

Ashe Co. 05-01

SOIL & MATERIAL ENGINEERS INC. ENGINEERING-TESTING-INSPECTION

1903 Harrison Avenue, Box 609, Cary, NC 27511, Phone (919) 481-0397 *Horizontal Exp.*
January 8, 1985

Municipal Engineering Service
P. O. Box 97
Garner, NC 27529

Attention: Mr. Jimmy D. Woodie

Reference: Ashe County Landfill Geologic Study
Ashe County, North Carolina
S&ME Project Number: 055-84-018-A

Gentlemen:

Based on your authorization, Soil & Material Engineers, Inc. (S&ME) has completed a geological investigation for a proposed 78 acre landfill expansion site which is adjacent to the existing Ashe County landfill. This land is located approximately 1½ miles northwest of Chestnut Hill, North Carolina, off State Road 1558, as depicted on Figure 1. The purpose of this investigation is to describe the subsurface soil, rock, ground water, and topographic and geologic conditions indicated by the test borings, laboratory analysis, and observations, in accordance with Solid and Hazardous Waste Management Branch, regulations for Solid Waste Management 10NCAC 10G. Discussion is also provided regarding the suitability and limitations of this site for landfilling purposes based on the data developed.

This report will be used by Municipal Engineering Service and the County for submittal of a site approval application and ultimately to develop an operations plan. It is our understanding that this landfill is to be used for the disposal of residential and non-toxic industrial wastes. No hazardous wastes will be disposed at this site.

RALEIGH, GREENSBORO, ASHEVILLE, WILMINGTON, FAYETTEVILLE, CHARLOTTE, NC
SPARTANBURG, COLUMBIA, CHARLESTON, MYRTLE BEACH, SC
ATLANTA, ALBANY, GA—TRI-CITIES, TN—CINCINNATI, OH—ORLANDO, TAMPA, FL

split-barrel sampling procedure (ASTM D-1586). All soil samples collected were classified by the Unified Soil Classification System (ASTM D-2487) and recorded on the attached Test Boring Records. In addition to the soil test borings, three auger probes were performed (Borings 1, 7, and 8) to determine depth to rock in areas that were likely to be used as cover material. Visual soil classification was made during the boring and is also contained on the attached Test Boring Records.

At the completion of drilling each borehole, a temporary piezometer was constructed and observations made for water accumulating in the borehole. A representative of S&ME also measured water levels in the boreholes approximately two weeks later. Based on current solid waste and ground water regulations, these temporary monitoring wells must be grouted closed prior to construction of the landfill or after any further water level observations are completed.

SITE CONDITIONS

The proposed landfill site area is estimated to be approximately 78 acres. However, due to perennial drainage features and buffer zones, only approximately 40 to 50 acres are being considered as a proposed landfill. This tract has a high topographic point along its eastern border (elevation 2,800 to 2,840) with a moderate to steep slope to the northwest and overall topographic relief of approximately 200 feet. The drainage at the site is characterized by three northwest trending drainage swales which connect just to the north of the site to a northward draining perennial stream. These swales divide the site into a series of north to northwest trending ridges. During the field reconnaissance, surface water was noticed at the lower (northern) reaches of the the drainage swales near the property boundaries. The perennial stream also crosses the property to the west of the area proposed for landfilling.

A north-south trending ridge to the west of the perennial stream is tentatively proposed as a future borrow area.

The results of the test borings and auger probes indicate that the surface conditions at this site consist of a very thin organic topsoil cover with a number



Ground Water

Ground water was observed in four boreholes ranging between 8.8 and 40.7 feet below ground surface. The phreatic surface is apparently below the bottom of the boreholes at the other test borings. Ground water levels may be expected to fluctuate due to many factors, including precipitation, evapotranspiration, and surface drainage. The North Carolina Department of Human Resources requires that the bottom of the landfill be a minimum of 4 feet above the seasonal high phreatic surface.

It is understood that the proposed method of landfilling at this site will be similar to the adjacent existing site. This is a "terracing" or "stair-stepping" method which will involve disposal in the dry upper portions of the existing swales with cover being excavated from the adjoining ridges. Based on observed water levels, the lower (northern) reaches of the three drainage swales cannot be used for landfilling since ground water is expected to exist at/or within 4 feet of the ground surface.

Excavation

One of the major considerations regarding the use of a landfill site is whether difficult excavation will be encountered due to weathered or sound rock. Although thin rock lenses were encountered in some borings, overall results indicate that the general depth of weathered rock ranges between 15 to 60 feet below land surface. When examining the weathered rock on existing highwalls of the present landfill, there is also a 10 to 20 foot section of weathered saprolite. In communication with the superintendent of the landfill, he explained that the front end loader presently used for excavation has historically had no problems excavating the weathered saprolite. Therefore, S&ME feels that the 15 to 60 foot excavation depths are reasonable figures and that the limiting criteria for depth in most areas will apparently be ground water.

Excavation to the proposed depths will create a number of high temporary slopes. No laboratory soil strength testing or slope stability analysis has been performed as a part of this study. Most soils of this type will stand on 1(v):1(h) temporary slope to heights of 30 to 50 feet. However, all such slopes should be



ground water monitoring wells will be required hydraulically downgradient from the landfill to detect any leachate infiltrating into the ground water.

Also, several deep domestic ground water wells exist along the eastern border of the proposed landfill property. Solid Waste Management Regulations require a minimum distance of 500 feet between domestic wells and buried wastes. It is recommended that at least one ground water monitoring well be located along this boundary and that periodic ground water quality sampling also be performed from the domestic wells to assess any possible contamination.

CONCLUSIONS

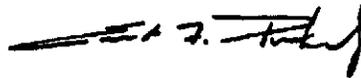
Overall, this site appears suitable for a properly designed and operated sanitary landfill. Careful design and operation will be required due to the nearby domestic wells, topographic slopes, and the proximity of shallow ground and surface water at the lower elevations.

We appreciate the opportunity to be of service to you. If you have any questions or if we can be of further assistance, please contact us.

Sincerely yours,
Soil & Material Engineers, Inc.



