	HOLES/ACRE
FML PLACEMENT QUALITY	=	1 - PERFECT	

GENERAL DESIGN AND EVAPORATIVE ZONE DATA

NOTE: SCS RUNOFF CURVE NUMBER WAS COMPUTED FROM DEFAULT SOIL DATA BASE USING SOIL TEXTURE # 5 WITH A POOR STAND OF GRASS, A SURFACE SLOPE OF 5.% AND A SLOPE LENGTH OF 200. FEET.

SCS RUNOFF CURVE NUMBER	=	78.10	
FRACTION OF AREA ALLOWING RUNOFF	=	100.0	PERCENT
AREA PROJECTED ON HORIZONTAL PLANE	=	1.000	ACRES
EVAPORATIVE ZONE DEPTH	=	10.0	INCHES
INITIAL WATER IN EVAPORATIVE ZONE	=	2.379	INCHES
UPPER LIMIT OF EVAPORATIVE STORAGE	=	4.570	INCHES
LOWER LIMIT OF EVAPORATIVE STORAGE	=	0.580	INCHES
INITIAL SNOW WATER	=	0.000	INCHES
INITIAL WATER IN LAYER MATERIALS	=	655.189	INCHES
TOTAL INITIAL WATER	=	655.189	INCHES
TOTAL SUBSURFACE INFLOW	=	0.00	INCHES/YEAR

EVAPOTRANSPIRATION AND WEATHER DATA

NOTE: EVAPOTRANSPIRATION DATA WAS OBTAINED FROM RALEIGH NORTH CAROLINA

STATION LATITUDE	=	35.87	DEGREES
MAXIMUM LEAF AREA INDEX	=	0.00	
START OF GROWING SEASON (JULIAN DATE)	=	86	
END OF GROWING SEASON (JULIAN DATE)	=	310	
EVAPORATIVE ZONE DEPTH	=	10.0	INCHES

AVERAGE ANNUAL WIND SPEED = 7.70 MPH
AVERAGE 1ST QUARTER RELATIVE HUMIDITY = 66.00 %
AVERAGE 2ND QUARTER RELATIVE HUMIDITY = 70.00 %
AVERAGE 3RD QUARTER RELATIVE HUMIDITY = 78.00 %
AVERAGE 4TH QUARTER RELATIVE HUMIDITY = 72.00 %

NOTE: PRECIPITATION DATA WAS SYNTHETICALLY GENERATED USING
COEFFICIENTS FOR RALEIGH NORTH CAROLINA

NORMAL MEAN MONTHLY PRECIPITATION (INCHES)

JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
3.55	3.43	3.69	2.91	3.67	3.66
4.38	4.44	3.29	2.73	2.87	3.14

NOTE: TEMPERATURE DATA WAS SYNTHETICALLY GENERATED USING
COEFFICIENTS FOR RALEIGH NORTH CAROLINA

NORMAL MEAN MONTHLY TEMPERATURE (DEGREES FAHRENHEIT)

JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
39.60	41.60	49.30	59.50	67.20	73.90
77.70	77.00	71.00	59.70	50.00	42.00

NOTE: SOLAR RADIATION DATA WAS SYNTHETICALLY GENERATED USING
COEFFICIENTS FOR RALEIGH NORTH CAROLINA
AND STATION LATITUDE = 35.87 DEGREES