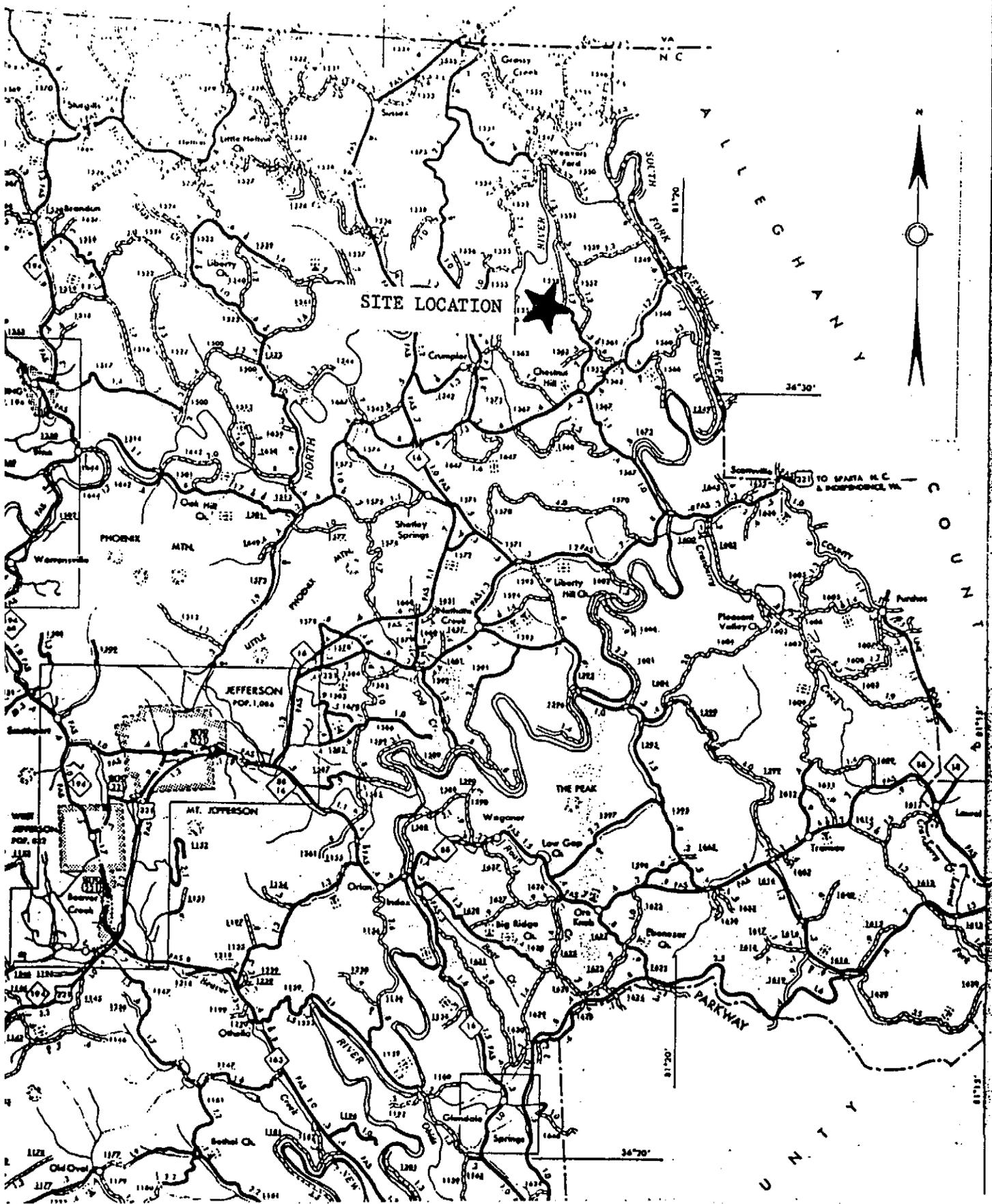
Phillip L. Rahn, Geologist



Ernest F. Parker, Jr. P.E.

PLR/pjc



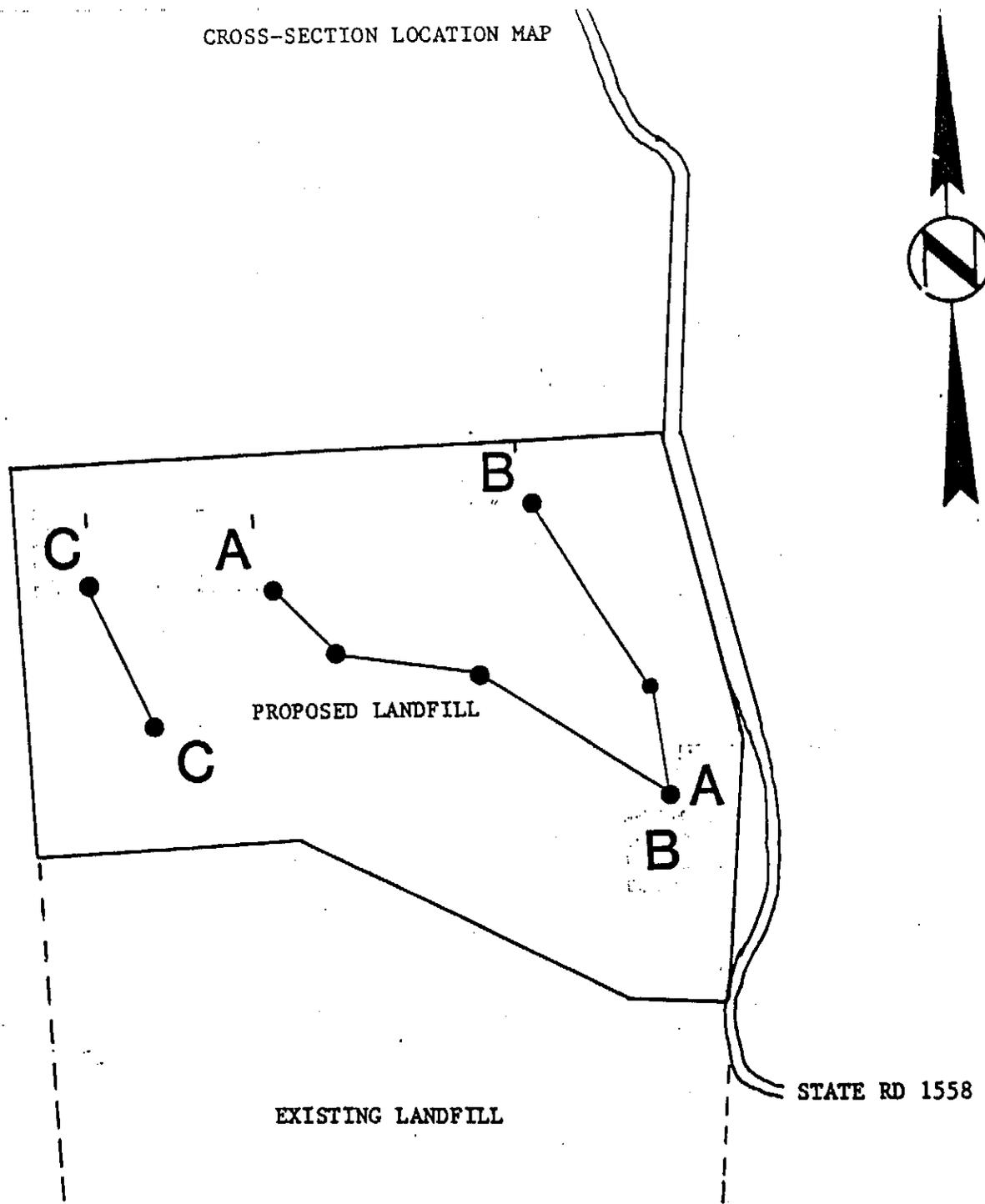


OBJECT
 SHEPHERD COUNTY LANDFILL
 GEOLOGIC STUDY
 HESTNUT HILL, N.C.

SOIL & MATERIAL ENGINEERS, INC.
 RALEIGH, NORTH CAROLINA

SCALE: N.T.S.
JOB NO: 055-84-018A
FIG. NO: 1

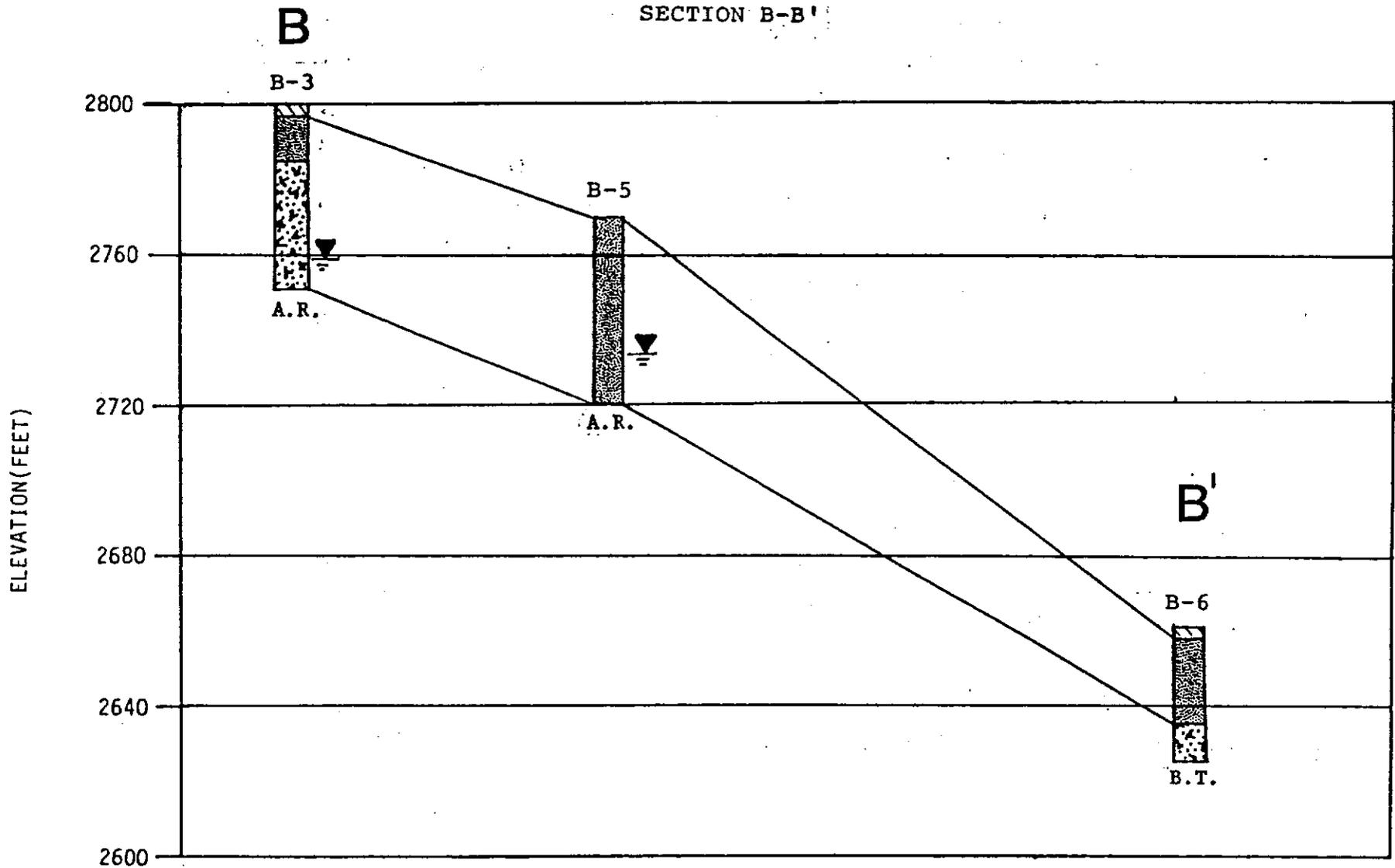
CROSS-SECTION LOCATION MAP



NOTE: BORING LOCATIONS ARE APPROXIMATE

<p>PROJECT HE COUNTY LANDFILL OLOGIC STUDY ESTNUT HILL, N.C.</p>	<p>SOIL & MATERIAL ENGINEERS, INC. RALEIGH, NORTH CAROLINA</p>	<p>SCALE: APPROX. 1"=500' JOB NO: 055-84-018A FIG. NO: 3</p>
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SECTION B-B'



A.R.=AUGER REFUSAL

B.T.=BORING TERMINATED

NOTE: TEST BORING ELEVATIONS ARE ESTIMATED

LEGEND: SILT, CLAY-SAND-SILT MIXTURE

CLAY, SAND-SILT-CLAY MIXTURE

WEATHERED ROCK

PROJECT

ASHE COUNTY LANDFILL
GEOLOGIC STUDY
CHESTNUT HILL, N.C.