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AVERAGE MONTHLY VALUES IN INCHES FOR YEARS 1 THROUGH 5

	JAN/JUL	FEB/AUG	MAR/SEP	APR/OCT	MAY/NOV	JUN/DEC
PRECIPITATION						
TOTALS	3.26	2.82	4.37	2.63	3.53	5.42
	4.57	6.26	2.81	3.77	2.52	2.90

```
     STD. DEVIATIONS        2.96     0.95     1.30     1.89     2.64
1.99
                            2.29     5.93     1.69     2.69     1.66
1.28

   RUNOFF
   ------
     TOTALS                 0.223    0.045    0.161    0.004    0.225
0.168
                            0.232    0.817    0.406    0.281    0.029
0.148

     STD. DEVIATIONS        0.397    0.045    0.278    0.005    0.504
0.175
                            0.168    1.693    0.779    0.470    0.041
0.254

   EVAPOTRANSPIRATION
   ------------------
     TOTALS                 1.538    1.959    2.367    2.636    2.882
3.376
                            3.250    2.956    2.037    1.924    1.683
1.268

     STD. DEVIATIONS        0.284    0.297    0.403    1.043    1.201
0.971
                            1.353    1.111    1.036    0.670    0.677
0.165

   LATERAL DRAINAGE COLLECTED FROM LAYER  3
   ----------------------------------------
     TOTALS                 0.1026   0.0798   0.1061   0.1744   0.1131
0.0869
                            0.2672   0.2171   0.1331   0.1143   0.1229
0.1292

     STD. DEVIATIONS        0.0107   0.0535   0.0831   0.2384   0.1720
0.0452
                            0.1323   0.1621   0.0795   0.0981   0.0603
0.1048

   PERCOLATION/LEAKAGE THROUGH LAYER  4
   -------------------------------------
     TOTALS                 0.0000   0.0000   0.0000   0.0000   0.0000
0.0000
                            0.0000   0.0000   0.0000   0.0000   0.0000
0.0000

     STD. DEVIATIONS        0.0000   0.0000   0.0000   0.0000   0.0000
0.0000
                            0.0000   0.0000   0.0000   0.0000   0.0000
0.0000
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                AVERAGES OF MONTHLY AVERAGED DAILY HEADS (INCHES)
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   DAILY AVERAGE HEAD ON TOP OF LAYER  4
   -------------------------------------
     AVERAGES               0.0507   0.0432   0.0525   0.0891   0.0559
0.0444
                            0.1321   0.1073   0.0680   0.0565   0.0628
0.0639

     STD. DEVIATIONS        0.0053   0.0290   0.0411   0.1218   0.0850
0.0231
                            0.0654   0.0801   0.0406   0.0485   0.0308
0.0518


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      AVERAGE ANNUAL TOTALS & (STD. DEVIATIONS) FOR YEARS    1 THROUGH
5
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                                       INCHES            CU. FEET
PERCENT
                                 -------------------   -------------   ---
------
  PRECIPITATION                   44.86    (   9.753)     162856.3
100.00

  RUNOFF                           2.739   (  3.1403)       9941.05
6.104

  EVAPOTRANSPIRATION              27.876   (  0.9707)     101190.28
62.135

  LATERAL DRAINAGE COLLECTED      1.64665 (  0.78993)      5977.352
3.67032
    FROM LAYER  3

  PERCOLATION/LEAKAGE THROUGH     0.00000 (  0.00000)         0.013
0.00001
    LAYER  4
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  AVERAGE HEAD ON TOP             0.069 (    0.033)
    OF LAYER  4

  CHANGE IN WATER STORAGE        12.603   (  6.0718)      45747.70
28.091


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                  PEAK DAILY VALUES FOR YEARS    1 THROUGH    5
    ---------------------------------------------------------------------
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                                                  (INCHES)      (CU. FT.)
                                                 ----------   ------------
-
       PRECIPITATION                               5.22         18948.600

       RUNOFF                                      2.706         9821.3838

       DRAINAGE COLLECTED FROM LAYER  3            0.02614
94.89672

       PERCOLATION/LEAKAGE THROUGH LAYER  4        0.000000
0.00016

       AVERAGE HEAD ON TOP OF LAYER  4             0.401

       MAXIMUM HEAD ON TOP OF LAYER  4             0.799

       LOCATION OF MAXIMUM HEAD IN LAYER  3
             (DISTANCE FROM DRAIN)                 0.0 FEET

       SNOW WATER                                  1.67          6061.1182


       MAXIMUM VEG. SOIL WATER (VOL/VOL)                   0.4570