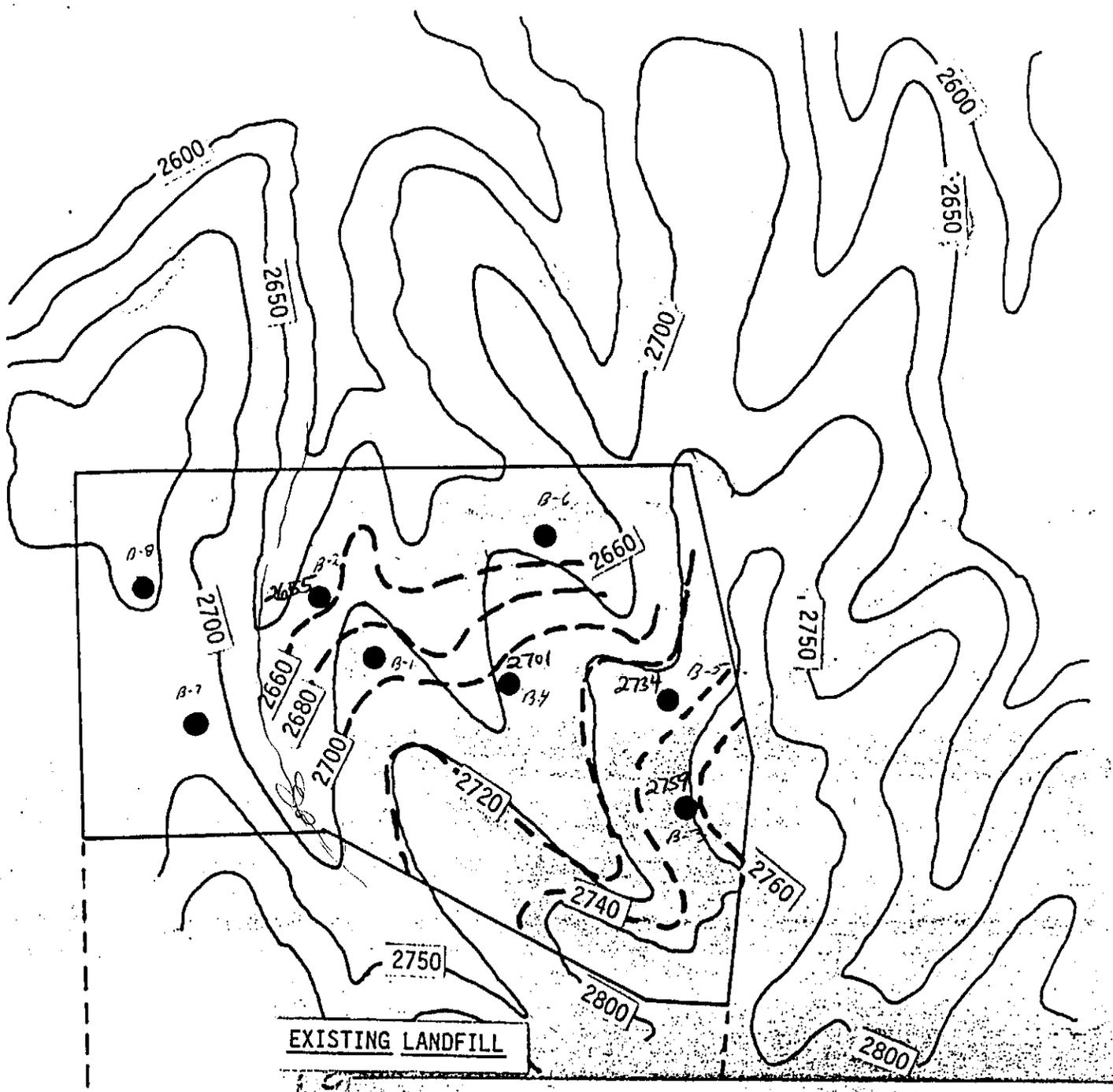
SOIL & MATERIAL ENGINEERS, INC.
RALEIGH, NORTH CAROLINA

SCALE: H:1"=200' V:1"=40'

JOB NO: 055-84-018A

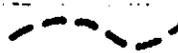
FIG NO: 5

ESTIMATED POTENTIOMETRIC CONTOUR MAP



EXISTING LANDFILL

LEGEND

- CONTOUR INTERVAL (50 FEET) 
- POTENTIOMETRIC INTERVAL 

PROJECT
HE COUNTY LANDFILL
GEOLOGIC STUDY
ESTNUT HILL, N.C.

SOIL & MATERIAL ENGINEERS, INC.
RALEIGH, NORTH CAROLINA

SCALE: APPROX. 1"=500'
JOB NO: 055-84-018A
FIG. NO: 7

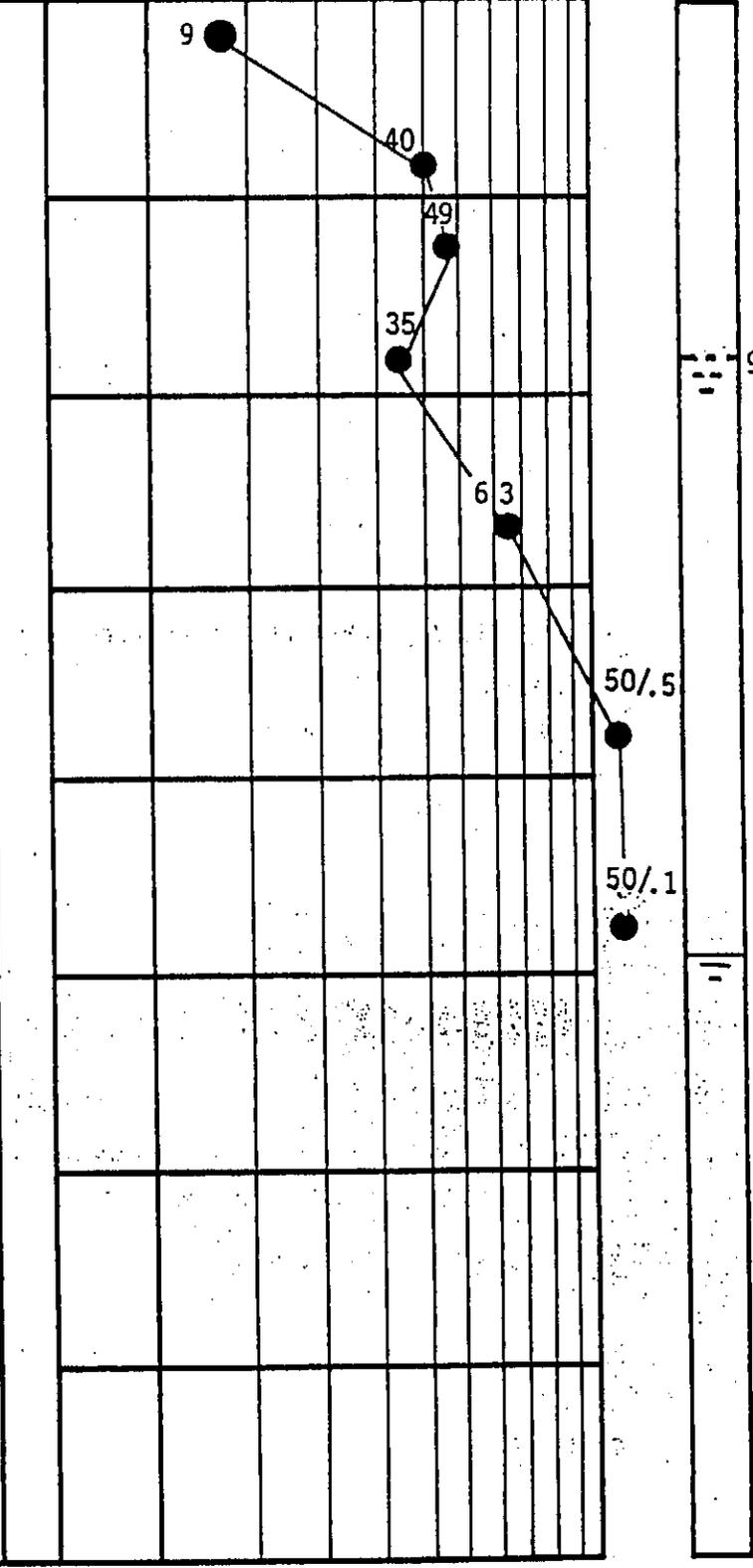
0 10 20 30 40 60 80 100

Stiff Brown Micaceous Slightly Sandy Silty CLAY (CL)

Brown Gray-White Very Micaceous Slightly Clayey Slightly Sandy SILT (ML) with Feldspar and Pyrite Fragments

Weathered Rock-Sampled Same as Above.

Auger Refusal at 25.0 feet.
No Water Encountered at Time of Boring.



TEST BORING RECORD

DRILLING AND SAMPLING MEETS ASTM D-1586
CORE DRILLING MEETS ASTM D-2113

BORING NO. B-2
DATE DRILLED 12/07/84
JOB NO. 055-84-018-A

PENETRATION IS THE NUMBER OF BLOWS OF 140 LB. HAMMER
NECESSARY TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

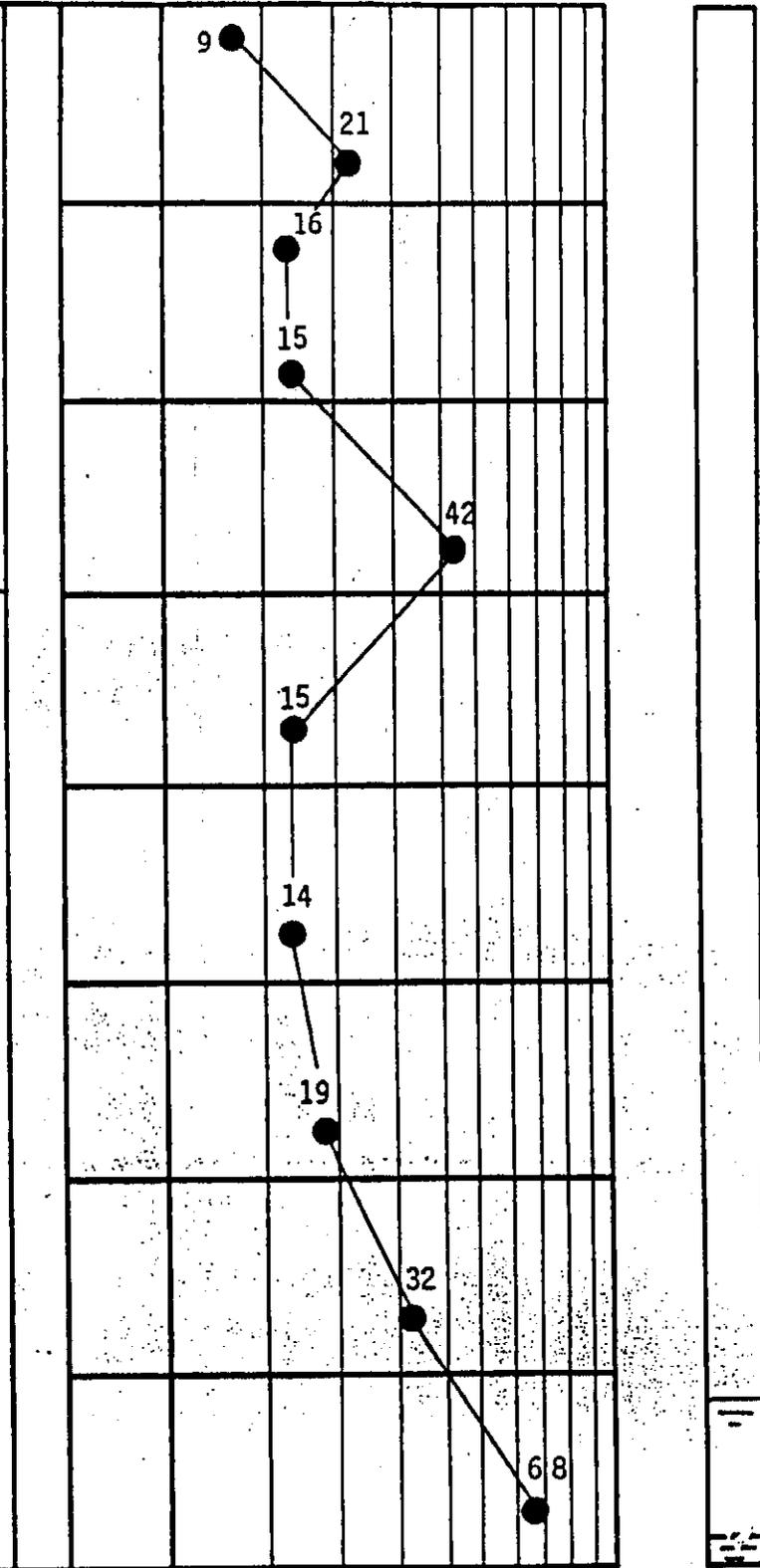
- UNDISTURBED SAMPLE
- ▬ WATER TABLE-24HR.
- ▬ WATER TABLE-1HR.
- % ROCK CORE RECOVERY
- LOSS OF DRILLING WATER