       MINIMUM VEG. SOIL WATER (VOL/VOL)                   0.1164


        ***  Maximum heads are computed using McEnroe's equations.  ***

             Reference:  Maximum Saturated Depth over Landfill Liner
                         by Bruce M. McEnroe, University of Kansas
                         ASCE Journal of Environmental Engineering
                         Vol. 119, No. 2, March 1993, pp. 262-270.
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FINAL WATER STORAGE AT END OF YEAR 5

LAYER	(INCHES)	(VOL/VOL)
1	3.3213	0.2768
2	713.8151	0.3305
3	1.0655	0.0444
4	0.0000	0.0000
SNOW WATER	0.000	

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ATTACHMENT 3

SSWMC Historical
Leachate Generation and
Movement

Month/Year	Cell 1 (GPAD)	Cell 2 (GPAD)
Jan-86		
Feb-86	25	0
Mar-86	29	0
Apr-86	34	0
May-86	22	0
Jun-86	18	0
Jul-86	31	0
Aug-86	45	0
Sep-86	27	0
Oct-86	23	0
Nov-86	26	0
Dec-86	74	0
Jan-87	116	0
Feb-87	180	0
Mar-87	146	0
Apr-87	106	0
May-87	145	0
Jun-87	152	0
Jul-87	132	0
Aug-87	72	0
Sep-87	61	0
Oct-87	69	0
Nov-87	64	0
Dec-87	143	0
Jan-88	193	0
Feb-88	465	0
Mar-88	266	0
Apr-88	346	0
May-88	222	0
Jun-88	129	0
Jul-88	79	0
Aug-88	98	0
Sep-88	133	0
Oct-88	90	0
Nov-88	239	0
Dec-88	155	0
Jan-89	190	0
Feb-89	196	0
Mar-89	516	0
Apr-89	446	0
May-89	348	0
Jun-89	301	0
Jul-89	0	0
Aug-89	0	0
Sep-89	0	0
Oct-89	0	0
Nov-89	0	0
Dec-89	0	0
Jan-90	304	58
Feb-90	229	38

Mar-90	168	28
Apr-90	220	48
May-90	178	47
Jun-90	146	17
Jul-90	121	37
Aug-90	114	18
Sep-90	76	11
Oct-90	72	33
Nov-90	74	35
Dec-90	125	59
Jan-91	267	176
Feb-91	104	96
Mar-91	163	107
Apr-91	133	128
May-91	114	84
Jun-91	143	90
Jul-91	136	90
Aug-91	190	95
Sep-91	115	56
Oct-91	123	61
Nov-91	109	64
Dec-91	175	58
Jan-92	159	60
Feb-92	170	82
Mar-92	207	129
Apr-92	134	74
May-92	114	88
Jun-92	142	115
Jul-92	97	74
Aug-92	181	344
Sep-92	180	313
Oct-92	131	148
Nov-92	208	165
Dec-92	261	144
Jan-93	321	155
Feb-93	199	91
Mar-93	312	146
Apr-93	227	202
May-93	174	123
Jun-93	119	153
Jul-93	92	111
Aug-93	91	139
Sep-93	107	217
Oct-93	93	174
Nov-93	216	165
Dec-93	294	210
Jan-94	298	1,365
Feb-94	515	798
Mar-94	462	1,420
Apr-94	331	1,413
May-94	170	357
Jun-94	150	281
Jul-94	117	272
Aug-94	253	361
Sep-94	232	280
Oct-94	139	246
Nov-94	250	236

Dec-94	146	228
Jan-95	260	217
Feb-95	253	210
Mar-95	300	223
Apr-95	134	177
May-95	273	229
Jun-95	149	206
Jul-95	132	179
Aug-95	102	146
Sep-95	82	163
Oct-95	192	169
Nov-95	321	231
Dec-95	288	190
Jan-96	833	347
Feb-96	684	366
Mar-96	342	225
Apr-96	747	479
May-96	343	462
Jun-96	248	244
Jul-96	276	312
Aug-96	187	223
Sep-96	375	240
Oct-96	316	245
Nov-96	135	189
Dec-96	519	308
Jan-97	364	258
Feb-97	573	306
Mar-97	423	258
Apr-97	267	227
May-97	155	177
Jun-97	112	195
Jul-97	140	172
Aug-97	114	174
Sep-97	105	186
Oct-97	222	226
Nov-97	405	211
Dec-97	264	181
Jan-98	340	193
Feb-98	343	185
Mar-98	271	144
Apr-98	194	138
May-98	204	128
Jun-98	202	163
Jul-98	144	124
Aug-98	131	120
Sep-98	111	119
Oct-98	102	116
Nov-98	82	110
Dec-98	83	113
Jan-99	90	112
Feb-99	83	105
Mar-99	90	126
Apr-99	78	102
May-99	75	93
Jun-99	80	95
Jul-99	90	112
Aug-99	89	107

Sep-99	102	144
Oct-99	78	105
Nov-99	68	94
Dec-99	66	100
Jan-00	67	104
Feb-00	78	96
Mar-00	84	119
Apr-00	73	89
May-00	81	90
Jun-00	76	112
Jul-00	82	93
Aug-00	72	105
Sep-00	80	126
Oct-00	51	78
Nov-00	59	83
Dec-00	54	77
Jan-01	67	89
Feb-01	64	83
Mar-01	71	104
Apr-01	63	77
May-01	66	78
Jun-01	86	105
Jul-01	74	89
Aug-01	101	116
Sep-01	50	60
Oct-01	39	71
Nov-01	58	76
Dec-01	48	63
Jan-02	53	75
Feb-02	48	64
Mar-02	56	70
Apr-02	60	80
May-02	79	66
Jun-02	56	61
Jul-02	58	61
Aug-02	63	69
Sep-02	97	92
Oct-02	68	83
Nov-02	92	81
Dec-02	89	139
Jan-03	64	58
Feb-03	94	105
Mar-03	75	74
Apr-03	74	75
May-03	83	68
Jun-03	72	68
Jul-03	75	67
Aug-03	79	80
Sep-03	90	83
Oct-03	85	60
Nov-03	71	74
Dec-03	75	74
Jan-04	48	59
Feb-04	56	59
Mar-04	54	55
Apr-04	72	73
May-04	57	51

Jun-04	54	56
Jul-04	53	54
Aug-04	73	67
Sep-04	47	52
Oct-04	74	58
Nov-04	49	58
Dec-04	43	55
Jan-05	55	56
Feb-05	57	60
Mar-05	62	63
Apr-05	60	56
May-05	61	66
Jun-05	52	57
Jul-05	57	54
Aug-05	55	52
Sep-05	43	42
Oct-05	68	81
Nov-05	64	58
Dec-05	63	57
Jan-06	117	68
Feb-06	45	48
Mar-06	35	40
Apr-06	49	46
May-06	51	45
Jun-06	85	70
Jul-06	68	57
Aug-06	54	45
Sep-06	68	70
Oct-06	71	63
Nov-06	127	108
Dec-06		
Jan-07		
Feb-07		
Mar-07		
Apr-07		
May-07		
Jun-07		
Jul-07		
Aug-07		
Sep-07		
Oct-07		
Nov-07		
Dec-07		
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Months	225	225
Minimum	0	0
Maximum	833	1,420
Average	146	120
Total		