SOIL & MATERIAL ENGINEERS, INC.

0 10 20 30 40 60 80 100

Stiff-Hard Orange-Tan-White Slightly Clayey Sandy SILT (ML) with Feldspar Fragments

Stiff-Hard Brown Micaceous Clayey SILT (ML) with Pyrite and Feldspar Fragments



36.0'
39.1'

TEST BORING RECORD

ING AND SAMPLING MEETS ASTM D-1586
 E DRILLING MEETS ASTM D-2113
 STRATION IS THE NUMBER OF BLOWS OF 140 LB. HAMMER
 LING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

■ UNDISTURBED SAMPLE ≡ WATER TABLE-24HR.
 % ROCK CORE RECOVERY ≡ WATER TABLE-1HR.
 LOSS OF DRILLING WATER

BORING NO. B-5
 DATE DRILLED 12/06/84
 JOB NO. 055-84-018-A

SOIL & MATERIAL ENGINEERS, INC.

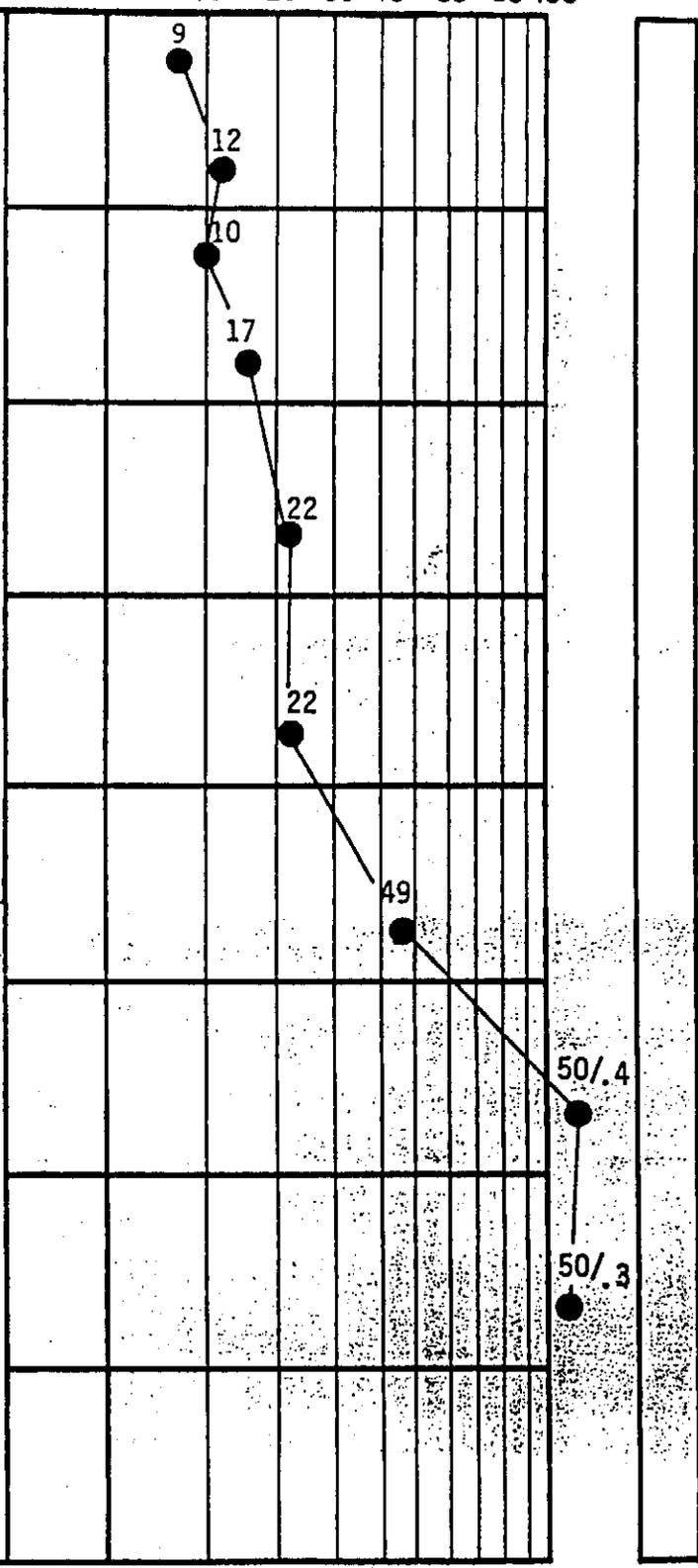
0 10 20 30 40 60 80 100

Stiff Red-Brown Micaceous Silty CLAY (CL)

Stiff-Hard Tan-Brown and Gray-White Micaceous Slightly Clayey Slightly Sandy SILT (ML) with Feldspar and Pyrite Fragments

Weathered Rock - Sampled Same as Above

Boring Terminated at 35.0 feet.
No Water Encountered at Time of Boring.



TEST BORING RECORD

TESTING AND SAMPLING MEETS ASTM D-1586
 DRILLING MEETS ASTM D-2113
 PENETRATION IS THE NUMBER OF BLOWS OF 140 LB. HAMMER
 DRIVING 30 IN. REQUIRED TO DRIVE 1.4 IN. I.D. SAMPLER 1 FT.

UNDISTURBED SAMPLE
 % ROCK CORE RECOVERY
 LOSS OF DRILLING WATER

WATER TABLE-24HR.
 WATER TABLE-1HR.

BORING NO. B-6
 DATE DRILLED 12/07/84
 JOB NO. 055-84-018-A

SOIL & MATERIAL ENGINEERS, INC.

Tables

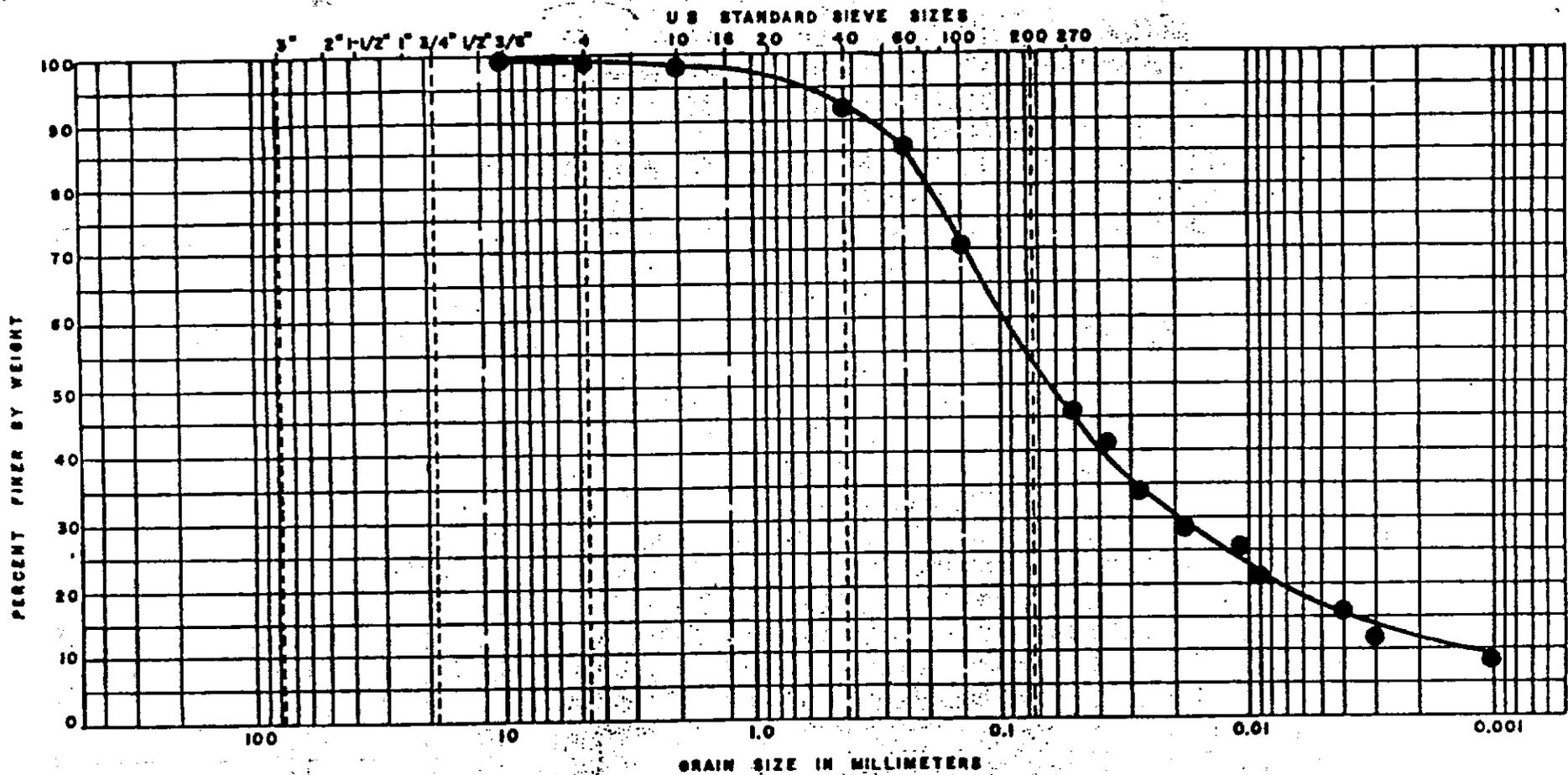
Table 2
Laboratory Data Summary
Ashe County Proposed Landfill
Chestnut Hill, North Carolina
S&ME Project Number: 055-84-018-A

<u>Boring Number</u>	<u>Sample Type</u>	<u>Sample Depth (ft)</u>	<u>Natural Moisture (%)</u>	<u>Liquid Limit</u>	<u>Plasticity Index</u>	<u>Specific Gravity</u>	<u>Dry Unit Weight (pcf)</u>	<u>Saturated Conductivity(cm/sec)</u>
B-1	Bag	0 - 2'	6.2	32	5	2.712	108.4	<u>5.0 x 10⁻⁴ * X Remolded</u>
B-3	Bag	0 - 2'	5.2	-	N.P.	2.676		
B-4	U.D.	10 - 12'	22.06	-	N.P.	2.872	108.02	7.0 x 10 ⁻⁴
B-8	Bag	0 - 2'	7.3	36	1.0	2.733		

U.D. = Undisturbed Sample

N.P. = Non-Plastic

* Remolded to Approximately 95% of Maximum Dry Density



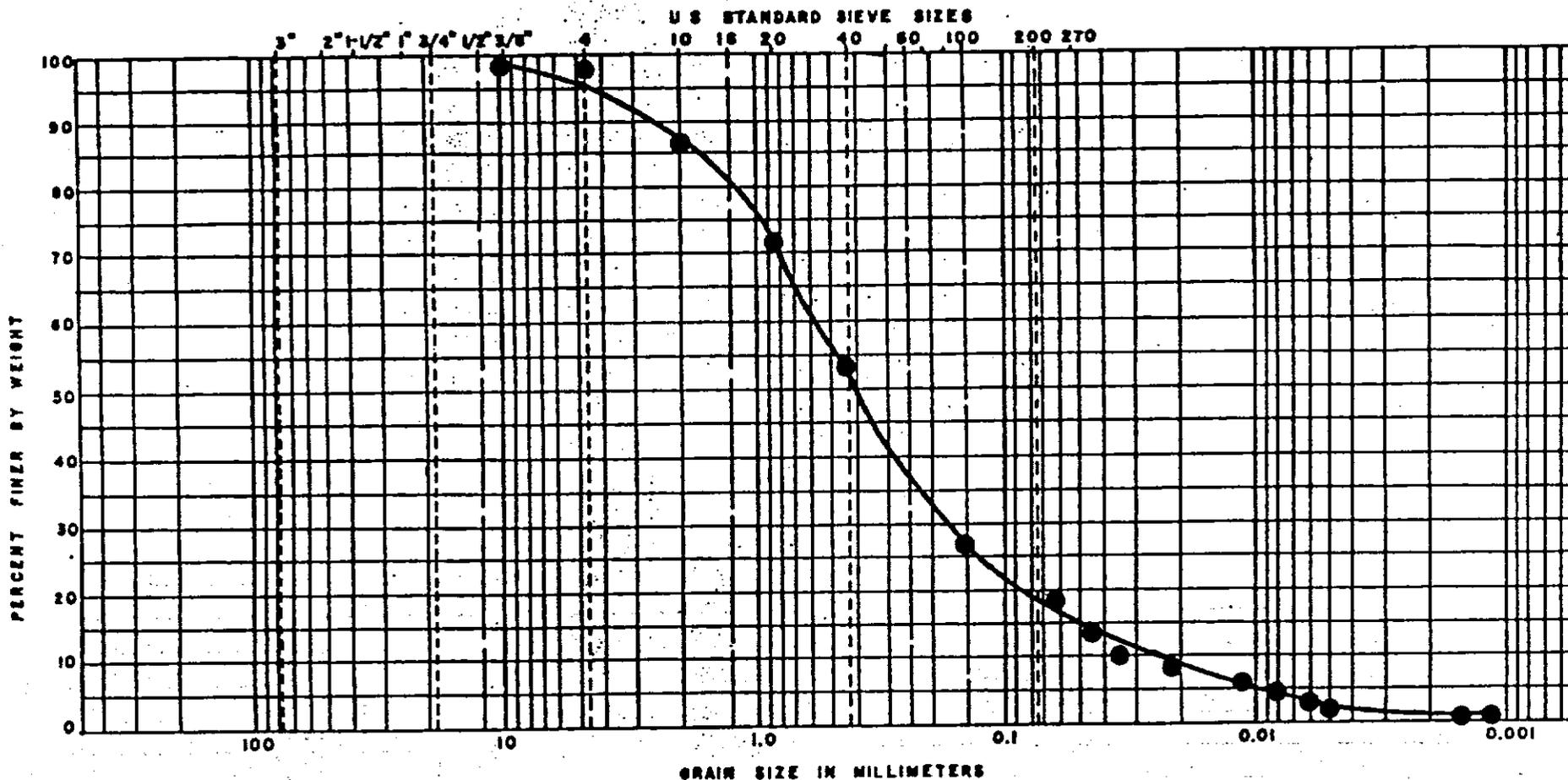
BOUL DERS	COBBLES	GRAVEL		SAND			FINES	
		COARSE	FINE	COARSE	MEDIUM	FINE	SILT SIZES	CLAY SIZES

BORING NO	ELEV OR DEPTH	NAT WC	LL	PL	PI	DESCRIPTION OR CLASSIFICATION
B-1	0-2'	6.2	32	27	5	Brown Micaceous Silty SAND - <i>ML</i> ■ Silty silt <i>A-4</i>

GRAIN SIZE DISTRIBUTION

JOB NO. 055-84-018-A

SOIL & MATERIAL ENGINEERS, INC.



BOUL DERS	COBBLES	GRAVEL		SAND			FINES	
		COARSE	FINE	COARSE	MEDIUM	FINE	SILT SIZES	CLAY SIZES

BORING NO	ELEV. OR DEPTH	NAT WC	LL	PL	PI	DESCRIPTION OR CLASSIFICATION
B-4	10 - 12'	22.06%	Non-Plastic			Brown Micaceous Silty SAND SM Sand A-2-4

GRAIN SIZE DISTRIBUTION

JOB NO. 055-84-006-A

SOIL & MATERIAL ENGINEERS, INC.

does not match boring log description

Proctor Test Results

SOIL & MATERIAL ENGINEERS, INC.

COMPACTION TEST

Boring B-3

JOB NUMBER 055-84-018-A
JOB NAME Ashe County Landfill
JOB LOCATION Ashe County
North Carolina

MOISTURE - DENSITY
RELATIONSHIP
METHOD OF TEST D-698A
MAX. DRY DENSITY 108.0 PCF
OPT. MOISTURE CONTENT 16.3 %
NAT. MOISTURE CONTENT 5.23 %
ATTERBERG LIMITS LL - PI N.P.
SOIL DESCRIPTION Tan Silty SAND

DRY DENSITY - POUNDS PER CUBIC FOOT

CURVES OF 100% SATURATION FOR
SPECIFIC GRAVITIES EQUAL TO:

2.80
2.70
2.60

